

ABSTRACT

THE DYNAMIC STABILITY OF RECTANGULAR PLATES

by

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Master of Science in Engineering

Youngstown State University, 1974

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## ABSTRACT

## THE DYNAMIC STABILITY OF RECTANGULAR PLATES

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The purpose of this thesis is to study the dynamic stability of rectangular plates subjected to transverse bending stress, shear stress and transverse and rotary inertia.

The proper equations of motion are solved using the classical Levy method for a rectangular plate simply supported along two parallel sides. The resulting set of equations are applicable to a particular group of rectangular plate problems subjected to various applied static and dynamic forces with various boundary restraints on the opposite two parallel sides. Thusly, the natural frequency of free vibration and the critical buckling-load are directly attainable.

The natural frequencies and static stability loads are analyzed by resulting equations for the special case of a simply supported rectangular plate on all four edges subjected to biaxial compression forces.

The curves and tables for the natural frequencies and critical buckling-loads are prepared for the simply supported rectangular plate on all four edges with uniaxial compression force acting on it.

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## LIST OF SYMBOLS

SYMBOL	DEFINITION
a,b	Rectangular plate dimension along the x and y axis respectively.
D	Flexural rigidity of the rectangular plate.
E	Modulus of elasticity.
G	Shear modulus of elasticity.
h	Thickness of the rectangular plate.
m,n	Number of modes in the x and y direction respectively.
q	Distributed transverse loading.
t	Time parameter.
u,v,w	Displacement components in direction of x,y, z respectively for any arbitrary point in the plate.
x,y,z	Rectangular co-ordinates.
$\alpha, \beta, r$	Lame's coefficient.
$\rho$	Mass density per unit mass.
$\nu$	Poisson's ratio.
$\Omega$	Natural frequency.
$N_x, N_y, N_{xy}, N_{yx}$	Normal stress resultants in unit of forces per unit length due to initial stress.
$M_x, M_y, M_{xy}, M_{yx}$	Bending stress couples in unit of moment per unit length.
$V_x, V_y$	Shear stress components in the direction of x and y respectively.

## NONDIMENSIONAL SYMBOLIC FORMS:

$$\hat{a} = a/h, \\ \hat{y} = y/h,$$

$$\hat{b} = b/h, \\ \hat{t} = t/t_1,$$

$$\hat{x} = x/h, \\ t_1 = \sqrt{2h/g},$$

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$$\hat{G} = \frac{5Gh^3}{GD}$$

$$\hat{m} = \frac{9h^5}{Dt_1^2}$$

$$\hat{I} = \frac{\hat{m}}{12} = \frac{9h^5}{12Dt_1^2}$$

$$\hat{N}_x = -\frac{N_x h^2}{D}$$

$$\hat{N}_y = -\frac{N_y h^2}{D}$$

$$\hat{N}_{xy} = -\frac{N_{xy} h^2}{D}$$

$$\hat{\omega} = \omega / h$$

$$\hat{\nabla}^2 = \frac{\partial^2}{\partial x^2} + \frac{\partial^2}{\partial y^2}$$

$$\hat{\Omega} = \Omega t_1$$

$$\hat{\beta} = \frac{m^2 \cdot \frac{b}{a} \cdot \hat{N}_x + n^2 \cdot \frac{a}{b} \cdot \hat{N}_y}{(\hat{N}_{B.A.S.})_{CR}}$$

$$\hat{\alpha} = \frac{m^2 \cdot \frac{b}{a} \cdot \hat{N}_x + n^2 \cdot \frac{a}{b} \cdot \hat{N}_y}{(\hat{N}_{B.A.})_{CR}}$$

$$(\hat{N}_{B.S.A.})_{CR} = \left(\frac{b^2}{\pi^2 D}\right) N_x$$

$$\hat{\beta} = \frac{N_x \left(\frac{b^2}{\pi^2 D}\right)}{(\hat{N}_{B.S.A.})_{CR}}$$

$$\hat{\alpha} = \frac{N_x \left(\frac{b^2}{\pi^2 D}\right)}{(\hat{N}_{B.A.})_{CR}}$$

$$\hat{\Omega}_{(B.S.A.T.I.)}^2 = \frac{\Omega_{(B.S.A.T.I.)}^2}{\pi^4 / \left(\frac{a^2 b^2 9h}{D}\right)}$$

$$\hat{\Omega}^2 = \frac{\hat{\Omega}_{(B.S.A.T.I.)}^2}{\hat{\Omega}_{(B.A.T.I.)}^2}$$

$$\hat{\Omega}_{(B.A.T.I.)}^2 = \frac{\Omega_{(B.A.T.I.)}^2}{\pi^4 / \left(\frac{a^2 b^2 9h}{D}\right)}$$

$$\hat{\Omega}_{(B.S.A.T.I.R.I.)}^2 = \frac{\Omega_{(B.S.A.T.I.R.I.)}^2}{\pi^4 / \left(\frac{a^2 b^2 9h}{D}\right)}$$

$$\hat{N} = \frac{(\hat{N}_{B.S.A.})_{CR}}{(\hat{N}_{B.A.})_{CR}}$$

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## CHAPTER I

## INTRODUCTION

Timoshenko and Gere<sup>(3)</sup> presented the static stability of rectangular plates using the energy method, subjected to transverse bending stress and axial forces. The rectangular plates with two parallel sides simply supported and the other two parallel sides subjected to different boundary conditions are analyzed for the critical buckling-loads.

Wu, C.I. and Vinson<sup>(5)</sup> have derived the sets of equations for the study of free vibrations of rectangular plates composed of transversely isotropic material, including the effect of transverse shear deformation and rotary inertia. The special case of a simply supported rectangular plates is studied in details. Different values of parameter  $E/G$ , the ratio of in plane modulus of elasticity to transverse shear modulus, are considered for transversely isotropic material.

Brunelle<sup>(2)</sup> derived the equations of motion that include the effect of transverse isotropy, initial stress and initial displacement. These plate equations are suitable for investigating free and forced vibrations, wave propagation and the static stability problem. However, the equations are specialized so that only static stability of a rectangular plates with two parallel sides simply supported and the remaining two sides subjected to a variety of boundary conditions is treated in detail. The effect of transverse isotropy is considered

by the parameters  $S$ , (ie  $S = E/G (h/b)^2 / (1 - \nu^2)$  where  $E/G$  is the ratio of in plane modulus of elasticity to transverse shear modulus,  $h$  is the thickness of plate,  $b$  is the width dimension of plate and  $\nu$  is the Poisson's ratio. Considering different values of the parameter  $S$ ,  $0 < S < 0.1$ , the critical buckling loads for the plate subjected to different boundary conditions for the other two parallel sides are studied in detail.

Archer<sup>(1)</sup> derived the nonlinear shear deformation theory for thin elastic shells, including the effect of shear deformation and transverse and rotary inertia. He presented the fifteen equations for the ten stress resultants and the five displacement functions in orthogonal curvilinear co-ordinates. The equations of motions, specialized for a thin rectangular plate by noting  $A=B=1$  and  $1/\sqrt{r_1} = 1/\sqrt{r_2} = 0$ , are utilized in this thesis.

The objective of this thesis is to study the dynamic stability of rectangular plates using the equations of motion derived from the reference (1). The natural frequency and the critical buckling-load equations are derived for the rectangular plate simply supported along two parallel sides and the other two parallel sides subjected to different boundary conditions.

From the resulting equations the dynamic stability of a rectangular plate simply supported on all four edges is studied in detail. Setting the natural frequency equal to zero the static critical buckling-load is determined. The effect of transverse shear stress is considered by using the parameter  $h^2/ab$ , which is related directly to the plate geometry namely the thickness and the length/width dimensions,

for the study of natural frequencies and the critical buckling-loads. The range of this parameter is taken as  $0.01 < h^2/ab < 0.05$ , which is considered sufficient for the theory to be valid (2).

#### STATEMENTS OF MOTION

The equations of motion of thin, isotropic, elastic plates subjected to initial stress conditions, as presented in reference (1) are as follows:

$$D \nabla^4 w + \frac{1}{2} \sigma_x \frac{\partial^2 w}{\partial x^2} + \frac{1}{2} \sigma_y \frac{\partial^2 w}{\partial y^2} - \rho h \ddot{w} = 0 \quad (1)$$

$$D \nabla^4 \phi + \frac{1}{2} \sigma_x \frac{\partial^2 \phi}{\partial x^2} + \frac{1}{2} \sigma_y \frac{\partial^2 \phi}{\partial y^2} - \rho h \ddot{\phi} = 0 \quad (2)$$

and

$$- \frac{55 \rho h \dot{w}}{2} - \frac{50 \rho h \dot{\phi}}{6} - \frac{50 \rho h \nabla^2 w}{6} - N_x \frac{\partial^2 w}{\partial x^2} - 2 N_{xy} \frac{\partial^2 w}{\partial x \partial y} - N_y \frac{\partial^2 w}{\partial y^2}$$

$$+ \rho h \frac{\partial^2 w}{\partial t^2} - \rho c(x, y, t) = 0 \quad (3)$$

The definitions for moment, axial and shear forces for the rectangular plate are as follows:

$$M_x = D \left( \frac{\partial^2 w}{\partial x^2} + \nu \frac{\partial^2 w}{\partial y^2} \right), \quad M_y = D \left( \frac{\partial^2 w}{\partial y^2} + \nu \frac{\partial^2 w}{\partial x^2} \right),$$

$$M_{xy} = M_{yx} = D \left( \frac{1-\nu}{2} \right) \left( \frac{\partial^2 w}{\partial x \partial y} + \frac{\partial^2 w}{\partial y \partial x} \right),$$

$$N_x = \frac{Eh}{(1-\nu^2)} \left[ \left( \frac{\partial v}{\partial x} + \frac{1}{2} \frac{\partial^2 w}{\partial x^2} \right) + \nu \left( \frac{\partial v}{\partial y} + \frac{1}{2} \frac{\partial^2 w}{\partial y^2} \right) \right], \quad (4)$$

$$N_y = \frac{Eh}{(1-\nu^2)} \left[ \left( \frac{\partial v}{\partial y} + \frac{1}{2} \frac{\partial^2 w}{\partial y^2} \right) + \nu \left( \frac{\partial v}{\partial x} + \frac{1}{2} \frac{\partial^2 w}{\partial x^2} \right) \right],$$

## CHAPTER II

## EQUATIONS OF MOTION

The equations of motion of thin, isotropic, elastic plates subjected to initial stress conditions, as presented in reference (1) are as follows:

$$\frac{D(1-\nu)}{2} \nabla^2 \phi + \frac{D(1+\nu)}{2} \frac{\partial^2 \phi}{\partial x^2} - \frac{5Gh}{6} \phi - \frac{9h^3}{12} \frac{\partial^2 \phi}{\partial t^2} + \frac{D(1+\nu)}{2} \frac{\partial^2 \psi}{\partial x \partial y} - \frac{5Gh}{6} \frac{\partial \omega}{\partial x} = 0 \quad \dots (1)$$

$$\frac{D(1+\nu)}{2} \frac{\partial^2 \phi}{\partial x \partial y} + \frac{D(1-\nu)}{2} \nabla^2 \psi + \frac{D(1+\nu)}{2} \frac{\partial^2 \psi}{\partial y^2} - \frac{5Gh}{6} \psi - \frac{9h^3}{12} \frac{\partial^2 \psi}{\partial t^2} - \frac{5Gh}{6} \frac{\partial \omega}{\partial y} = 0 \quad \dots (2)$$

and

$$-\frac{5Gh}{6} \frac{\partial \phi}{\partial x} - \frac{5Gh}{6} \frac{\partial \psi}{\partial y} - \frac{5Gh}{6} \nabla^2 \omega - N_x \frac{\partial^2 \omega}{\partial x^2} - 2N_{xy} \frac{\partial^2 \omega}{\partial x \partial y} - N_y \frac{\partial^2 \omega}{\partial y^2}$$

$$+ 9h \frac{\partial^2 \omega}{\partial t^2} - q(x, y, t) = 0 \quad \dots (3)$$

The definitions for moment, axial and shear forces for the rectangular plate are as follows:

$$M_x = D \left( \frac{\partial \phi}{\partial x} + \nu \frac{\partial \psi}{\partial y} \right), \quad M_y = D \left( \frac{\partial \psi}{\partial y} + \nu \frac{\partial \phi}{\partial x} \right),$$

$$M_{xy} = M_{yx} = D \left( \frac{1-\nu}{2} \right) \left( \frac{\partial \psi}{\partial x} + \frac{\partial \phi}{\partial y} \right),$$

$$N_x = \frac{Eh}{(1-\nu^2)} \left[ \left( \frac{\partial \psi}{\partial x} + \frac{1}{2} \frac{\partial^2 \omega}{\partial x^2} \right) + \nu \left( \frac{\partial \psi}{\partial y} + \frac{1}{2} \frac{\partial^2 \omega}{\partial y^2} \right) \right], \quad \dots (4)$$

$$N_y = \frac{Eh}{(1-\nu^2)} \left[ \left( \frac{\partial \psi}{\partial y} + \frac{1}{2} \frac{\partial^2 \omega}{\partial y^2} \right) + \nu \left( \frac{\partial \psi}{\partial x} + \frac{1}{2} \frac{\partial^2 \omega}{\partial x^2} \right) \right],$$

$$N_{xy} = N_{yx} = Gh \left[ \frac{\partial v}{\partial x} + \frac{\partial u}{\partial y} + \frac{\partial w}{\partial x} \frac{\partial w}{\partial y} \right],$$

$$V_x = \frac{5Gh}{6} \left( \frac{\partial w}{\partial x} + \phi \right), \text{ and } V_y = \frac{5Gh}{6} \left( \frac{\partial w}{\partial y} + \psi \right).$$

Boundary conditions for the rectangular plate at edges  $y=0$  and  $y=b$  are as follows:

- (a) Simply supported at  $y=0$  and  $y=b$ .

$$W(y)=0, \phi(y)=0, \text{ and } M_y(y)=0. \text{ ---- (5)}$$

- (b) Fixed at  $y=0$  and  $y=b$ .

$$W(y)=0, \phi(y)=0, \text{ and } \Psi(y)=0. \text{ ---- (6)}$$

- (c) Free at  $y=0$  and  $y=b$ .

$$M_y(y)=0, M_{yx}(y)=0, \text{ and } V_y(y)=0. \text{ ---- (7)}$$

For the free vibration problem  $q(x,y,t)=0$ ; upon rearranging equations (1)-(3), the following matrix form is obtained:

$$[A] \begin{Bmatrix} \phi(x,y,t) \\ \Psi(x,y,t) \\ w(x,y,t) \end{Bmatrix} = \begin{Bmatrix} 0 \\ 0 \\ 0 \end{Bmatrix}, \text{ ---- (8)}$$

Where the elements of the symmetric operator matrix  $[A]$ , defined as  $a_{ij}$ , are given by the equations

$$\left. \begin{aligned} a_{11} &= \frac{D(1-\nu)}{2} \nabla^2 + \frac{D(1+\nu)}{2} \frac{\partial^2}{\partial x^2} - \frac{5Gh}{6} - \frac{\rho h^3}{12} \frac{\partial^2}{\partial t^2}, \\ a_{12} &= a_{21} = \frac{D(1+\nu)}{2} \frac{\partial^2}{\partial x \partial y}, \\ a_{13} &= a_{31} = -\frac{5Gh}{6} \frac{\partial}{\partial x}, \\ a_{22} &= \frac{D(1-\nu)}{2} \nabla^2 + \frac{D(1+\nu)}{2} \frac{\partial^2}{\partial y^2} - \frac{5Gh}{6} - \frac{\rho h^3}{12} \frac{\partial^2}{\partial t^2}, \\ a_{23} &= a_{32} = -\frac{5Gh}{6} \frac{\partial}{\partial y}, \text{ and} \\ a_{33} &= -\frac{5Gh}{6} \nabla^2 - N_x \frac{\partial^2}{\partial x^2} - 2N_{xy} \frac{\partial^2}{\partial x \partial y} - N_y \frac{\partial^2}{\partial y^2} + \rho h \frac{\partial^2}{\partial t^2}. \end{aligned} \right\} \text{ (9)}$$

The uncoupled operator for  $w(x,y,t)$ ,  $\phi(x,y,t)$  and  $\psi(x,y,t)$  is obtained by successive differentiation and combination of the results of equations (1)-(3). The uncoupled operator for  $w(x,y,t)$ ,  $\phi(x,y,t)$  and  $\psi(x,y,t)$  is given by the determinant of the operator matrix [A] as

$$\left[ \mathcal{D}\nabla^4 + \left\{ (N_x \frac{\partial^2}{\partial x^2} + 2N_{xy} \frac{\partial^2}{\partial x \partial y} + N_y \frac{\partial^2}{\partial y^2}) - \rho h \frac{\partial^2}{\partial t^2} \right\} \right. \\ \left. \left\{ \frac{\mathcal{D}\nabla^2}{5GhI_G} - 1 - \frac{\rho h^3}{12} \cdot \frac{1}{5GhI_G} \frac{\partial^2}{\partial t^2} \right\} - \frac{\rho h^3}{12} \nabla^2 \frac{\partial^2}{\partial t^2} \right] \\ \left[ \frac{\mathcal{D}(1-\mathcal{D})}{2} \cdot \frac{1}{5GhI_G} \nabla^2 - 1 - \frac{\rho h^3}{12} \cdot \frac{1}{5GhI_G} \cdot \frac{\partial^2}{\partial t^2} \right] (\ ) = 0 \quad \text{---(10)}$$

Neglecting the effect of shear and rotary inertia, equation (10) reduces to the classical 4th order equation of motion including the effect of bending, axial forces and transverse inertia.

The uncoupled operator for  $w(x,y,t)$  is subsequently to reduce to a 4th order operator rather than the 6th order operator as shown in the equation (10).

Differentiating equation (1) with respect to  $x$ , equation (2) with respect to  $y$ , adding the results, and introducing the function  $\Phi \equiv \frac{\partial \phi}{\partial x} + \frac{\partial \psi}{\partial y}$ , yields

$$\mathcal{D}\nabla^2 \Phi - \frac{5Gh}{6} \Phi - \frac{\rho h^3}{12} \frac{\partial^2 \Phi}{\partial t^2} - \frac{5Gh}{6} \nabla^2 w = 0 \quad \text{---(11)}$$

substituting  $\Phi \equiv \frac{\partial \phi}{\partial x} + \frac{\partial \psi}{\partial y}$  into equation (3), one obtains

$$-\frac{5Gh}{6} \Phi - \frac{5Gh}{6} \nabla^2 w - N_x \frac{\partial^2 w}{\partial x^2} - 2N_{xy} \frac{\partial^2 w}{\partial x \partial y} - N_y \frac{\partial^2 w}{\partial y^2} + \rho h \frac{\partial^2 w}{\partial t^2} \\ - q(x,y,t) = 0 \quad \text{(12)}$$

For the free vibration problem  $q(x,y,t)=0$ ; upon rearranging equations (11) and (12) the following matrix form is obtained:

$$[B] \begin{Bmatrix} w(x,y,t) \\ \Phi(x,y,t) \end{Bmatrix} = \begin{Bmatrix} 0 \\ 0 \end{Bmatrix}, \quad \text{---(13)}$$

where the elements of the nonsymmetric operator matrix [B], defined as  $b_{ij}$ , are given by the equations

$$\left. \begin{aligned} b_{11} &= -\frac{5Gh}{6} \nabla^2 - N_x \frac{\partial^2}{\partial x^2} - 2N_{xy} \frac{\partial^2}{\partial x \partial y} - N_y \frac{\partial^2}{\partial y^2} + \rho h \frac{\partial^2}{\partial t^2} \\ b_{12} &= -\frac{5Gh}{6} , \quad b_{21} = -\frac{5Gh}{6} \nabla^2 , \quad \text{and} \\ b_{22} &= \nabla^2 - \frac{5Gh}{6} - \frac{\rho h^3}{12} \frac{\partial^2}{\partial t^2} . \end{aligned} \right\} \text{---(14)}$$

The uncoupled operator for  $w(x,y,t)$  [and for the function  $\Phi(x,y,t)$ ] is given by the determinant of operator matrix [B]. The uncoupled equation for  $w(x,y,t)$  is expressed in the following 4th order operator form:

$$\left[ D \nabla^4 + \left\{ (N_x \frac{\partial^2}{\partial x^2} + 2N_{xy} \frac{\partial^2}{\partial x \partial y} + N_y \frac{\partial^2}{\partial y^2}) - \rho h \frac{\partial^2}{\partial t^2} \right\} \right. \\ \left. \left[ \frac{D \nabla^2}{5Gh/6} - 1 - \frac{\rho h^3}{12} \frac{1}{5Gh/6} \frac{\partial^2}{\partial t^2} \right] - \frac{\rho h^3}{12} \nabla^2 \frac{\partial^2}{\partial t^2} \right] [w(x,y,t)] = 0 \text{---(15)}$$

Upon defining the following terms,

$$\left. \begin{aligned} \frac{5Gh^3}{6D} &= \hat{G} , \quad \frac{\rho h^5}{D t_1^2} = \hat{m} , \quad \frac{\rho h^5}{12 D t_1^2} = \frac{\hat{m}}{12} = \hat{i} , \quad -\frac{N_x h^2}{D} = \hat{N}_x , \\ -\frac{N_y h^2}{D} &= \hat{N}_y , \quad -\frac{N_{xy} h^2}{D} = \hat{N}_{xy} , \quad \frac{x}{h} = \hat{x} , \quad \frac{y}{h} = \hat{y} , \quad \hat{\nabla}^2 = \frac{\partial^2}{\partial \hat{x}^2} + \frac{\partial^2}{\partial \hat{y}^2} \\ \frac{t}{t_1} &= \hat{t} , \quad t_1 = \sqrt{\frac{2h}{g}} , \quad \text{and} \quad \frac{\omega}{h} = \hat{\omega} . \end{aligned} \right\} \text{---(16)}$$

Equation (10) reduces to the following nondimensional form:

$$\left\{ \left[ \hat{\nabla}^2 - \left( \frac{\hat{N}_x}{\hat{G}} \frac{\partial^2}{\partial \hat{x}^2} + \frac{2\hat{N}_{xy}}{\hat{G}} \frac{\partial^2}{\partial \hat{x} \partial \hat{y}} + \frac{\hat{N}_y}{\hat{G}} \frac{\partial^2}{\partial \hat{y}^2} \right) - \frac{\hat{m}}{\hat{G}} \frac{\partial^2}{\partial \hat{t}^2} \right] \right. \\ \left. \left[ \hat{\nabla}^2 - \hat{G} - \hat{i} \frac{\partial^2}{\partial \hat{t}^2} \right] + \hat{G} \hat{\nabla}^2 \right\} \left\{ \frac{(1-\nu)}{2} \hat{\nabla}^2 - \hat{G} - \hat{i} \frac{\partial^2}{\partial \hat{t}^2} \right\} \\ \left[ \Phi(\hat{x}, \hat{y}, \hat{t}) , \Psi(\hat{x}, \hat{y}, \hat{t}) \right] = 0 . \quad \text{---(17)}$$



Similarly equation (15) becomes

$$\left\{ \left[ \hat{\nabla}^2 - \left( \frac{\hat{N}_x}{\hat{C}} \frac{\partial^2}{\partial \hat{x}^2} + 2 \frac{\hat{N}_{xy}}{\hat{C}} \frac{\partial^2}{\partial \hat{x} \partial \hat{y}} + \frac{\hat{N}_y}{\hat{C}} \frac{\partial^2}{\partial \hat{y}^2} \right) - \frac{\hat{m}}{\hat{C}} \frac{\partial^2}{\partial \hat{t}^2} \right] \right. \\ \left. \left[ \hat{\nabla}^2 - \hat{G} - \hat{I} \frac{\partial^2}{\partial \hat{t}^2} \right] + \hat{G} \hat{\nabla}^2 \right\} [\hat{W}(\hat{x}, \hat{y}, \hat{t})] = 0. \quad \text{---(18)}$$

### 3.1 Free Vibration Of A Rectangular Plate Simply Supported Along Two Parallel Sides:

Using the method of Bernoulli Separation of Variables, and proceeding in the manner as presented by Levy<sup>(4)</sup>, the following two-dimensional displacement and rotations conditions are assumed:

$$\left. \begin{aligned} \hat{w}(\hat{x}, \hat{y}, \hat{t}) &= \hat{W}(\hat{y}) \sin \frac{m\pi \hat{x}}{\hat{a}} e^{i\hat{\lambda} \hat{t}} \\ \hat{\phi}(\hat{x}, \hat{y}, \hat{t}) &= \hat{\phi}(\hat{y}) \cos \frac{m\pi \hat{x}}{\hat{a}} e^{i\hat{\lambda} \hat{t}}, \text{ and} \\ \hat{\psi}(\hat{x}, \hat{y}, \hat{t}) &= \hat{\psi}(\hat{y}) \sin \frac{m\pi \hat{x}}{\hat{a}} e^{i\hat{\lambda} \hat{t}} \end{aligned} \right\} \text{---(19)}$$

where,  $\hat{\lambda} = \omega \hat{t}$ ,  $\hat{a} = \frac{a}{h}$ , and ----- (20)

$m$  is the number of half waves in the  $x$  direction. The assumed solutions hold for the special case of simply supported boundary conditions on the edges  $\hat{x}=0$ , and  $\hat{x}=\frac{\hat{a}}{h}$ . The boundary conditions on the edges are arbitrary.

Substituting equation (19) into equations (17) and (18) with  $\hat{x}=\frac{\hat{a}}{h}$ , yields, respectively

$$[A_4 \frac{d^4}{d\hat{y}^4} + A_2 \frac{d^2}{d\hat{y}^2} + A_0][C_2 \frac{d^2}{d\hat{y}^2} + C_0][\hat{\phi}(\hat{y}), \hat{\psi}(\hat{y})] = 0; \quad \text{(21)}$$

and

$$[A_4 \frac{d^4}{d\hat{y}^4} + A_2 \frac{d^2}{d\hat{y}^2} + A_0][\hat{W}(\hat{y})] = 0, \quad \text{---(22)}$$

## CHAPTER III

## ANALYSIS

### 3.1 Free Vibration Of A Rectangular Plate Simply Supported Along Two Parallel Sides:

Using the method of Bernoulli Separation of Variables, and proceeding in the manner as presented by Levy<sup>(4)</sup>, the following non-dimensional displacement and rotations conditions are assumed:

$$\left. \begin{aligned} \hat{w}(\hat{x}, \hat{y}, \hat{t}) &= \hat{W}(\hat{y}) \sin \frac{m\pi \hat{x}}{\hat{a}} e^{i\hat{\Omega} \hat{t}}, \\ \phi(\hat{x}, \hat{y}, \hat{t}) &= \phi(\hat{y}) \cos \frac{m\pi \hat{x}}{\hat{a}} e^{i\hat{\Omega} \hat{t}}, \text{ and} \\ \psi(\hat{x}, \hat{y}, \hat{t}) &= \psi(\hat{y}) \sin \frac{m\pi \hat{x}}{\hat{a}} e^{i\hat{\Omega} \hat{t}}, \end{aligned} \right\} \text{---(19)}$$

where,  $\hat{\Omega} = \Omega t_1$ ,  $\hat{a} = \frac{a}{h}$ , and -----(20)

$m$  is the number of half waves in the  $x$  direction. The assumed solutions hold for the special case of simply supported boundary conditions on the edges  $\hat{x}=0$ , and  $\hat{x}=\hat{a}$ . The boundary conditions on the edges are arbitrary.

Substituting equation (19) into equations (17) and (18) with  $\hat{N}_{xy}=0$ , yields, respectively

$$\left[ A_4 \frac{d^4}{d\hat{y}^4} + A_2 \frac{d^2}{d\hat{y}^2} + A_0 \right] \left[ C_2 \frac{d^2}{d\hat{y}^2} + C_0 \right] \{ \phi(\hat{y}), \psi(\hat{y}) \} = 0, \text{---(21)}$$

and

$$\left[ A_4 \frac{d^4}{d\hat{y}^4} + A_2 \frac{d^2}{d\hat{y}^2} + A_0 \right] \{ \hat{W}(\hat{y}) \} = 0, \text{----(22)}$$

where

$$A_4 = 1 - \frac{\hat{N}_y}{\hat{G}},$$

$$A_2 = -\left\{ \left( \frac{m\pi}{\hat{a}} \right)^2 + \hat{G} - \hat{i}\hat{\Omega}^2 \right\} \left\{ 1 - \frac{\hat{N}_y}{\hat{G}} \right\} - \left( \frac{m\pi}{\hat{a}} \right)^2 \left( 1 - \frac{\hat{N}_x}{\hat{G}} \right) + \frac{\hat{m}}{\hat{G}} \hat{\Omega}^2,$$

$$A_0 = \left\{ \left( \frac{m\pi}{\hat{a}} \right)^4 + \left( \frac{m\pi}{\hat{a}} \right)^2 \hat{G} - \left( \frac{m\pi}{\hat{a}} \right)^2 \hat{i}\hat{\Omega}^2 \right\} \left\{ 1 - \frac{\hat{N}_x}{\hat{G}} \right\} + \frac{\hat{i}\hat{m}}{\hat{G}} \hat{\Omega}^4 - \left\{ \left( \frac{m\pi}{\hat{a}} \right)^2 \frac{\hat{m}}{\hat{G}} + \hat{m} \right\} \hat{\Omega}^2 - \left( \frac{m\pi}{\hat{a}} \right)^2 \hat{G}, \text{ and}$$

$$C_2 = (1-\nu)/2, \text{ and}$$

$$C_0 = -\left( \frac{1-\nu}{2} \right) \left( \frac{m\pi}{\hat{a}} \right)^2 - \hat{G} + \hat{i}\hat{\Omega}^2.$$

Since equations (21) and (22) are linear differential equations with constant coefficients, the solution is assumed in the form:

$$\Phi(\hat{y}) = P_1 e^{\lambda \hat{y}},$$

$$\Psi(\hat{y}) = P_2 e^{\lambda \hat{y}}, \text{ and}$$

$$\hat{W}(\hat{y}) = P_3 e^{\lambda \hat{y}}.$$

The roots of the biquadratic factor in equation (21) and (22) are

$$\lambda_{1,2} = \pm \alpha, \text{ and}$$

$$\lambda_{3,4} = \pm i\beta.$$

The roots of the quadratic factor in equation (21) are

$$\lambda_{1,2} = \pm \tau,$$

where  $\alpha$ ,  $\beta$  and  $\tau$  are real quantities.

It follows that,

$$\begin{aligned}
 \alpha^2 &= \frac{-A_2}{2A_4} + \left[ \left( \frac{A_2}{2A_4} \right)^2 - \frac{A_0}{A_4} \right]^{1/2}, \\
 \beta^2 &= \frac{A_2}{2A_4} + \left[ \left( \frac{A_2}{2A_4} \right)^2 - \frac{A_0}{A_4} \right]^{1/2}, \text{ and} \\
 \gamma^2 &= -\frac{C_0}{C_2}.
 \end{aligned}
 \tag{28}$$

Hence, the solutions of the functions of  $\hat{W}, \phi, \psi$  become, respectively,

$$\begin{aligned}
 \hat{W}(\hat{y}) &= C_1 \text{Sinh } \alpha \hat{y} + C_2 \text{Cosh } \alpha \hat{y} + C_3 \text{Sin } \beta \hat{y} + C_4 \text{Cos } \beta \hat{y}, \\
 \phi(\hat{y}) &= C_5 \text{Sinh } \alpha \hat{y} + C_6 \text{Cosh } \alpha \hat{y} + C_7 \text{Sin } \beta \hat{y} + C_8 \text{Cos } \beta \hat{y} \\
 &\quad + C_9 \text{Sinh } \gamma \hat{y} + C_{10} \text{Cosh } \gamma \hat{y}, \text{ and} \\
 \psi(\hat{y}) &= C_{11} \text{Sinh } \alpha \hat{y} + C_{12} \text{Cosh } \alpha \hat{y} + C_{13} \text{Sin } \beta \hat{y} \\
 &\quad + C_{14} \text{Cos } \beta \hat{y} + C_{15} \text{Sinh } \gamma \hat{y} + C_{16} \text{Cosh } \gamma \hat{y},
 \end{aligned}
 \tag{29}$$

where constants  $C_1$  through  $C_{16}$  are constants of integration.

The above solutions in equation (29) also must satisfy the two parent equations (1) and (2). Thus, the following relationships among the sixteen constants are obtained:

$$\begin{aligned}
 C_5 &= \delta_1 \left( \frac{m\pi}{\alpha} \right) C_1, & C_6 &= \delta_1 \left( \frac{m\pi}{\alpha} \right) C_2, \\
 C_7 &= -\delta_2 \left( \frac{m\pi}{\alpha} \right) C_3, & C_8 &= \delta_2 \left( \frac{m\pi}{\alpha} \right) C_4, \\
 C_{11} &= \delta_1 \alpha C_1, & C_{12} &= \delta_1 \alpha C_2, \\
 C_{13} &= -\delta_2 \beta C_3, & C_{14} &= \delta_2 \beta C_4,
 \end{aligned}
 \tag{30}$$

$$C_{15} = \left( \frac{m\pi}{\hat{a}} \right) \frac{1}{\gamma} C_{10}, \quad \text{and}$$

$$C_{16} = \left( \frac{m\pi}{\hat{a}} \right) \frac{1}{\gamma} C_9,$$

$$\left. \begin{aligned} \text{where } \delta_1 &= \hat{G} / [h^2 \alpha^2 + \hat{i} \hat{\Omega}^2 - \hat{G} - (m\pi/\hat{a})^2], \\ \text{and } \delta_2 &= \hat{G} / [h^2 \beta^2 + \hat{G} + (m\pi/\hat{a})^2 - \hat{i} \hat{\Omega}^2]. \end{aligned} \right] \quad \text{---(31)}$$

The relationships reduce the number of constants from sixteen to six.

Substituting equation (30) into equation (29), one obtains

$$\begin{Bmatrix} \hat{W} \\ \Phi \\ \Psi \end{Bmatrix} = [C] \begin{Bmatrix} C_1 \\ C_2 \\ C_3 \\ C_4 \\ C_9 \\ C_{10} \end{Bmatrix}, \quad \text{---(32)}$$

where the elements of matrix [C], are given by the equation

$$[C] = \begin{bmatrix}
 \text{Sinh } \alpha y & \text{Cosh } \alpha y & \text{Sin } \beta y & \text{Cos } \beta y & 0 & 0 \\
 \delta_1 (\pi \pi / \hat{\alpha}) \cdot \text{Sinh } \alpha y & \delta_1 (\pi \pi / \hat{\alpha}) \cdot \text{Cosh } \alpha y & -\delta_2 (\pi \pi / \hat{\alpha}) \cdot \text{Sin } \beta y & -\delta_2 (\pi \pi / \hat{\alpha}) \cdot \text{Cos } \beta y & \text{Sinh } \alpha y & \text{Cosh } \alpha y \\
 \delta_1 \alpha \text{cosh } \alpha y & \delta_1 \alpha \text{sinh } \alpha y - \delta_2 \beta \cdot \text{cos } \beta y & \delta_2 \beta \cdot \text{sin } \beta y & \text{sin } \beta y & (\pi \pi / \hat{\alpha}) \cdot \frac{1}{y} & (\pi \pi / \hat{\alpha}) \cdot \frac{1}{y}
 \end{bmatrix} \quad \text{---(33)}$$

Equation (32) is used, along with three appropriate boundary conditions on each edge  $\hat{y}=0$  and  $\hat{y}=\hat{b}=\frac{b}{h}$  to solve the free vibration problem of a biaxially compressed plate subjected to transverse shear stress, bending stress, and transverse and rotary inertia.

and using equation (16) the following nondimensional form of equations of motion are obtained:

$$\left(\frac{1-\nu}{2}\right) \hat{\nabla}^2 \phi + \left(\frac{1-\nu}{2}\right) \frac{\partial^2 \phi}{\partial \hat{x}^2} - \hat{G} \phi - i \frac{\partial^2 \phi}{\partial \hat{t}^2} + \left(\frac{1+\nu}{2}\right) \frac{\partial^2 \psi}{\partial \hat{x} \partial \hat{y}} - \hat{G} \frac{\partial \phi}{\partial \hat{x}} = 0, \quad (20)$$

$$\left(\frac{1+\nu}{2}\right) \frac{\partial^2 \phi}{\partial \hat{x} \partial \hat{y}} + \left(\frac{1-\nu}{2}\right) \hat{\nabla}^2 \psi + \left(\frac{1+\nu}{2}\right) \frac{\partial^2 \psi}{\partial \hat{y}^2} - \hat{G} \psi - i \frac{\partial^2 \psi}{\partial \hat{t}^2} - \hat{G} \frac{\partial \psi}{\partial \hat{y}} = 0, \quad \text{and} \quad (21)$$

$$-\hat{G} \frac{\partial \phi}{\partial \hat{x}} - \hat{G} \frac{\partial \psi}{\partial \hat{y}} - \hat{G} \hat{\nabla}^2 \hat{w} + \hat{N}_x \frac{\partial^2 \hat{w}}{\partial \hat{x}^2} + 2\hat{N}_{xy} \frac{\partial^2 \hat{w}}{\partial \hat{x} \partial \hat{y}} + \hat{N}_y \frac{\partial^2 \hat{w}}{\partial \hat{y}^2} + m \frac{\partial^2 \hat{w}}{\partial \hat{t}^2} - \frac{h^3}{D} q(x, y, t) = 0. \quad (22)$$

The dynamic stability equations for the free vibration of a simply supported plate at edges  $\hat{x}=0$  and  $\hat{x}=\hat{a}=\frac{a}{h}$ , with  $\hat{N}_x$  and  $\hat{N}_y$  acting, are obtained by assuming the nondimensional displacement and rotations given by the form:

$$\left. \begin{aligned} \phi &= P_1 e^{\lambda \hat{y}} \cos\left(\frac{m\pi \hat{x}}{a}\right) e^{i\hat{\omega} \hat{t}}, \\ \psi &= P_2 e^{\lambda \hat{y}} \sin\left(\frac{m\pi \hat{x}}{a}\right) e^{i\hat{\omega} \hat{t}}, \quad \text{and} \\ \hat{w} &= P_3 e^{\lambda \hat{y}} \sin\left(\frac{m\pi \hat{x}}{a}\right) e^{i\hat{\omega} \hat{t}} \end{aligned} \right\} \quad (23)$$

where  $m$  is the number of half waves in the  $x$  direction. The parameter  $\lambda$  is obtained for the given plate by the form of the appropriate boundary conditions on the sides  $\hat{y}=0$ , and  $\hat{y}=\hat{b}=\frac{b}{h}$ .

### 3.2 Dynamics Stability Of A Rectangular Plate Simply Supported Along Two Parallel Sides:

Multiplying equations (1) and (2) by  $h^2/D$ , equation (3) by  $h^3/D$  and using equation (16) the following nondimensional form of equations of motion are obtained:

$$\left(\frac{1-\nu}{2}\right) \hat{\nabla}^2 \phi + \left(\frac{1-\nu}{2}\right) \frac{\partial^2 \phi}{\partial \hat{x}^2} - \hat{G} \phi - \hat{i} \frac{\partial^2 \phi}{\partial \hat{t}^2} + \left(\frac{1+\nu}{2}\right) \frac{\partial^2 \psi}{\partial \hat{x} \partial \hat{y}} - \hat{G} \frac{\partial \hat{w}}{\partial \hat{x}} = 0, \quad (34)$$

$$\begin{aligned} &\left(\frac{1+\nu}{2}\right) \frac{\partial^2 \phi}{\partial \hat{x} \partial \hat{y}} + \left(\frac{1-\nu}{2}\right) \hat{\nabla}^2 \psi + \left(\frac{1+\nu}{2}\right) \frac{\partial^2 \psi}{\partial \hat{y}^2} - \hat{G} \psi - \hat{i} \frac{\partial^2 \psi}{\partial \hat{t}^2} \\ &- \hat{G} \frac{\partial \hat{w}}{\partial \hat{y}} = 0, \quad \text{and} \quad \text{-----} \quad (35) \end{aligned}$$

$$\begin{aligned} &-\hat{G} \frac{\partial \phi}{\partial \hat{x}} - \hat{G} \frac{\partial \psi}{\partial \hat{y}} - \hat{G} \hat{\nabla}^2 \hat{w} + \hat{N}_x \frac{\partial^2 \hat{w}}{\partial \hat{x}^2} + 2\hat{N}_{xy} \frac{\partial^2 \hat{w}}{\partial \hat{x} \partial \hat{y}} + \hat{N}_y \frac{\partial^2 \hat{w}}{\partial \hat{y}^2} \\ &+ \hat{m} \frac{\partial^2 \hat{w}}{\partial \hat{t}^2} - \frac{h^3}{D} q(x, y, t) = 0. \quad \text{-----} \quad (36) \end{aligned}$$

The dynamic stability equations for the free vibration of a simply supported plate at edges  $\hat{x}=0$  and  $\hat{x}=\hat{a}=\frac{\hat{a}}{h}$ , with  $\hat{N}_x$  and  $\hat{N}_y$  acting, are obtained by assuming the nondimensional displacement and rotations given in the form:

$$\left. \begin{aligned} \phi &= P_1 e^{\lambda \hat{y}} \cos\left(\frac{m\pi \hat{x}}{\hat{a}}\right) e^{i\hat{\Omega} \hat{t}}, \\ \psi &= P_2 e^{\lambda \hat{y}} \sin\left(\frac{m\pi \hat{x}}{\hat{a}}\right) e^{i\hat{\Omega} \hat{t}}, \quad \text{and} \\ \hat{w} &= P_3 e^{\lambda \hat{y}} \sin\left(\frac{m\pi \hat{x}}{\hat{a}}\right) e^{i\hat{\Omega} \hat{t}}. \end{aligned} \right\} \text{-----} \quad (37)$$

where  $m$  is the number of half waves in the  $x$  direction. The parameter  $\lambda$  is obtained for the given plate by the form of the appropriate boundary conditions on the sides  $\hat{y}=0$ , and  $\hat{y}=\hat{b}=\frac{\hat{b}}{h}$ .



Substituting equation (37) into the above equations (34)-(36), upon rearranging terms, the following matrix form is obtained:

$$[D] \begin{Bmatrix} P_1 \\ P_2 \\ P_3 \end{Bmatrix} = \begin{Bmatrix} 0 \\ 0 \\ 0 \end{Bmatrix} \quad \text{--- (38)}$$

where the elements of the matrix [D] defined as  $d_{ij}$ , are given by the equations

$$\left. \begin{aligned} d_{11} &= \left(\frac{1-\nu}{2}\right) \left\{ \lambda^2 - \left(\frac{m\pi}{a}\right)^2 \right\} - \left(\frac{1+\nu}{2}\right) \left(\frac{m\pi}{a}\right)^2 - \hat{G} + \hat{i} \hat{\Omega}^2, \\ d_{12} &= -d_{21} = \left(\frac{1+\nu}{2}\right) \left(\frac{m\pi}{a}\right) \lambda, \\ d_{13} &= -d_{31} = \hat{C} (m\pi / \hat{a}), \\ d_{22} &= \left(\frac{1-\nu}{2}\right) \left\{ \lambda^2 - \left(\frac{m\pi}{a}\right)^2 \right\} + \left(\frac{1+\nu}{2}\right) \lambda^2 - \hat{G} + \hat{i} \hat{\Omega}^2, \\ d_{23} &= d_{32} = -\hat{C} \lambda, \text{ and} \\ d_{33} &= -\hat{G} \left\{ \lambda^2 - \left(\frac{m\pi}{a}\right)^2 \right\} - \hat{N}_x \left(\frac{m\pi}{a}\right)^2 + \hat{N}_y \lambda^2 - \hat{m} \hat{\Omega}^2. \end{aligned} \right\} \quad (39)$$

The uncoupled equation for  $\hat{\Omega}$  is obtained by formulating the determinant of operator matrix [D] which yields,

$$[B_4 \hat{\Omega}^4 + B_2 \hat{\Omega}^2 + B_0][D_2 \hat{\Omega}^2 + D_0] = 0, \quad (40)$$

where

$$\left. \begin{aligned}
 B_4 &= \frac{\hat{m} \hat{I}}{\hat{C}} , \\
 B_2 &= - \left[ \frac{\hat{I} \pi^2}{(\hat{a} \hat{b})} \left\{ m^2 \left( \frac{\hat{b}}{\hat{a}} \right) \left( 1 - \frac{\hat{N}_x}{\hat{C}} \right) - \left( \frac{\lambda \hat{b}}{\pi} \right)^2 \left( \frac{\hat{a}}{\hat{b}} \right) \left( 1 - \frac{\hat{N}_y}{\hat{C}} \right) \right\} \right. \\
 &\quad \left. + \hat{m} \left( \frac{\pi^2}{\hat{a} \hat{b} \hat{C}} \right) \left\{ m^2 \left( \frac{\hat{b}}{\hat{a}} \right) - \left( \frac{\lambda \hat{b}}{\pi} \right)^2 \left( \frac{\hat{a}}{\hat{b}} \right) \right\} + 1 \right] , \\
 B_0 &= \frac{\pi^4}{(\hat{a} \hat{b})^2} \left\{ m^2 \left( \frac{\hat{b}}{\hat{a}} \right) \left( 1 - \frac{\hat{N}_x}{\hat{C}} \right) - \left( \frac{\lambda \hat{b}}{\pi} \right)^2 \left( \frac{\hat{a}}{\hat{b}} \right) \left( 1 - \frac{\hat{N}_y}{\hat{C}} \right) \right\} \\
 &\quad \left\{ m^2 \left( \frac{\hat{a}}{\hat{b}} \right) - \left( \frac{\lambda \hat{a}}{\pi} \right)^2 \left( \frac{\hat{b}}{\hat{a}} \right) \right\} - \frac{\pi^2}{(\hat{a} \hat{b})} \left\{ m^2 \left( \frac{\hat{a}}{\hat{b}} \right) \hat{N}_x - \left( \frac{\lambda \hat{a}}{\pi} \right)^2 \left( \frac{\hat{b}}{\hat{a}} \right) \hat{N}_y \right\} ,
 \end{aligned} \right\} \quad (41)$$

$$\left. \begin{aligned}
 D_2 &= \hat{I} , \text{ and} \\
 D_0 &= - \left( \frac{1-\nu}{2} \right) \left( \frac{\pi^2}{\hat{a} \hat{b}} \right) \left\{ m^2 \left( \frac{\hat{b}}{\hat{a}} \right) - \left( \frac{\lambda \hat{b}}{\pi} \right)^2 \left( \frac{\hat{a}}{\hat{b}} \right) \right\} - \hat{G} .
 \end{aligned} \right\} \quad (42)$$

The natural frequency of free vibration of the plate is obtained from the biquadratic form of equation (40), which is written as

$$B_4 \hat{\Omega}^4 + B_2 \hat{\Omega}^2 + B_0 = 0 \quad \text{--- --- --- --- ---} \quad (43)$$

From the above equation (43), two independent families of frequency curves are obtained:

the higher set is given as

$$\hat{\Omega}_1^2 = \frac{-B_2}{2B_4} \left[ 1 + \left( 1 - \frac{4B_4B_0}{B_2^2} \right)^{1/2} \right] , \quad \text{--- --- --- --- ---} \quad (44)$$

and the lower set becomes

$$\hat{\Omega}_2^2 = \frac{-B_2}{2B_4} \left[ 1 - \left( 1 - \frac{4B_4B_0}{B_2^2} \right)^{1/2} \right] . \quad \text{--- --- --- --- ---} \quad (45)$$

Substituting  $\hat{\Omega} = 0$  into the equation (43), the equation for critical buckling load is obtained, that is,

$$B_0 = 0 \text{ ----- (46)}$$

Equation (46) yields

$$\begin{aligned} & \left[ m^2 \left( \frac{b}{a} \right) \hat{N}_x - \left( \frac{\lambda b}{\pi} \right)^2 \left( \frac{a}{b} \right) \hat{N}_y \right] \\ & = \frac{\left( \frac{\pi^2}{a b} \right) \left\{ m^2 \left( \frac{b}{a} \right) - \left( \frac{\lambda b}{\pi} \right)^2 \left( \frac{a}{b} \right) \right\}^2}{\left[ 1 + \frac{\pi^2}{a b a} \left\{ m^2 \left( \frac{b}{a} \right) - \left( \frac{\lambda b}{\pi} \right)^2 \left( \frac{a}{b} \right) \right\} \right]} \text{ ----- (47)} \end{aligned}$$

For uniaxial compression case in the  $\hat{x}$  direction, that is  $\hat{N}_y = 0$ , equation (47) becomes

$$\hat{N}_x = \frac{\frac{\pi^2}{m^2 b^2} \left\{ m^2 \cdot \frac{b}{a} - \left( \frac{\lambda b}{\pi} \right)^2 \left( \frac{a}{b} \right) \right\}^2}{\left[ 1 + \frac{\pi^2}{a b a} \left\{ m^2 \cdot \frac{b}{a} - \left( \frac{\lambda b}{\pi} \right)^2 \left( \frac{a}{b} \right) \right\} \right]} \text{ ----- (48)}$$

For uniaxial compression case in the  $\hat{y}$  direction, that is  $\hat{N}_x = 0$ , equation (47) becomes

$$\hat{N}_y = \frac{\frac{\pi^2}{a^2 (\lambda b / \pi)^2} \left\{ m^2 \left( \frac{b}{a} \right) - \left( \frac{\lambda b}{\pi} \right)^2 \left( \frac{a}{b} \right) \right\}^2}{\left[ 1 + \frac{\pi^2}{a b a} \left\{ m^2 \cdot \frac{b}{a} - \left( \frac{\lambda b}{\pi} \right)^2 \left( \frac{a}{b} \right) \right\} \right]} \text{ ----- (49)}$$

For the special case of biaxial compression with  $\hat{N}_x = \hat{N}_y = \hat{N}$ , equation (47) becomes

$$\hat{N} = \frac{\frac{\pi^2}{a b} \left\{ m^2 \left( \frac{b}{a} \right) - \left( \frac{\lambda b}{\pi} \right)^2 \left( \frac{a}{b} \right) \right\}}{\left[ 1 + \frac{\pi^2}{a b a} \left\{ m^2 \cdot \frac{b}{a} - \left( \frac{\lambda b}{\pi} \right)^2 \left( \frac{a}{b} \right) \right\} \right]} \text{ ----- (50)}$$

## CHAPTER IV

## SAMPLE EXAMPLE

Consider a simply supported rectangular plate on all four edges.

## 4.1 Free Vibration Of A Simply Supported Plate.

The following boundary conditions are applied for a simply supported rectangular plate along the lines  $\hat{y}=0$  and  $\hat{y}=b/h=b^A$ .

$$W(0)=0,$$

$$W(b)=0,$$

$$\phi(0)=0,$$

$$\phi(b)=0,$$

$$M_y(0)=0, \text{ and}$$

$$M_y(b)=0.$$

$$\left. \begin{array}{l} \\ \\ \\ \\ \\ \end{array} \right\} \text{---(51)}$$

The last two boundary conditions reduce to the form

$$\psi^1(0)=0, \text{ and}$$

$$\psi^1(b)=0, \text{ where } ( )^1 \text{ denotes the derivative with respect to } y.$$

By noting equations (32) and (51), one obtains

$$\begin{bmatrix}
 0 & 1 & 0 & 1 & 0 & 0 \\
 \sinh \alpha \hat{b} & \cosh \alpha \hat{b} & \sin \beta \hat{b} & \cos \beta \hat{b} & 0 & 0 \\
 0 & \left(\frac{m\pi}{\hat{a}}\right) \delta_1 & 0 & -\left(\frac{m\pi}{\hat{a}}\right) \delta_2 & 0 & 1 \\
 (m\pi/\hat{a})\delta_1 & (m\pi/\hat{a})\delta_1 & -(m\pi/\hat{a})\delta_2 & -(m\pi/\hat{a})\delta_2 & \sinh \tau \hat{b} & \cosh \tau \hat{b} \\
 \sinh \alpha \hat{b} & \cosh \alpha \hat{b} & \sin \beta \hat{b} & \cos \beta \hat{b} & 0 & 0 \\
 0 & \delta_1 \alpha^2 & 0 & \delta_2 \beta^2 & 0 & (m\pi/\hat{a}) \\
 \delta_1 \alpha^2 & \delta_1 \alpha^2 & \delta_2 \beta^2 & \delta_2 \beta^2 & (m\pi/\hat{a}) & (m\pi/\hat{a}) \\
 \sinh \alpha \hat{b} & \cosh \alpha \hat{b} & \sin \beta \hat{b} & \cos \beta \hat{b} & \sinh \tau \hat{b} & \cosh \tau \hat{b}
 \end{bmatrix}
 \begin{bmatrix}
 C_1 \\
 C_2 \\
 C_3 \\
 C_4 \\
 C_5 \\
 C_9 \\
 C_{10}
 \end{bmatrix}
 =
 \begin{bmatrix}
 0 \\
 0 \\
 0 \\
 0 \\
 0 \\
 0 \\
 0
 \end{bmatrix}
 \quad \text{---(52a)}$$

or in symbolic matrix form

$$[E] \{C\} = \{0\}. \quad \text{-----} (52b)$$

For nontrivial solutions of the vector  $\{C\}$ , the determinant of  $[E]$  must be zero. This conditions yields

$$\left[ \delta_1 \left\{ \alpha^2 - \left( \frac{m\pi}{a} \right)^2 \right\} - \delta_2 \left\{ \beta^2 + \left( \frac{m\pi}{a} \right)^2 \right\} \right] \left[ \text{Sinh } \tau \hat{b} \right] \\ \left[ \text{Sinh } (\alpha \hat{b}) \right] \left[ \text{Sin } \beta \hat{b} \right] \quad \text{-----} (53)$$

Since the first three terms of the equation (53) are always nonzero, it follows that

$$\text{Sin } \beta \hat{b} = 0, \quad \text{-----} (54)$$

which is satisfied by the conditions

$$\beta \hat{b} = n\pi, \text{ where } n=1, 2, 3, \dots. (55)$$

Combining equations (26) and (55), yields

$$\lambda \eta = i \left( \frac{n\pi}{b} \right). \quad n=1, 2, 3, \dots. (56)$$

Combining equations (54) and (52a), one obtains the conditions

$$C_1 = C_2 = C_4 = C_9 = C_{10} = 0, \text{ and } \text{-----} (57a)$$

letting  $C_3 = C_n$ , it follows that

$$\left. \begin{aligned} \hat{W}(\hat{y}) &= C_n \text{Sin } \frac{n\pi}{b} \hat{y}, \\ \phi(\hat{y}) &= C_n \left( -\delta_2 \frac{m\pi}{a} \right) \text{Sin } \frac{n\pi}{b} \hat{y}, \\ \psi(\hat{y}) &= C_n \left( -\delta_2 \frac{m\pi}{a} \right) \text{Cos } \frac{n\pi}{b} \hat{y}, \end{aligned} \right\} \text{, and } \text{-----} (57b.)$$

$$\text{where } \delta_2 = \hat{G} / \left[ h^2 \left( \frac{n\pi}{b} \right)^2 + \left( \frac{m\pi}{a} \right)^2 + \hat{G} - \hat{I} \hat{h}^2 \right].$$

Substituting equation (56) into equation (43), yields

$$B_4 \hat{\Omega}^4 + B_2 \hat{\Omega}^2 + B_0 = 0, \quad \text{-----(58)}$$

where

$$\left. \begin{aligned} B_4 &= \frac{\hat{m} \hat{I}}{\hat{G}}, \\ B_2 &= - \left[ \hat{I} \left( \frac{\pi^2}{\hat{a} \hat{b}} \right)^2 \left\{ m^2 \left( \frac{\hat{b}}{\hat{a}} \right) \left( 1 - \frac{\hat{N}_x}{\hat{G}} \right) + n^2 \left( \frac{\hat{a}}{\hat{b}} \right) \left( 1 - \frac{\hat{N}_y}{\hat{G}} \right) \right\} \right. \\ &\quad \left. + \hat{m} \left\{ \frac{\pi^2}{\hat{a} \hat{b} \hat{G}} \left( m^2 \cdot \frac{\hat{b}}{\hat{a}} + n^2 \frac{\hat{a}}{\hat{b}} \right) + 1 \right\} \right], \text{ and} \\ B_0 &= \frac{\pi^4}{(\hat{a} \hat{b})^2} \left\{ m^2 \left( \frac{\hat{b}}{\hat{a}} \right) \left( 1 - \frac{\hat{N}_x}{\hat{G}} \right) + n^2 \left( \frac{\hat{a}}{\hat{b}} \right) \left( 1 - \frac{\hat{N}_y}{\hat{G}} \right) \right\} \\ &\quad \left\{ m^2 \cdot \frac{\hat{b}}{\hat{a}} + n^2 \cdot \frac{\hat{a}}{\hat{b}} \right\} - \frac{\pi^2}{\hat{a} \hat{b}} \left\{ m^2 \cdot \frac{\hat{b}}{\hat{a}} \cdot \hat{N}_x + n^2 \cdot \frac{\hat{a}}{\hat{b}} \cdot \hat{N}_y \right\}. \end{aligned} \right] \quad \text{---(59)}$$

The higher set of frequency equations is given as

$$\hat{\Omega}_1^2 = \frac{-B_2}{2B_4} \left[ 1 + \left( 1 - \frac{4B_4 B_0}{B_2^2} \right)^{1/2} \right], \quad \text{-----(60)}$$

the lower set of frequency equations is given as

$$\hat{\Omega}_2^2 = \frac{-B_2}{2B_4} \left[ 1 - \left( 1 - \frac{4B_4 B_0}{B_2^2} \right)^{1/2} \right]. \quad \text{-----(61)}$$

Substituting  $\hat{\Omega}=0$  into the equation (58), the equation for critical buckling load is obtained, that is, for

$$B_0=0, \quad \text{-----(62)}$$

equation (62) yields

$$\left[ m^2 \cdot \hat{b} / \hat{a} \cdot \hat{N}_x + n^2 \cdot \hat{a} / \hat{b} \cdot \hat{N}_y \right] \\ = \frac{\left( \frac{\pi^2}{\hat{a} \hat{b}} \right) \cdot \left( m^2 \cdot \frac{\hat{b}}{\hat{a}} + n^2 \cdot \frac{\hat{a}}{\hat{b}} \right)^2}{\left[ 1 + \frac{\pi^2}{\hat{a} \hat{b} \hat{G}} \left( m^2 \cdot \frac{\hat{b}}{\hat{a}} + n^2 \cdot \frac{\hat{a}}{\hat{b}} \right) \right]} \quad \text{-----(63)}$$

For the uniaxial compression case in the  $\hat{x}$  direction, that is  $\hat{N}_y=0$ , equation (63) becomes

$$\hat{N}_x = \frac{\frac{\pi^2}{\hat{a} \hat{b}} \cdot \left( m^2 \cdot \frac{\hat{b}}{\hat{a}} + n^2 \cdot \frac{\hat{a}}{\hat{b}} \right)^2}{\left[ 1 + \frac{\pi^2}{\hat{a} \hat{b} \hat{G}} \left( m^2 \cdot \frac{\hat{b}}{\hat{a}} + n^2 \cdot \frac{\hat{a}}{\hat{b}} \right) \right]} \quad \text{-----(64)}$$

$$\hat{N}_y = \frac{\frac{\pi^2}{a^2} n^2 \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right)^2}{\left[ 1 + \frac{\pi^2}{a b G} \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right) \right]} \quad \text{----- (65)}$$

For the special case of biaxial compression with  $\hat{N}_x = \hat{N}_y = \hat{N}$ , equation (63) becomes

$$\hat{N} = \frac{\frac{\pi^2}{a b} \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right)}{\left[ 1 + \frac{\pi^2}{a b G} \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right) \right]} \quad \text{----- (66)}$$

By substituting equation (55) directly into equation (28), and noting equation (23) equation (59) is produced directly.

where

$$R = 1 + \frac{b}{a G} \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right) + \frac{\pi^2}{12} \left( \frac{1}{a b} \right) \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right) - \frac{\pi^2}{12} \left( \frac{1}{a b} \right) \frac{1}{G} \left( m^2 \frac{b}{a} \hat{N}_x + n^2 \frac{a}{b} \hat{N}_y \right)$$

Similarly the second set of the natural frequency as obtained by the equation (61) is written as

$$\hat{\Omega}^2 (B.S.A.T.I.R.I) = \frac{12 \pi^2}{26 a^3} [R]$$

$$\left[ 1 - \frac{\frac{4 \hat{I}}{m G} \left[ \frac{\pi^2}{(a b)^2} \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right)^2 - \frac{\pi^2}{a b} \left( 1 + \frac{b}{a G} \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right) \right) \right]}{\left[ m^2 \frac{b}{a} \hat{N}_x + n^2 \frac{a}{b} \hat{N}_y \right]} \right] \quad \text{----- (68)}$$



## 4.2 Dynamic Stability

(4.2a) Natural frequency of free vibration including bending stress, shear stress, biaxial forces, transverse and rotary inertia.

The natural frequency as obtained from the equation (60) is written as

$$\hat{\Omega}^2 (\text{B.S.A.T.I.R.I.}) = \frac{12\pi^2}{2k\hat{m}} [R]$$

$$\left[ 1 + \left\{ 1 - \frac{\frac{4\hat{I}}{\hat{m}\hat{G}} \left[ \frac{\pi^4}{(\hat{a}\hat{b})^2} \left( m^2 \cdot \frac{\hat{b}}{\hat{a}} + n^2 \cdot \frac{\hat{a}}{\hat{b}} \right)^2 - \frac{\pi^2}{\hat{a}\hat{b}} \left\{ 1 + \frac{k}{\hat{a}\hat{b}} \left( m^2 \frac{\hat{b}}{\hat{a}} + n^2 \frac{\hat{a}}{\hat{b}} \right) \right\} \right\}}{\left[ R^2 \right]} \left\{ m^2 \cdot \frac{\hat{b}}{\hat{a}} \cdot \hat{N}_x + n^2 \cdot \frac{\hat{a}}{\hat{b}} \cdot \hat{N}_y \right\} \right\} \right] \quad \text{----- (67)}$$

where

$$k = \pi^2 / 5 (1 - \nu),$$

$$R = 1 + \frac{k}{\hat{a}\hat{b}} \left( m^2 \cdot \frac{\hat{b}}{\hat{a}} + n^2 \cdot \frac{\hat{a}}{\hat{b}} \right) + \frac{\pi^2}{12} \left( \frac{1}{\hat{a}\hat{b}} \right) \left( m^2 \cdot \frac{\hat{b}}{\hat{a}} + n^2 \cdot \frac{\hat{a}}{\hat{b}} \right) - \frac{\pi^2}{12} \left( \frac{1}{\hat{a}\hat{b}} \right) \frac{1}{\hat{G}} \left( m^2 \cdot \frac{\hat{b}}{\hat{a}} \cdot \hat{N}_x + n^2 \cdot \frac{\hat{a}}{\hat{b}} \cdot \hat{N}_y \right).$$

Similarly the second set of the natural frequency as obtained by the equation (61) is written as

$$\hat{\Omega}^2 (\text{B.S.A.T.I.R.I.}) = \frac{12\pi^2}{2k\hat{m}} [R]$$

$$\left[ 1 - \left\{ 1 - \frac{\frac{4\hat{I}}{\hat{m}\hat{G}} \left[ \frac{\pi^4}{(\hat{a}\hat{b})^2} \left( m^2 \cdot \frac{\hat{b}}{\hat{a}} + n^2 \cdot \frac{\hat{a}}{\hat{b}} \right)^2 - \frac{\pi^2}{\hat{a}\hat{b}} \left\{ 1 + \frac{k}{\hat{a}\hat{b}} \left( m^2 \frac{\hat{b}}{\hat{a}} + n^2 \frac{\hat{a}}{\hat{b}} \right) \right\} \right\}}{\left[ R^2 \right]} \left\{ m^2 \cdot \frac{\hat{b}}{\hat{a}} \cdot \hat{N}_x + n^2 \cdot \frac{\hat{a}}{\hat{b}} \cdot \hat{N}_y \right\} \right\} \right] \quad \text{----- (68)}$$

(4.2b) Natural frequency considering bending stress, shear stress, biaxial forces and transverse inertia.

The natural frequency neglecting rotary inertia, (i.e.,  $\hat{I}=0$ ), becomes

$$\hat{\Omega}^2(\text{B.S.A.TI}) = \frac{\pi^4}{(\hat{a}\hat{b})^2 \hat{m}} \left[ \frac{\left( m^2 \frac{\hat{b}}{\hat{a}} + n^2 \frac{\hat{a}}{\hat{b}} \right)^2}{1 + \frac{k}{\hat{a}\hat{b}} \left( m^2 \frac{\hat{b}}{\hat{a}} + n^2 \frac{\hat{a}}{\hat{b}} \right)} - \left( \frac{\hat{a}\hat{b}}{\pi^2} \right) \left( m^2 \frac{\hat{b}}{\hat{a}} \hat{N}_x + n^2 \frac{\hat{a}}{\hat{b}} \hat{N}_y \right) \right] \text{----- (69)}$$

(4.2c) Natural frequency considering bending stress, biaxial forces and transverse inertia.

The natural frequency neglecting rotary inertia, (i.e.,  $\hat{I}=0$ ) and shear stress, (i.e.,  $1/\hat{G}=0$ ), becomes

$$\hat{\Omega}^2(\text{B.A.TI}) = \frac{\pi^4}{(\hat{a}\hat{b})^2 \hat{m}} \left[ \left( m^2 \frac{\hat{b}}{\hat{a}} + n^2 \frac{\hat{a}}{\hat{b}} \right)^2 - \left( \frac{\hat{a}\hat{b}}{\pi^2} \right) \left( m^2 \frac{\hat{b}}{\hat{a}} \hat{N}_x + n^2 \frac{\hat{a}}{\hat{b}} \hat{N}_y \right) \right] \text{----- (70)}$$

(4.2d) Natural Frequency Ratio.

A nondimensional ratio of natural frequency including and excluding shear stress for the case when rotary inertia is neglected takes the form.

$$\hat{\Omega}^2 = \frac{\hat{\Omega}^2(\text{B.S.A.TI})}{\hat{\Omega}^2(\text{B.A.TI})} = \frac{1 - \frac{\frac{\hat{a}\hat{b}}{\pi^2} \left\{ m^2 \frac{\hat{b}}{\hat{a}} \hat{N}_x + n^2 \frac{\hat{a}}{\hat{b}} \hat{N}_y \right\} \left\{ \frac{k}{\hat{a}\hat{b}} \left( m^2 \frac{\hat{b}}{\hat{a}} + n^2 \frac{\hat{a}}{\hat{b}} \right) \right\}}{\left\{ m^2 \frac{\hat{b}}{\hat{a}} + n^2 \frac{\hat{a}}{\hat{b}} \right\}^2 - \left\{ \frac{\hat{a}\hat{b}}{\pi^2} \left( m^2 \frac{\hat{b}}{\hat{a}} \hat{N}_x + n^2 \frac{\hat{a}}{\hat{b}} \hat{N}_y \right) \right\}}}{1 + \frac{k}{\hat{a}\hat{b}} \left( m^2 \frac{\hat{b}}{\hat{a}} + n^2 \frac{\hat{a}}{\hat{b}} \right)} \text{--- (71)}$$

### 4.3 Static Stability Load

The critical buckling load considering bending stress, biaxial forces and shear stress is given in equation (63).

The critical buckling load considering bending stress and biaxial compression neglecting shear stress, (i.e.,  $1/\hat{G} = 0$ ), into the equation (63), yields

$$\begin{aligned} (\hat{N}_{B.A.})_{CR} &= m^2 \cdot \frac{b}{a} \cdot \hat{N}_x + n^2 \cdot \frac{a}{b} \cdot \hat{N}_y \\ &= \left( \frac{\pi^2}{a^2 b^2} \right) \left\{ m^2 \cdot \frac{b}{a} + n^2 \cdot \frac{a}{b} \right\}^2 \end{aligned} \quad \text{----- (72)}$$

The critical buckling load ratio is the ratio of the critical buckling load including shear and excluding shear. This yields

$$\begin{aligned} \hat{N} &= \frac{(\hat{N}_{B.S.A.})_{CR}}{(\hat{N}_{B.A.})_{CR}} \\ &= \frac{1}{1 + \left( \frac{k}{a^2 b^2} \right) \left( m^2 \cdot \frac{b}{a} + n^2 \cdot \frac{a}{b} \right)} \end{aligned} \quad \text{----- (73)}$$

Combining the equation (63) and (69), yields

$$\hat{\Omega}^2(\text{B.S.A.T.L.}) = \frac{\pi^4}{(a^2 b^2)^2} (\hat{N}_{B.S.A.})_{CR} (1 - \beta), \quad \text{----- (74)}$$

where

$$\beta = \frac{m^2 \cdot \frac{b}{a} \hat{N}_x + n^2 \cdot \frac{a}{b} \hat{N}_y}{(\hat{N}_{B.A.S.})_{CR}}, \quad 0 < \beta < 1. \quad \text{----- (75)}$$

(4.4c) Natural frequency considering bending stress, biaxial forces and transverse inertia.

Combining the equation (70) and (72), yields

$$\hat{\Omega}^2(\text{B.A.T.L.}) = \frac{\pi^4}{(a^2 b^2)^2} (\hat{N}_{B.A.})_{CR} (1 - \frac{\beta}{\hat{N}}) \quad \text{----- (76)}$$

#### 4.4 Relationship Between Natural Frequency And Critical Buckling Load.

(4.4a) The natural frequency considering bending stress, shear stress, biaxial forces, and transverse and rotary inertia, becomes

$$\hat{\Omega}^2_{(B.S.A.TI.RI)} = \frac{12\pi^2}{2k\hat{m}} R \cdot \left[ 1 + \left[ 1 - \frac{(4\hat{I}/\hat{a}) \hat{\Omega}^2_{(B.S.A.TI)}}{\hat{N} \cdot R^2} \right]^{1/2} \right] \dots (74)$$

where

$$R = \frac{1}{\hat{N}} + \frac{\pi^2}{12} \left( \frac{1}{\hat{a}\hat{b}} \right) \left( m^2 \frac{\hat{b}}{\hat{a}} + n^2 \frac{\hat{a}}{\hat{b}} \right) - \frac{\pi^2}{12} \left( \frac{1}{\hat{a}\hat{b}} \right) \frac{1}{\hat{a}} \left( m^2 \frac{\hat{b}}{\hat{a}} \cdot \hat{N}_x + n^2 \frac{\hat{a}}{\hat{b}} \cdot \hat{N}_y \right)$$

similarly the equation (68) becomes

$$\hat{\Omega}^2_{(B.S.A.TI.RI)} = \frac{12\pi^2}{2k\hat{m}} \cdot R \cdot \left[ 1 - \left\{ 1 - \frac{(4\hat{I}/\hat{a}) \hat{\Omega}^2_{(B.S.A.TI)}}{\hat{N} \cdot R^2} \right\}^{1/2} \right] \dots (75)$$

(4.4b) Natural frequency considering bending stress, shear stress, biaxial forces and transverse inertia.

Combining the equation (63) and (69), yields

$$\hat{\Omega}^2_{(B.S.A.TI)} = \frac{\pi^4}{(\hat{a}\hat{b})^2 \hat{m}} \left( \hat{N}_{B.S.A.} \right)_{CR} (1 - \hat{\beta}), \dots (76)$$

where

$$\hat{\beta} = \frac{m^2 \frac{\hat{b}}{\hat{a}} \cdot \hat{N}_x + n^2 \frac{\hat{a}}{\hat{b}} \cdot \hat{N}_y}{\left( \hat{N}_{B.A.S.} \right)_{CR}}, \quad 0 < \hat{\beta} < 1. \dots (77)$$

(4.4c) Natural frequency considering bending stress, biaxial forces and transverse inertia.

Combining the equation (70) and (72), yields

$$\hat{\Omega}^2_{(B.A.TI)} = \frac{\pi^4}{(\hat{a}\hat{b})^2 \hat{m}} \cdot \left( \hat{N}_{B.A.} \right)_{CR} (1 - \hat{\alpha}), \dots (78)$$

where  $\hat{\alpha} = \frac{m^2 \frac{b}{a} \cdot \hat{N}_x + \eta^2 \frac{a}{b} \cdot \hat{N}_y}{(\hat{N}_{B.A.})_{CR}}$ ,  $0 < \hat{\alpha} < 1$ .-----(79)

(4.4d) Natural frequency ratio.

Combining the equations (76)-(79) and equation (73), yields

$$\hat{\Omega}^2 = \frac{\hat{\Omega}^2 (B.S.A.T.I.)}{\hat{\Omega}^2 (B.A.T.I.)} = \frac{1}{1 + \hat{\gamma}}$$

-----(80)

where

$$\hat{\gamma} = \frac{1 - \hat{N}}{\hat{N}(1 - \beta)}$$

-----(81)

SUMMARY SHEET

Natural frequency

$$\hat{\Omega}_{(B.S.A.T.I.R.I.)}^2 = \frac{12\pi^2}{2k\hat{m}} (R) \left[ 1 + \left[ 1 - \frac{4\hat{I}}{\hat{m}\hat{a}} \left[ \frac{\pi^2}{(\hat{a}\hat{b})^2} \left( m^2 \frac{\hat{b}}{\hat{a}} + n^2 \frac{\hat{a}}{\hat{b}} \right)^2 - \frac{\pi^2}{\hat{a}\hat{b}} \left\{ 1 + \frac{k}{\hat{a}\hat{b}} \left( m^2 \frac{\hat{b}}{\hat{a}} + n^2 \frac{\hat{a}}{\hat{b}} \right) \right\} \left\{ m^2 \frac{\hat{b}}{\hat{a}} \hat{N}_x + n^2 \frac{\hat{a}}{\hat{b}} \hat{N}_y \right\} \right] \right] \right]$$

where

$$R = 1 + \frac{k}{\hat{a}\hat{b}} \cdot \left( m^2 \frac{\hat{b}}{\hat{a}} + n^2 \frac{\hat{a}}{\hat{b}} \right) + \frac{\pi^2}{12} \cdot \frac{1}{\hat{a}\hat{b}} \cdot \left( m^2 \frac{\hat{b}}{\hat{a}} + n^2 \frac{\hat{a}}{\hat{b}} \right) - \frac{\pi^2}{12} \left( \frac{1}{\hat{a}\hat{b}} \right) \frac{1}{\hat{a}} \left( m^2 \frac{\hat{b}}{\hat{a}} \hat{N}_x + n^2 \frac{\hat{a}}{\hat{b}} \hat{N}_y \right), \quad k = \frac{\pi^2}{5(1-\nu)}$$

The other set of natural frequency.

$$\hat{\Omega}_{(B.S.A.T.I.R.I.)}^2 = \frac{12\pi^2}{2k\hat{m}} (R) \left[ 1 - \left[ 1 - \frac{4\hat{I}}{\hat{m}\hat{a}} \left[ \frac{\pi^2}{(\hat{a}\hat{b})^2} \left( m^2 \frac{\hat{b}}{\hat{a}} + n^2 \frac{\hat{a}}{\hat{b}} \right)^2 - \frac{\pi^2}{\hat{a}\hat{b}} \left\{ 1 + \frac{k}{\hat{a}\hat{b}} \left( m^2 \frac{\hat{b}}{\hat{a}} + n^2 \frac{\hat{a}}{\hat{b}} \right) \right\} \left\{ m^2 \frac{\hat{b}}{\hat{a}} \hat{N}_x + n^2 \frac{\hat{a}}{\hat{b}} \hat{N}_y \right\} \right] \right] \right]$$

Natural frequency neglecting rotary inertia.

$$\hat{\Omega}_{(B.S.A.T.I.)}^2 = \frac{\pi^4}{(\hat{a}\hat{b})^2 \hat{m}} \left[ \frac{\left( m^2 \frac{\hat{b}}{\hat{a}} + n^2 \frac{\hat{a}}{\hat{b}} \right)^2}{1 + k \cdot \left( \frac{1}{\hat{a}\hat{b}} \right) \left( m^2 \frac{\hat{b}}{\hat{a}} + n^2 \frac{\hat{a}}{\hat{b}} \right)} - \frac{\hat{a}\hat{b}}{\pi^2} \left( m^2 \frac{\hat{b}}{\hat{a}} \hat{N}_x + n^2 \frac{\hat{a}}{\hat{b}} \hat{N}_y \right) \right]$$

Natural frequency neglecting rotary inertia and shear stress.

$$\hat{\Omega}_{(B.A.T.I.)}^2 = \frac{\pi^4}{(\hat{a}\hat{b})^2 \hat{m}} \left[ \left( m^2 \frac{\hat{b}}{\hat{a}} + n^2 \frac{\hat{a}}{\hat{b}} \right)^2 - \left( \frac{\hat{a}\hat{b}}{\pi^2} \right) \left( m^2 \frac{\hat{b}}{\hat{a}} \hat{N}_x + n^2 \frac{\hat{a}}{\hat{b}} \hat{N}_y \right) \right]$$

Natural frequency ratio.

$$\hat{\Omega}^2 = \frac{\hat{\Omega}^2_{(B.S.A.TI)}}{\hat{\Omega}^2_{(B.A.TI)}} = \frac{1 - \frac{\hat{a}\hat{b}}{\pi^2} \left\{ m^2 \frac{\hat{b}}{\hat{a}} \hat{N}_x + n^2 \frac{\hat{a}}{\hat{b}} \hat{N}_y \right\} \left\{ \frac{k}{\hat{a}\hat{b}} \left( m^2 \frac{\hat{b}}{\hat{a}} + n^2 \frac{\hat{a}}{\hat{b}} \right) \right\}}{1 + \frac{k}{\hat{a}\hat{b}} \left( m^2 \frac{\hat{b}}{\hat{a}} + n^2 \frac{\hat{a}}{\hat{b}} \right) \left\{ \left( m^2 \frac{\hat{b}}{\hat{a}} + n^2 \frac{\hat{a}}{\hat{b}} \right)^2 - \frac{\hat{a}\hat{b}}{\pi^2} \left( m^2 \frac{\hat{b}}{\hat{a}} \hat{N}_x + n^2 \frac{\hat{a}}{\hat{b}} \hat{N}_y \right) \right\}}$$

when  $\hat{\Omega} = 0$ ,

Critical buckling load.

$$(\hat{N}_{B.S.A.})_{CR} = m^2 \frac{\hat{b}}{\hat{a}} \hat{N}_x + n^2 \frac{\hat{a}}{\hat{b}} \hat{N}_y = \frac{\pi^2 \left( m^2 \frac{\hat{b}}{\hat{a}} + n^2 \frac{\hat{a}}{\hat{b}} \right)^2}{1 + \frac{k}{\hat{a}\hat{b}} \left( m^2 \frac{\hat{b}}{\hat{a}} + n^2 \frac{\hat{a}}{\hat{b}} \right)}$$

Critical buckling load neglecting shear stress.

$$(\hat{N}_{B.A.})_{CR} = m^2 \frac{\hat{b}}{\hat{a}} \hat{N}_x + n^2 \frac{\hat{a}}{\hat{b}} \hat{N}_y = \frac{\pi^2}{\hat{a}\hat{b}} \left( m^2 \frac{\hat{b}}{\hat{a}} + n^2 \frac{\hat{a}}{\hat{b}} \right)^2$$

Critical buckling load ratio.

$$\hat{N} = \frac{(\hat{N}_{B.S.A.})_{CR}}{(\hat{N}_{B.A.})_{CR}} = \frac{1}{1 + k \left( \frac{1}{\hat{a}\hat{b}} \right) \left( m^2 \frac{\hat{b}}{\hat{a}} + n^2 \frac{\hat{a}}{\hat{b}} \right)}$$

Relationship between natural frequency and critical buckling load.

Natural frequency.

$$\hat{\Omega}^2_{(B.S.A.TI.RI)} = \frac{12\pi^2}{2k\hat{m}} (R) \left[ 1 + \left\{ 1 - \frac{(4\hat{I}/\hat{G}) \hat{\Omega}^2_{(B.S.A.TI)}}{\hat{N}(R^2)} \right\}^{1/2} \right]$$

where

$$R = \frac{1}{\hat{N}} + \frac{\pi^2}{12} \left( \frac{1}{\hat{a}\hat{b}} \right) \left( m^2 \frac{\hat{b}}{\hat{a}} + n^2 \frac{\hat{a}}{\hat{b}} \right) - \frac{\pi^2}{12} \left( \frac{1}{\hat{a}\hat{b}} \right) \frac{1}{\hat{a}} \left( m^2 \frac{\hat{b}}{\hat{a}} \hat{N}_x + n^2 \frac{\hat{a}}{\hat{b}} \hat{N}_y \right)$$

The other set of natural frequency.

$$\hat{\Omega}_{(B.S.A.T.I.)}^2 = \frac{12\pi^2}{2k\hat{m}} (R) \left[ 1 - \left\{ 1 - \frac{(4\hat{I}/\hat{G}) \hat{\Omega}_{(B.S.A.T.I.)}^2}{\hat{N}(R^2)} \right\}^{1/2} \right]$$

Natural frequency neglecting rotary inertia.

$$\hat{\Omega}_{(B.S.A.T.I.)}^2 = \frac{\pi^4}{(\hat{a}\hat{b})^2 \hat{m}} (\hat{N}_{B.S.A.})_{CR} (1 - \hat{\beta}),$$

where

$$\hat{\beta} = \frac{m^2 \frac{\hat{b}}{\hat{a}} \hat{N}_x + n^2 \frac{\hat{a}}{\hat{b}} \hat{N}_y}{(\hat{N}_{B.S.A.})_{CR}}$$

Natural frequency neglecting rotary inertia and shear stress.

$$\hat{\Omega}_{(B.A.T.I.)}^2 = \frac{\pi^4}{(\hat{a}\hat{b})^2 \hat{m}} (\hat{N}_{B.A.})_{CR} (1 - \hat{\alpha}),$$

where

$$\hat{\alpha} = \frac{m^2 \frac{\hat{b}}{\hat{a}} \hat{N}_x + n^2 \frac{\hat{a}}{\hat{b}} \hat{N}_y}{(\hat{N}_{B.A.})_{CR}}$$

Natural frequency ratio.

$$\hat{\Omega}^2 = \frac{\hat{\Omega}_{(B.S.A.T.I.)}^2}{\hat{\Omega}_{(B.A.T.I.)}^2} = \frac{1}{1 + \hat{\gamma}}$$

where

$$\hat{\gamma} = \frac{1 - \hat{N}}{\hat{N}(1 - \hat{\beta})}$$



## CHAPTER V

## GRAPHICAL REPRESENTATION

Using the equations derived in the chapter IV the curves and tables are prepared for the natural frequencies and the critical buckling-loads. The curves and tables are prepared considering the following:

- (1) Thin plate theory states that the minimum thickness of the plate is about one tenth of the smaller dimensions of the plate, which implies that  $1/\hat{\alpha}^2 = h^2/ab \leq 0.01$ . Shear stress effect are considerable as the thickness of the plate increases. The parameter  $h^2/ab$ , where  $0.01 \leq h^2/ab \leq 0.05$  are considered from the reference (2).
- (2) The parameter  $\hat{\beta}$ , which is defined as the ratio of actual in plane force to the critical buckling-load including shear, varies over the range  $0 \leq \hat{\beta} < 1$ . The specific values which are considered are  $\hat{\beta} = 0, 0.01, 0.25, 0.50, 0.75$  and  $0.99$ .
- (3) The parameter  $\hat{\alpha}$ , which is defined as the ratio of actual in plane force to the critical buckling-load excluding shear, varies over the range  $0 \leq \hat{\alpha} < 1$ . The specific values which are considered are  $\hat{\alpha} = 0, 0.01, 0.25, 0.50, 0.75$  and  $0.99$ .
- (4) The combinations of modes for the curves of natural frequency and the critical buckling-load, ( $m=1, 2, 3$  and  $n=1, 2, 3$ ) are used.
- (5) The plate dimensions  $0 < a/b < 5$ , (i.e., length/width) are used.

- (6) Poisson's Ratio is assumed as  $\nu = 0,30$ .
- (7) For curves and tables the uniaxial force in the x direction, that is  $\hat{N}_y = 0$ , is considered.
- (8) In general, for all values of  $\hat{\beta}$  or  $\hat{\alpha}$  and  $h^2/ab$  the curve for the first mode, (i.e.,  $m=1, n=1$ ), for the natural frequency and critical buckling-loads are plotted. For all other modes (i.e.,  $m=1, 2, 3$  and  $n=1, 2, 3$ ) the values of natural frequency and critical buckling-loads are given in tabular form.
- (9) A summary sheet table for minimum or maximum values of frequency and critical buckling-loads are tabulated. These values are obtained by noting that the extremum condition requires  $a/b = m/n$ .

### 5.1 Critical Buckling-Load Including Shear vs a/b.

For  $\hat{N}_y=0$ , the equation (63) becomes

$$(\hat{N}_{B.S.A.})_{CR} = \left(\frac{\hat{b}}{\pi}\right)^2 \hat{N}_x = \frac{\frac{1}{m^2} \left(m^2 \frac{\hat{b}}{\hat{a}} + n^2 \frac{\hat{a}}{\hat{b}}\right)^2}{1 + \left(\frac{k}{\hat{a}\hat{b}}\right) \left(m^2 \frac{\hat{b}}{\hat{a}} + n^2 \frac{\hat{a}}{\hat{b}}\right)} .$$

or

$$(\hat{N}_{B.S.A.})_{CR} = \left(\frac{\hat{b}^2}{\pi^2 D}\right) N_x = \frac{\frac{1}{m^2} \left(m^2 \frac{\hat{b}}{\hat{a}} + n^2 \frac{\hat{a}}{\hat{b}}\right)^2}{1 + k \cdot \frac{h^2}{\hat{a}\hat{b}} \cdot \left(m^2 \frac{\hat{b}}{\hat{a}} + n^2 \frac{\hat{a}}{\hat{b}}\right)} . \quad \text{---- (82)}$$

Using the above equation (82) figures 1A and 1B are drawn and tables 1a through 1d are tabulated.

The figure 1A shows that critical buckling-load decreases as the parameter  $h^2/ab$  increases. As the parameter  $h^2/ab$  increases from 0.01 to 0.05 the values of minimum critical buckling-load decreases about 17.60%.

## 5.2 Critical Buckling-Load Excluding Shear vs a/b.

For  $\hat{N}_y=0$ , the equation (73) becomes

$$(\hat{N}_{B.A.})_{CR} = \left(\frac{b}{\pi}\right)^2 \hat{N}_x = \frac{1}{m^2} \left(m^2 \frac{b}{a} + n^2 \frac{a}{b}\right)^2$$

or

$$(\hat{N}_{B.A.})_{CR} = \left(\frac{b^2}{\pi^2 D}\right) N_x = \frac{1}{m^2} \left(m^2 \frac{b}{a} + n^2 \frac{a}{b}\right)^2 \quad \text{----(83)}$$

Using the equation (83) the curve, (#1), is plotted on the figure 1A. and table 1e is tabulated.

Figure 1A shows that the minimum critical buckling-load excluding shear is about 5.34% higher than the minimum critical buckling load including shear, for the parameter  $h^2/ab=0.01$  and is about 22% higher for the parameter  $h^2/ab=0.05$ .

## 5.3 Critical Buckling-Load Ratio vs a/b.

Combining the equations (82) and (83), yields

$$\hat{N} = \frac{(\hat{N}_{B.S.A.})_{CR}}{(\hat{N}_{B.A.})_{CR}} = \frac{1}{1 + \left(\frac{k}{ab}\right) \left(m^2 \frac{b}{a} + n^2 \frac{a}{b}\right)}$$

or

$$\hat{N} = \frac{(\hat{N}_{B.S.A.})_{CR}}{(\hat{N}_{B.A.})_{CR}} = \frac{1}{1 + k \cdot \frac{h^2}{ab} \cdot \left(m^2 \frac{b}{a} + n^2 \frac{a}{b}\right)} \quad (84)$$

Using the above equation (84) figures 2A and 2B are drawn and tables 2a through 2d are tabulated.

Figure 2A shows that the critical buckling-load ratio decreases as the parameter  $h^2/ab$  increases. As the parameter  $h^2/ab$  increases from 0.01 to 0.05 values, the maximum critical buckling-load ratio decreases by about 17.60%.

- (a) Figures (3) through (7) shows that, as the values of  $\beta$  increases the value of natural frequency including shear decreases.
- (b) For the case  $\beta = 0.99$ , the value of natural frequency approaches zero for  $0.5 < a/b < 5.0$ .
- (c) As the parameter  $h^2/ab$  increases from 0.01 to 0.05 for each value of  $\beta$  the minimum natural frequency value decreases about 17% to 18%.

## 5.4 Natural Frequency Of Free Vibration Including Shear vs a/b.

Equation (70) becomes

$$\hat{\Omega}_{(B.S.A.T.I.)}^2 = \frac{\Omega_{(B.S.A.T.I.)}^2}{\pi^4 / (\alpha^2 b^2 q h)} = \frac{(m^2 \cdot \frac{b}{a} + n^2 \cdot \frac{a}{b})^2 (1 - \hat{\beta})}{[1 + k \cdot \frac{h^2}{ab} (m^2 \cdot \frac{b}{a} + n^2 \cdot \frac{a}{b})]} \dots\dots\dots (85)$$

where

$$\hat{\beta} = \frac{N_x \left( \frac{b^2}{\pi^2 D} \right)}{(\hat{N}_{B.S.A.})_{LR}}, \quad 0 \leq \hat{\beta} \leq 1 \dots\dots\dots (86)$$

Using the above equations (85) and (86) figures (3) through (7) are plotted and tables (3a) through (3e) to (7a) through (7e) are tabulated.

From the figures (3) through (7) following observations are noted:

- (a) Figures (3) through (7) shows that, as the values of  $\hat{\beta}$  increases the value of natural frequency including shear decreases.
- (b) For the case  $\hat{\beta} = 0.99$ , the value of natural frequency approximately zero for  $0.5 < a/b < 5.0$ .
- (c) As the parameter  $h^2/ab$  increases from 0.01 to 0.05 for each value of  $\hat{\beta}$  the minimum natural frequency value decreases about 17% to 18%.

### 5.5 Natural Frequency Excluding Shear vs a/b.

Equation (71), yields (85) through (89), yields

$$\hat{\Omega}_{(B.A.T.I.)}^2 = \frac{\Omega_{(B.A.T.I.)}^2}{\pi^4 / \left( \frac{a^2 b^2 \eta h}{D} \right)} = \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right)^2 (1 - \hat{\alpha}), \quad \text{----- (87)}$$

where

$$\hat{\alpha} = \frac{N_x (b^2 / \pi^2 D)}{(N_{B.A.})_{CR}}, \quad 0 \leq \hat{\alpha} < 1. \quad \text{----- (88)}$$

Using the equations (87) and (88) figure (8) is plotted and tables (8a) through (8e) are tabulated.

Figure (8) shows that, as the value of  $\hat{\alpha}$  increases the value of natural frequency excluding shear decreases. For the case  $\hat{\alpha} = 0.99$ , the value of natural frequency approximately zero for  $0.5 < a/b < 5.0$ .

increases.

(b) For the case  $\hat{\alpha} \rightarrow 0$  the values are equal to the values of critical buckling-load ratio.

(c) As the parameter  $h^2/ab$  increases from the value 0.01 to 0.05 for  $0.01 < \hat{\alpha} < 0.99$ , the maximum value of natural frequency ratio decreases from 184 to 774.

## 5.6 Natural Frequency Ratio vs a/b.

Combining the equations (85) through (89), yields

$$\hat{\Omega}^2 = \frac{\hat{\Omega}^2_{(B.S.A.T.I.)}}{\hat{\Omega}^2_{(B.A.T.I.)}} = \frac{1}{1 + \hat{\gamma}} \quad (89)$$

where

$$\hat{\gamma} = \frac{1 - \hat{N}}{\hat{N}(1 - \hat{\beta})} \quad (90)$$

Using the above equations (89) and (90) the figures (9) through (13) are plotted and tables (9a) through (9e) to (13a) through (13e) are tabulated.

From the figures (9) through (13) following observations made:

- (a) The natural frequency ratio decreases as the value of  $\hat{\beta}$  increases.
- (b) For the case  $\hat{\beta} = 0$  the values are equal to the values of critical buckling-load ratio.
- (c) As the parameter  $h^2/ab$  increases from the value 0.01 to 0.05 for  $0.01 < \hat{\beta} < 0.99$ , the maximum value of natural frequency ratio decreases from 18% to 77%.



5.7 Lower Set of Natural Frequency Including Shear and Rotary Inertia vs  $a/b$ .

Equation (75) becomes

$$\hat{\Omega}_{(B.S.A.R.I.II)}^2 = \frac{\omega_{(B.S.A.T.I.R.I.)}^2}{\pi^4 / (\alpha^2 b^2 e h)} = \frac{1}{2 \frac{\pi^2}{12} k} \left( \frac{ab}{h^2} \right)^2 (R).$$

where

$$\left[ 1 - \left\{ 1 - \frac{4 \cdot \frac{\pi^2}{12} k \cdot \left( \frac{h^2}{ab} \right)^2 (m^2 \frac{b}{a} + n^2 \frac{a}{b})^2 (1 - \hat{\beta})}{(R)^2} \right\}^{1/2} \right] \dots (91)$$

$$R = 1 + \left( k + \frac{\pi^2}{12} \right) \cdot \frac{h^2}{ab} \cdot \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right) - \frac{k \cdot \frac{\pi^2}{12} \hat{\beta} \left( \frac{h^2}{ab} \right)^2 (m^2 \frac{b}{a} + n^2 \frac{a}{b})^2}{1 + k \cdot \frac{h^2}{ab} \cdot \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right)}$$

Using the equation (91) figures (14) through (18) are drawn and tables (13a) through (13e) to (18a) through (18d) are tabulated.

Following observations are made:

- As the value of  $\hat{\beta}$  increases the natural frequency ratio decreases.
- As the parameter  $h^2/ab$  increases from 0.01 to 0.05, for each  $0.01 < \hat{\beta} < 0.99$ , the minimum value of natural frequency decreases to about 20.41%.

### 5.8 Higher Set of Natural Frequency Including Shear and Rotary Inertia vs $a/b$ .

Equation (74) becomes

$$\hat{\Omega}_{(B.S.A.T.I.R.I.)}^2 = \frac{\Omega_{(B.S.A.T.I.R.I.)}^2}{\pi^4 / \left( \frac{\alpha^2 b^2 e h}{D} \right)} = \frac{1}{2 \frac{\pi^2}{12} k} \cdot \left( \frac{ab}{h^2} \right)^2 (R).$$

$$\left[ 1 + \left[ 1 - \frac{4 \cdot \frac{\pi^2}{12} \cdot \left( \frac{h^2}{ab} \right)^2 \cdot \hat{\beta} \cdot \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right)^2}{(R)^2} \right]^{\frac{1}{2}} \right] \dots (92)$$

where

$$R = 1 + \left( k + \frac{\pi^2}{12} \right) \cdot \frac{h^2}{ab} \cdot \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right) - \frac{k \cdot \frac{\pi^2}{12} \cdot \hat{\beta} \cdot \left( \frac{h^2}{ab} \right)^2 \cdot \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right)^2}{1 + k \cdot \frac{h^2}{ab} \cdot \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right)}$$

Using the equation (92) and  $\hat{\beta} = 0$ , figure (19) is drawn and tables (19a) through (19d) are tabulated.

Figure (19) shows that, as the parameter  $h^2/ab$  increases from 0.01 to 0.05 the minimum value of natural frequency decreases by about 94.97%.

### 5.8 Higher Set of Natural Frequency Including Shear and Rotary Inertia vs $a/b$ .

Equation (74) becomes

$$\hat{\Omega}_{(B.S.A.T.I.R.I.)}^2 = \frac{\Omega_{(B.S.A.T.I.R.I.)}^2}{\pi^4 / \left( \frac{a^2 b^2 e h}{D} \right)} = \frac{1}{2 \frac{\pi^2}{12} k} \cdot \left( \frac{ab}{h^2} \right)^2 (R).$$

$$\left[ 1 + \left[ 1 - \frac{4 \cdot \frac{\pi^2}{12} \cdot \left( \frac{h^2}{ab} \right)^2 \cdot \hat{\beta} \cdot \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right)^2}{(R)^2} \right]^{\frac{1}{2}} \right] \dots (92)$$

where

$$R = 1 + \left( k + \frac{\pi^2}{12} \right) \cdot \frac{h^2}{ab} \cdot \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right) - \frac{k \cdot \frac{\pi^2}{12} \cdot \hat{\beta} \cdot \left( \frac{h^2}{ab} \right)^2 \cdot \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right)^2}{1 + k \cdot \frac{h^2}{ab} \cdot \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right)}$$

Using the equation (92) and  $\hat{\beta} = 0$ , figure (19) is drawn and tables (19a) through (19d) are tabulated.

Figure (19) shows that, as the parameter  $h^2/ab$  increases from 0.01 to 0.05 the minimum value of natural frequency decreases by about 94.97%.

## CHAPTER VI

## SUMMARY

## Discussion

Using the equations of motion for the free vibrations of a rectangular plate the uncoupled 6th order operator for the functions  $\phi(x,y,t)$  and  $\psi(x,y,t)$  and the uncoupled 4th order operator for  $w(x,y,t)$  are obtained. Neglecting the effect of shear and rotary inertia the above uncoupled operator reduces to the classical 4th order equation of motion including the effect of bending stress, axial force and transverse inertia.

The sets of natural frequencies are determined from the biquadratic equation for the two parallel sides simply supported and other two parallel sides subjected to different boundary conditions. Setting natural frequency equal to zero the critical buckling loads are also determined, thus the static stability of the rectangular plates are studied from the dynamic stability equations.

For the special case of a simply supported plate on all four edges the natural frequencies are determined. It is seen that the natural frequency excluding rotary inertia, and excluding both shear and rotary inertia yield one set of natural frequency curves, while the natural frequency including rotary inertia and shear have two independent sets of frequencies curves. From numerical analysis it is found that the values of natural frequency excluding rotary inertia, natural frequency excluding both shear and rotary inertia, and the

lower set of natural frequency including shear and rotary inertia are dependent on the axial force while the higher set of natural frequency values are affected a negligible amount by the presence of axial force.

### Conclusions

From the curves and tables of the natural frequency and the critical buckling-load the following observations are noted:

- (1) For  $a/b < 0.50$ , a large decrease of critical buckling-load occurs for a given change in the parameter  $h^2/ab$ .
- (2) As the parameter  $h^2/ab$ , which accentuates the effect of shear stress, increases the critical buckling-load decreases. Thus the critical buckling-load excluding shear is higher in value than the value of critical buckling load including shear. The minimum critical buckling load excluding shear is about 5.34% higher than the minimum critical buckling-load including shear for the parameter  $h^2/ab=0.01$ , and is about 22% higher for the parameter  $h^2/ab=0.05$ .
- (3) Considering the different characteristic mode shapes from the tables, it is seen that when rectangular plate buckles it subdivides approximately into squares.
- (4) For  $a/b < 0.50$ , the decrease in the values of natural frequencies including shear is about 97.72% for the parameters  $h^2/ab=0.01$  and  $\hat{\beta}=0$  and it is 95.60% for the parameters  $h^2/ab=0.05$  and  $\hat{\beta}=0$ . For the parameter  $\hat{\beta}=0.25$  the values of natural frequencies including shear decreases by about 97.72% for the parameter  $h^2/ab=0.01$  and by 95.60% for the parameter  $h^2/ab=0.05$ .

(5) As the parameter  $h^2/ab$ , which accentuates the effect of shear, increases the natural frequency decreases. This means that the natural frequency excluding shear is always higher in value than the value of natural frequency including shear. The minimum value of natural frequency excluding shear is about 5.34% higher than the minimum value of natural frequency including shear for the parameters  $h^2/ab=0.01$  and  $\hat{\beta}=0$ , and is about 22% higher for the parameters  $h^2/ab=0.05$  and  $\hat{\beta}=0$ . For the parameter  $\hat{\beta}=0.25$  the minimum value of the natural frequency excluding shear is about 6.99% more than the value of natural frequency including shear for the parameter  $h^2/ab=0.01$  and is about 27.32% for the parameter  $h^2/ab=0.05$ .

(6) As the parameters  $\hat{\beta}$  or  $\hat{\alpha}$ , which accentuate the effect of axial force, increase the natural frequency decreases. Thus, the value of natural frequency excluding axial force is higher than the value of natural frequency including axial force. If the parameter  $\hat{\beta}$  increases from 0.0 to 0.25 the minimum value of natural frequency including shear decreases by about 24.99%; when  $\hat{\beta}$  increases from 0.25 to 0.50 the minimum value of natural frequency including shear decreases by 33.34%; when  $\hat{\beta}$  increases from 0.50 to 0.75 the minimum value of natural frequency including shear decreases by about 49.97%.

(7) Comparing the value of natural frequency excluding rotary inertia with the lower set of natural frequency including rotary inertia shows that the minimum value of natural frequency excluding rotary inertia is about 1.44% higher for the parameter  $h^2/ab=0.01$  and is about 4.81% higher for the parameter  $h^2/ab=0.05$  for the value  $\hat{\beta}=0$ .

(8) The ratio of higher set of natural frequencies including shear and rotary inertia to the lower set of natural frequencies including

shear and rotary inertia is about 35:1 for the parameter  $h^2/ab=0.01$  and is about 8.86:1 for the parameter  $h^2/ab=0.05$ .

(9) The effect of the parameter  $\hat{\beta}$  is negligible on the value of higher set of natural frequencies including shear and rotary inertia.

Brunelle analyzed the rectangular plates subjected to different boundary conditions for evaluating the critical buckling-loads. The combinations of the various boundary conditions considered for two parallel sides are (a) simply supported and free, (b) fixed and free, (c) both simply supported and (d) both fixed. Based on procedure outlined in this thesis, evaluation for the natural frequencies of free vibrations for the above boundary conditions may be carried out for the dynamics analysis of rectangular plates.

## APPENDIX A

## GRAPHICAL RESULTS

$$(N_{b,s,a})_{CR} = \frac{1}{m^2} \frac{(m^2 b^2 + n^2 a^2)^2}{1 + R \frac{h^2}{ab} (m^2 \frac{b^2}{a^2} + n^2 \frac{a^2}{b^2})}$$

where

$$R = \eta^2 / (50(1-\nu)), \text{ and}$$

$$\nu = 0.30.$$

$$\textcircled{m} (N_{b,s,a})_{CR} = \frac{1}{m^2} (m^2 \frac{b^2}{a^2} + n^2 \frac{a^2}{b^2})^2$$

$$(N_{b,s,a})_{CR, \text{Minimum}} = 4.000.$$

$\frac{h^2}{ab}$	$(N_{b,s,a})_{CR}$ Minimum
0.01	3.7564
0.02	3.6945
0.03	3.6518
0.04	3.6257
0.05	3.6011

( $m=1, n=1$ )

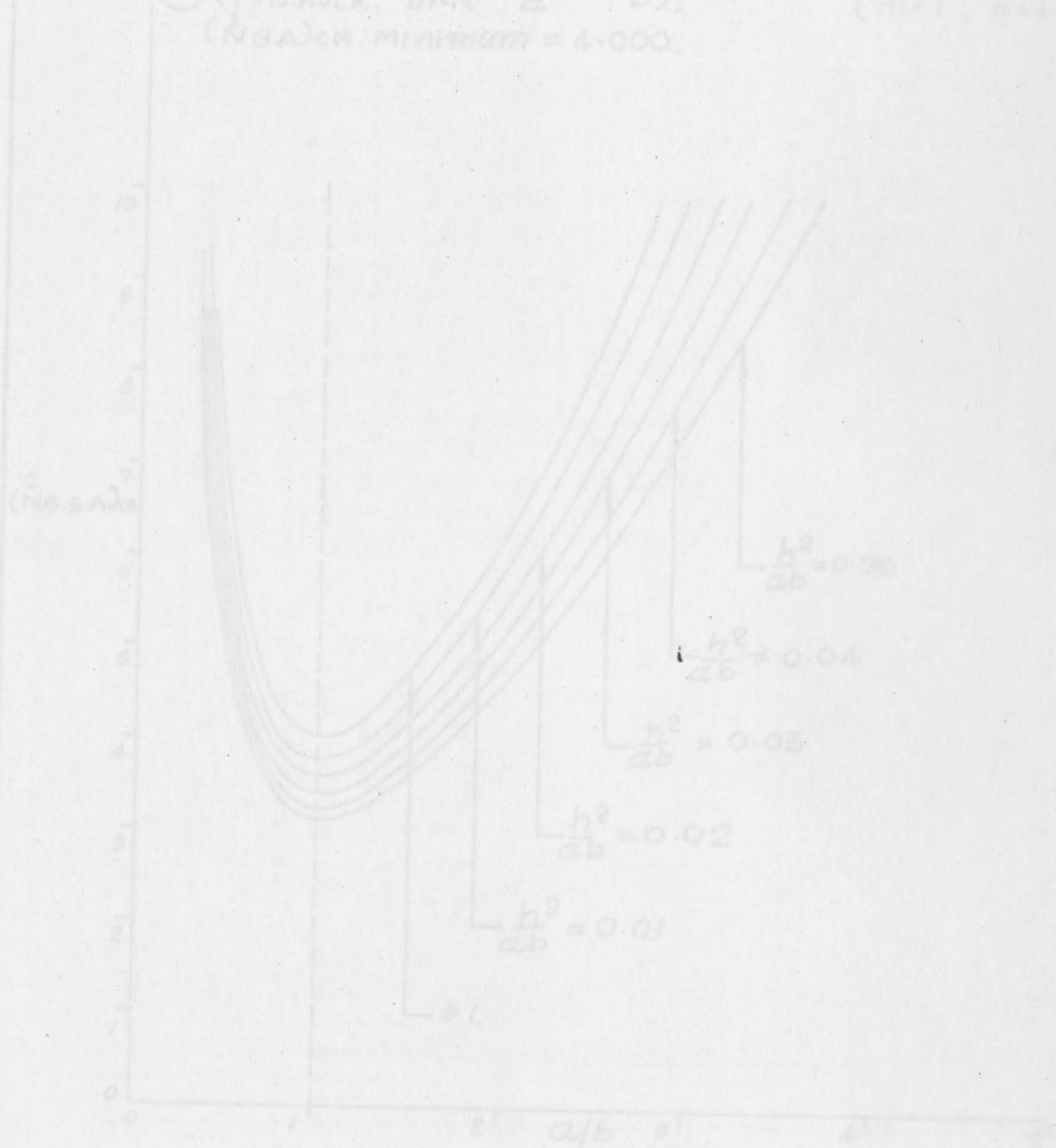


Figure 1-A. Critical Buckling Load Including Shear vs.  $a/b$ .



$$(\hat{N}_{B.S.A.})_{CR} = \frac{\frac{1}{m^2} \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right)^2}{1 + k \frac{h^2}{ab} \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right)}$$

where  
 $k = \pi^2 / 5(1 - \nu)$ , and  
 $\nu = 0.30$ .

$\frac{h^2}{ab}$	$(\hat{N}_{B.S.A.})_{CR}$ Minimum
0.01	3.7864
0.02	3.5945
0.03	3.4212
0.04	3.2637
0.05	3.1201

( $m=1, n=1$ )

#1.  $(\hat{N}_{B.A.})_{CR} = \frac{1}{m^2} \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right)^2$   
 $(\hat{N}_{B.A.})_{CR, \text{Minimum}} = 4.000$

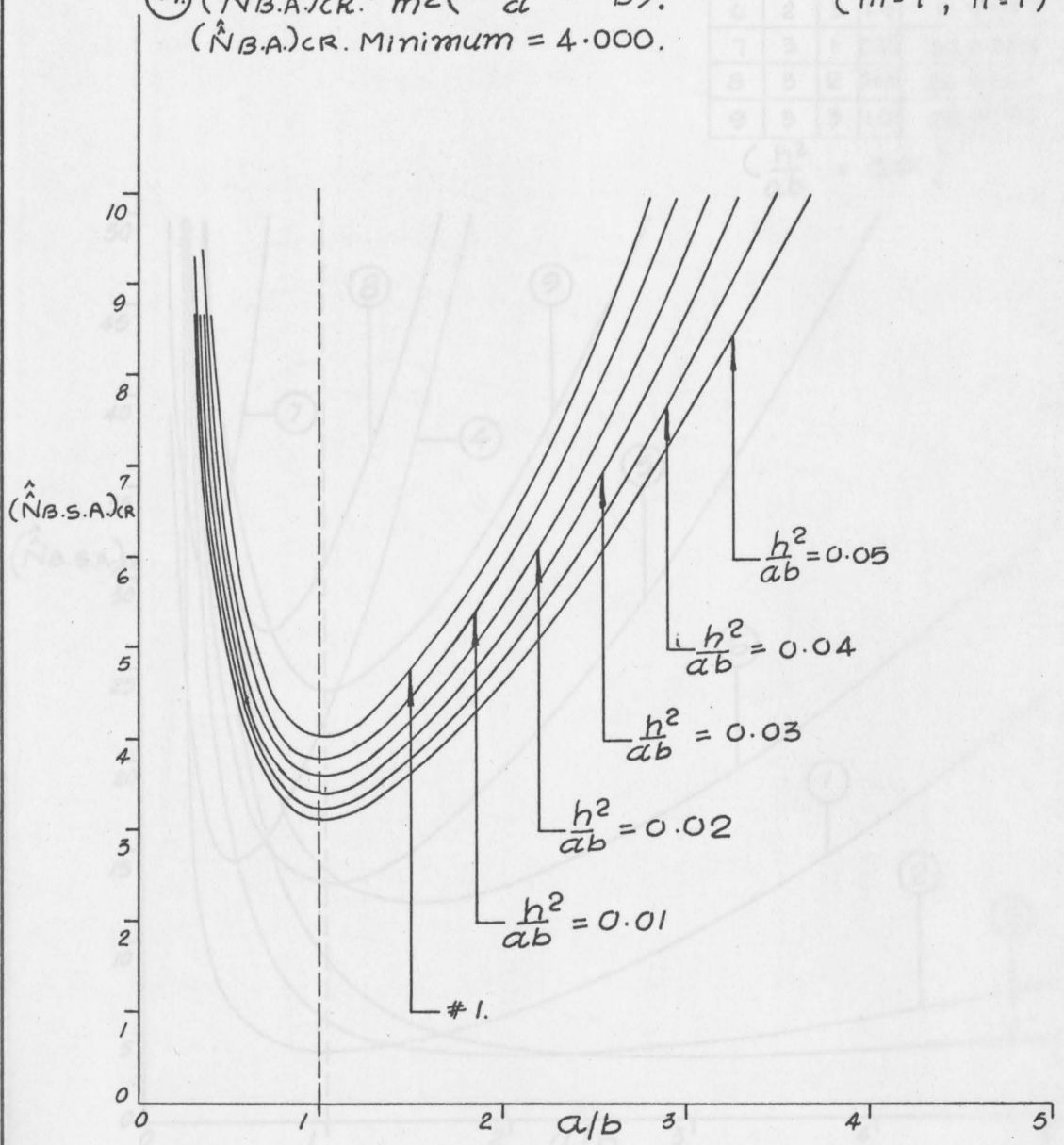


Figure 1-A. Critical Buckling Load Including Shear vs.  $a/b$ .

$$(\hat{N}_{B.S.A.})_{CR.} = \frac{\frac{1}{m^2} (m^2 \frac{b}{a} + n^2 \frac{a}{b})^2}{1 + k \frac{h^2}{ab} (m^2 \frac{b}{a} + n^2 \frac{a}{b})}$$

where

$k = \pi^2/5(1-\nu)$ , and

$\nu = 0.30$ .

#	n	m	a/b	( $\hat{N}_{B.S.A.}$ ) CR. Minimum
1	1	1	1	3.7864
2	1	2	2	3.5945
3	1	3	3	3.4212
4	2	1	0.5	14.3782
5	2	2	1.0	13.0549
6	2	3	1.5	11.9547
7	3	1	0.35	30.8585
8	3	2	0.65	26.9131
9	3	3	1.0	23.8792

$$\left(\frac{h^2}{ab} = 0.01\right)$$

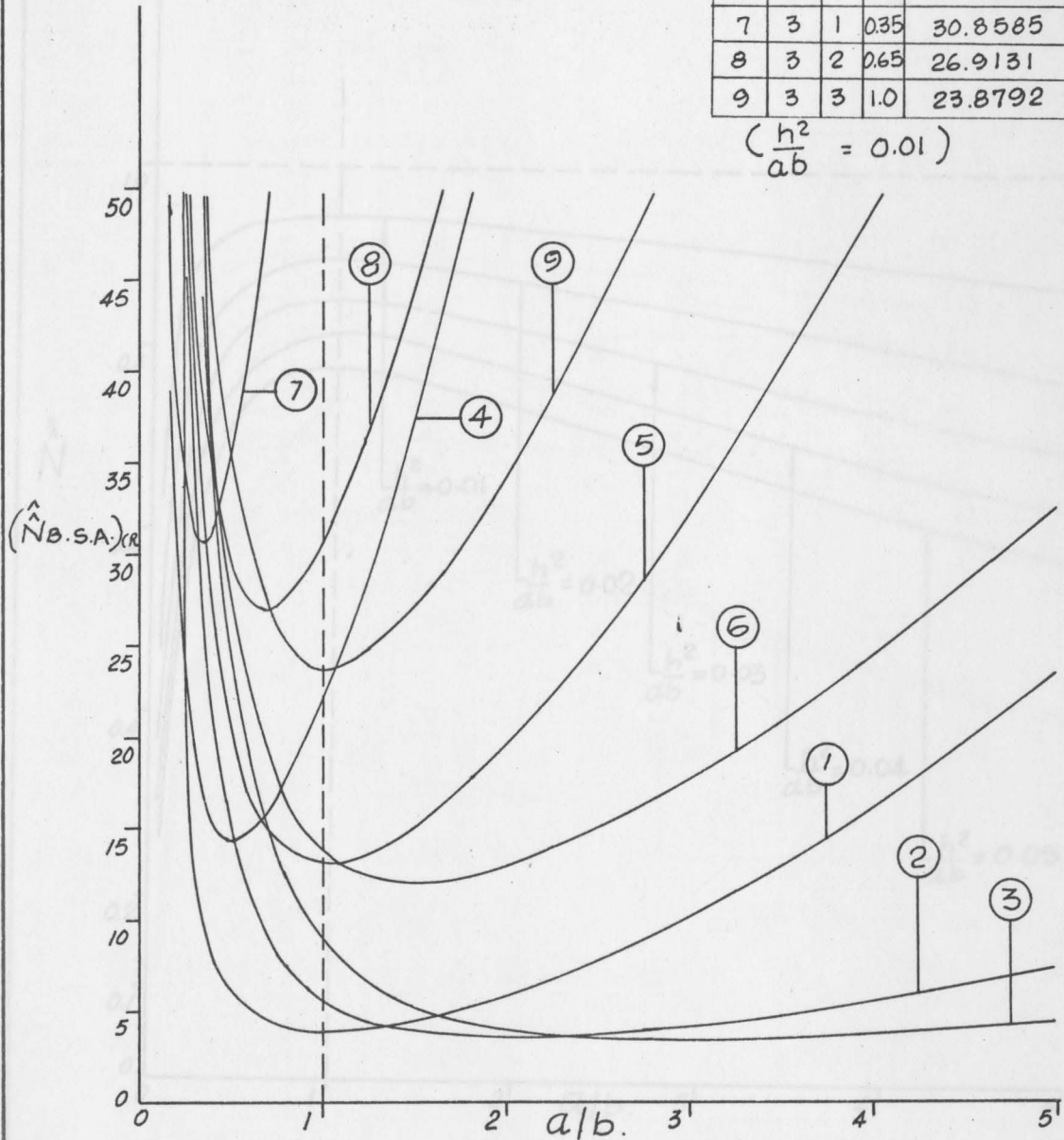


Figure 1-B. Critical Buckling Load Including Shear vs. a/b.

$$\hat{N} = \frac{(\hat{N}_{B.S.A.})_{CR.}}{(\hat{N}_{B.A.})_{CR.}} = \frac{1}{1 + k \frac{h^2}{ab} (m^2 \frac{b}{a} + n^2 \frac{a}{b})}$$

where  
 $k = \pi^2 / 5(1-\nu)$ , and  
 $\nu = 0.30$ .

$\frac{h^2}{ab}$	$\hat{N}$ Maximum
0.01	0.9466
0.02	0.8986
0.03	0.8553
0.04	0.8159
0.05	0.7800

(m = 1, n = 1)

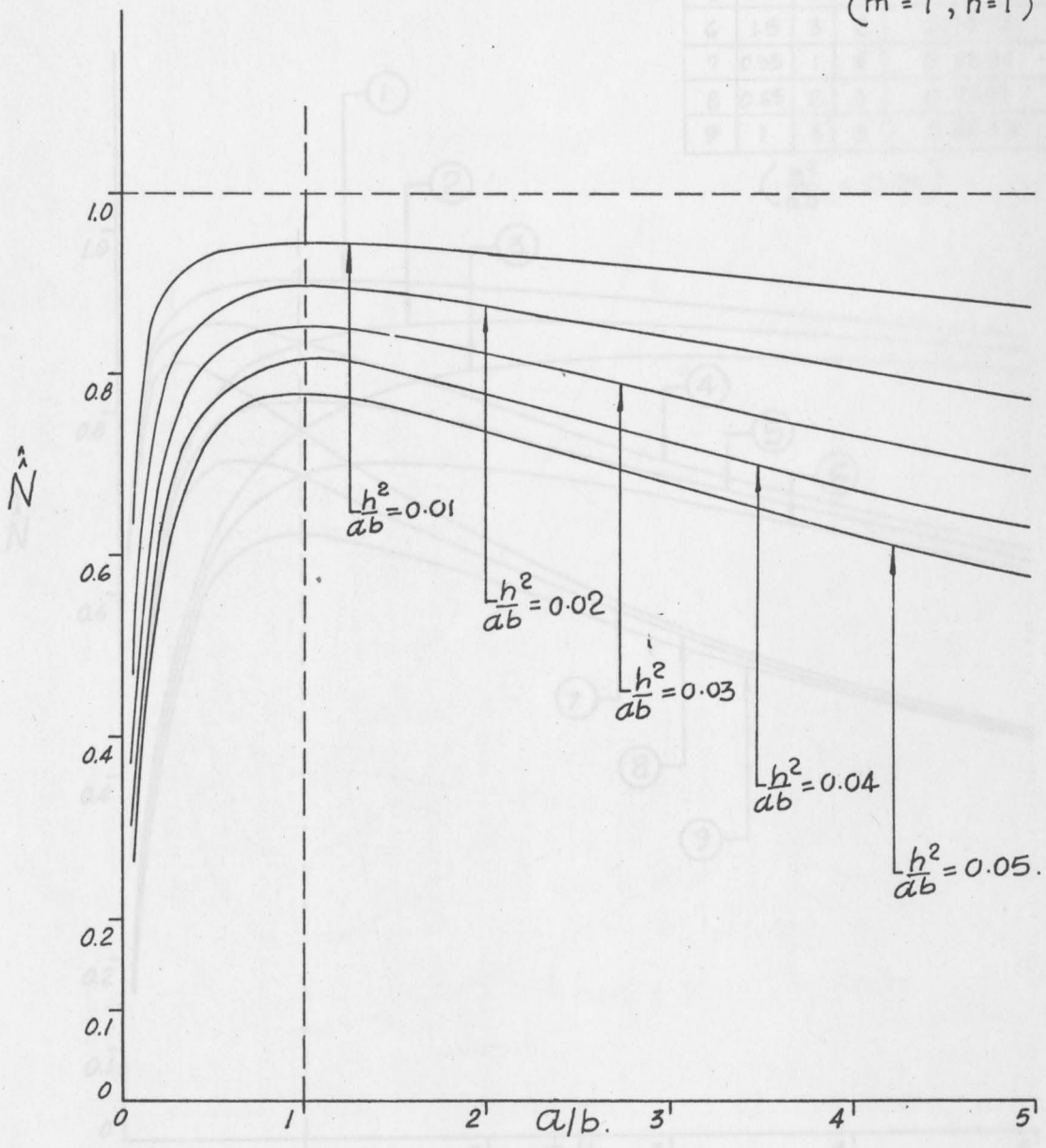


Figure 2-A. Critical Buckling Load Ratio vs. a/b.

Figure 2-B. Critical Buckling Load Ratio vs. a/b.

$$\hat{N} = \frac{(\hat{N}_{B.S.A.})_{CR.}}{(\hat{N}_{B.A.})_{CR.}} = \frac{1}{1 + k \frac{h^2}{ab} (m^2 \frac{b}{a} + n^2 \frac{a}{b})},$$

where

$k = \pi^2 / 5(1 - \nu)$ , and

$\nu = 0.30$ .

#	n	m	$\frac{a}{b}$	$\hat{N}$ Maximum
1	1	1	1	0.9466
2	2	2	1	0.8986
3	3	3	1	0.8553
4	0.5	1	2	0.8986
5	1.0	2	2	0.8159
6	1.5	3	2	0.7472
7	0.35	1	3	0.8551
8	0.65	2	3	0.7471
9	1	3	3	0.6633

$$\left(\frac{h^2}{ab} = 0.01\right)$$

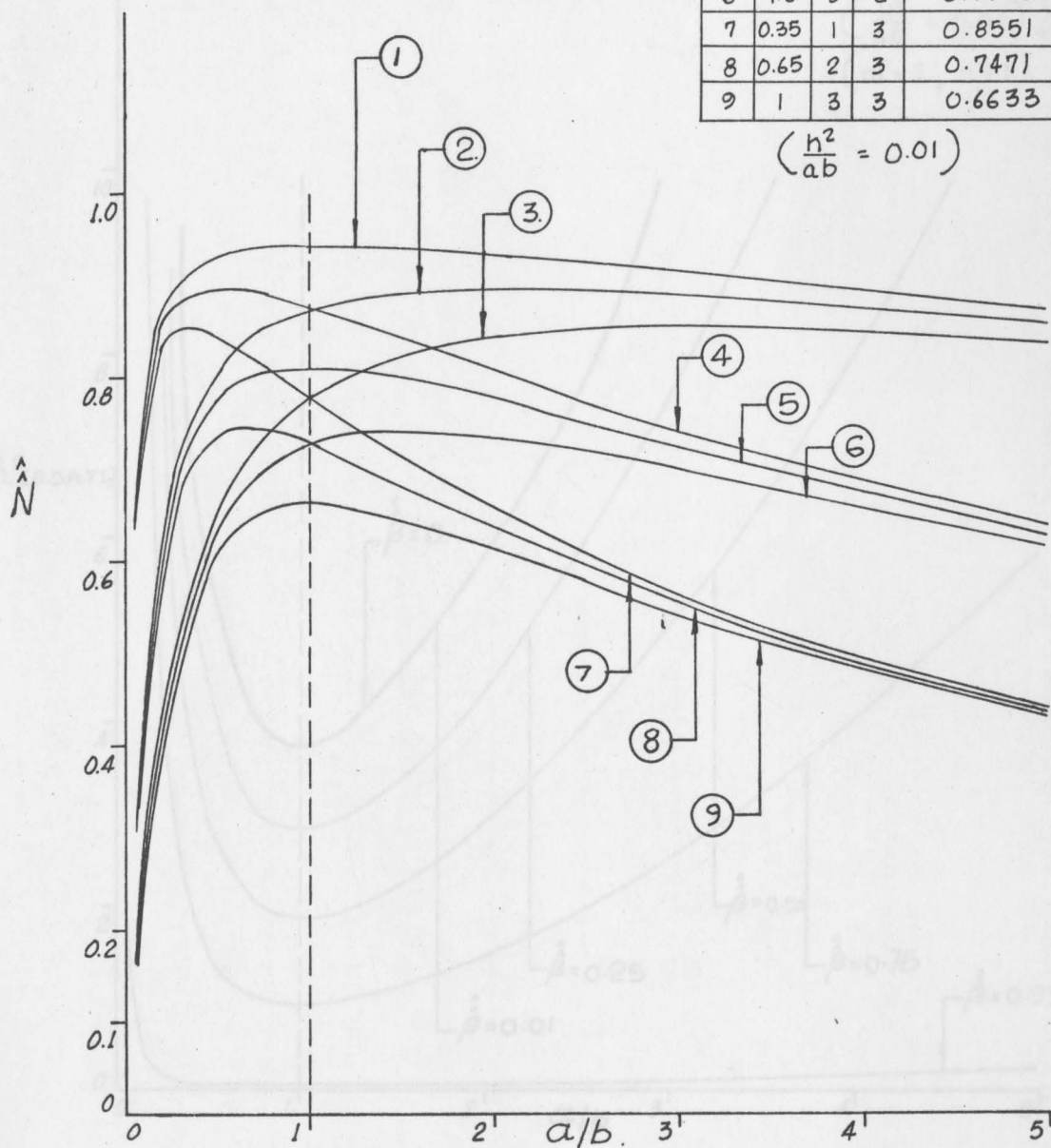


Figure 2-B. Critical Buckling Load Ratio vs.  $a/b$ .

$$\hat{\Omega}_{(B.S.A.T.I.)}^2 = \frac{(1-\hat{\beta}) \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right)^2}{1 + k \frac{h^2}{ab} \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right)^2},$$

where

$$k = \pi^2 / 5(1-\nu), \text{ and}$$

$$\nu = 0.30.$$

$\hat{\beta}$	$\hat{\Omega}_{(B.S.A.T.I.)}^2$ Minimum
0.0	3.786
0.01	3.749
0.25	2.840
0.50	1.893
0.75	0.947
0.99	0.038

$$\left( \frac{h^2}{ab} = 0.01 \right),$$

$$(m=1, n=1)$$

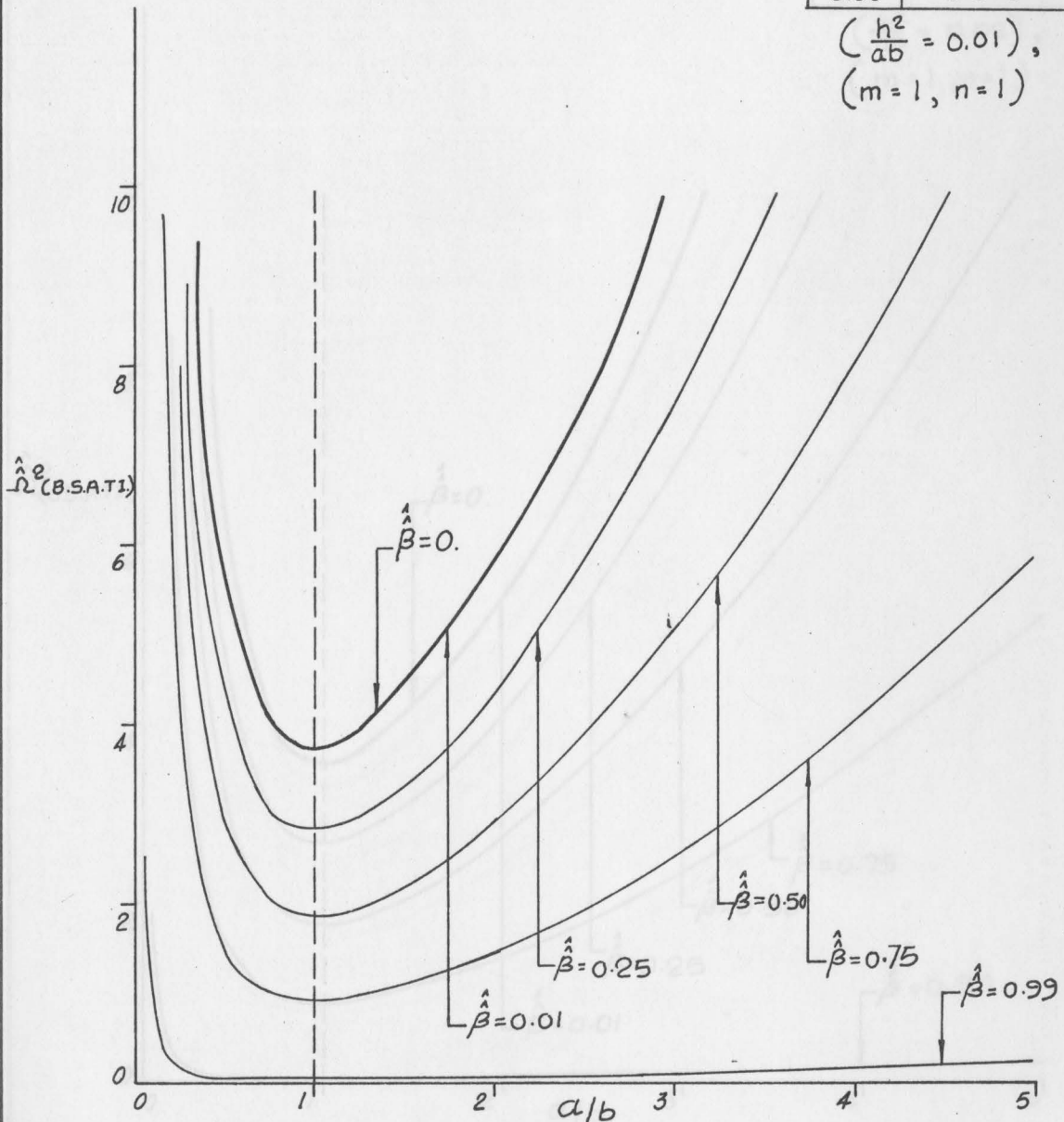


Figure 3. Natural Frequency Including Shear vs.  $a/b$ .

$$\hat{\Omega}^2_{(B.S.A.T.I)} = \frac{(1-\hat{\beta}) \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right)^2}{1 + k \cdot \frac{h^2}{ab} \cdot \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right)}$$

where  
 $k = \pi^2/5(1-\nu)$ , and  
 $\nu = 0.30$ .

$\hat{\beta}$	$\hat{\Omega}^2_{(B.S.A.T.I)}$ Minimum
0.0	3.595
0.01	3.559
0.25	2.696
0.50	1.797
0.75	0.899
0.99	0.036

$\left( \frac{h^2}{ab} = 0.02 \right)$ ,  
 $(m=1, n=1)$

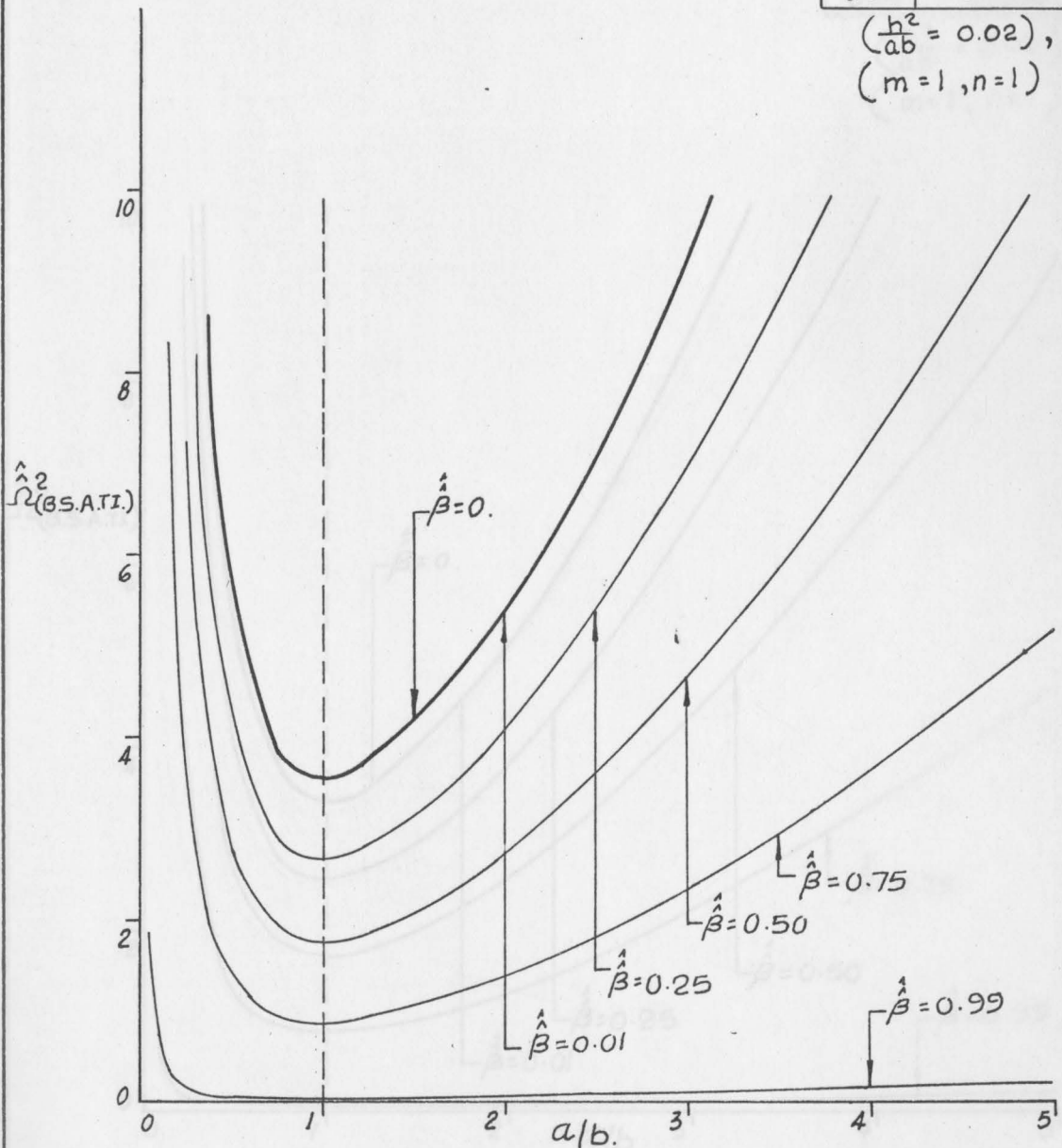


Figure 4. Natural Frequency Including Shear vs.  $a/b$ .

$$\hat{\Omega}^2_{(B.S.A.T.I.)} = \frac{(1-\hat{\beta}) \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right)^2}{1 + k \frac{h^2}{ab} \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right)},$$

where

$$k = \pi^2 / 5(1-\nu), \text{ and}$$

$$\nu = 0.30.$$

$\hat{\beta}$	$\hat{\Omega}^2_{(B.S.A.T.I.)}$ Minimum
0.0	3.421
0.01	3.387
0.25	2.566
0.50	1.711
0.75	0.855
0.99	0.034

$$\left( \frac{h^2}{ab} = 0.03 \right),$$

$$(m=1, n=1)$$

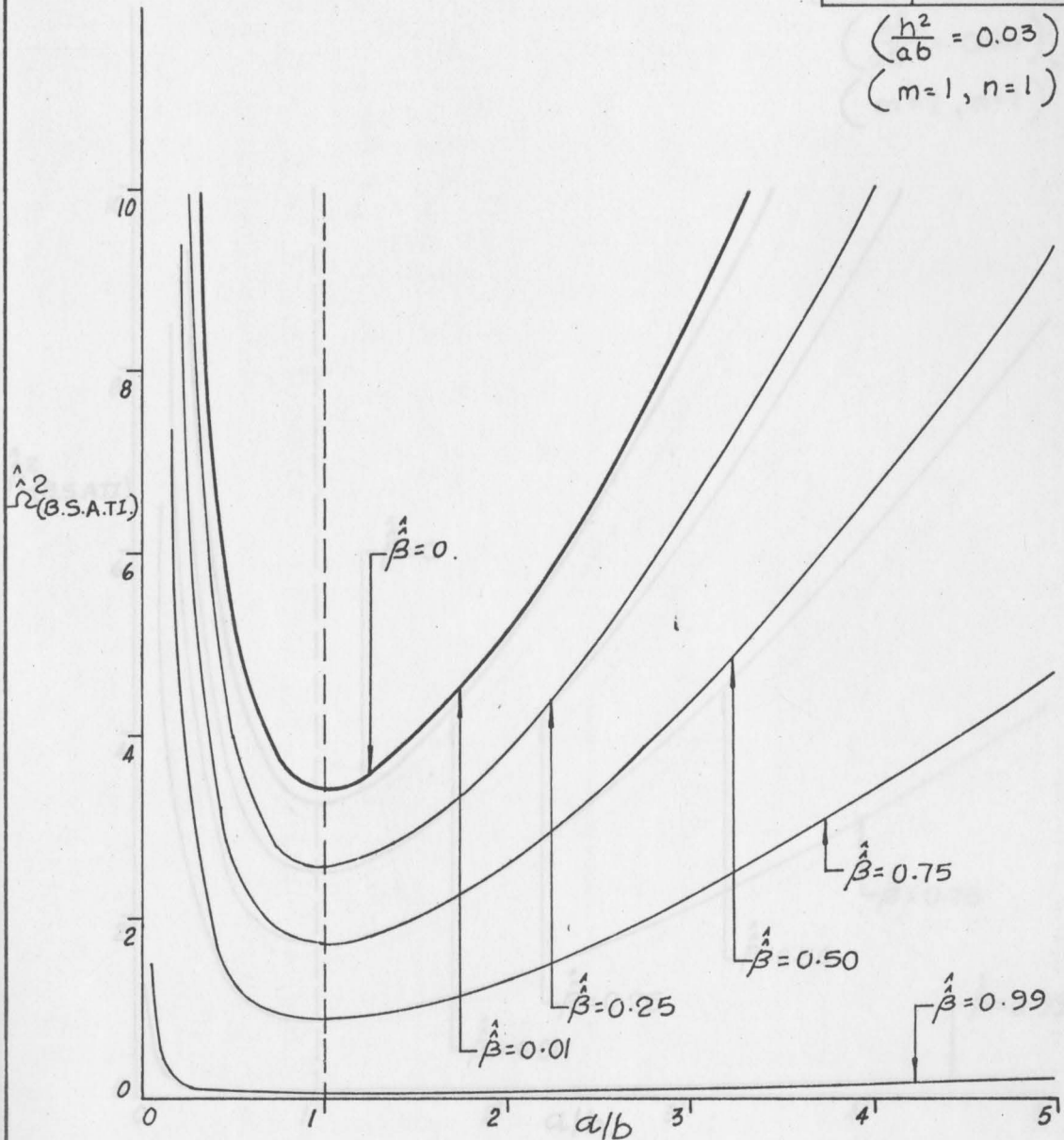


Figure 5. Natural Frequency Including Shear vs.  $a/b$ .

$$\hat{\Omega}_{(B.S.A.T.I.)}^2 = \frac{(1 - \hat{\beta}) \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right)^2}{1 + k \frac{h^2}{ab} \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right)}$$

where  
 $k = \pi^2 / 5(1 - \nu)$ , and  
 $\nu = 0.30$ .

$\hat{\beta}$	$\hat{\Omega}_{(B.S.A.T.I.)}^2$ Minimum
0.0	3.264
0.01	3.231
0.25	2.448
0.50	1.632
0.75	0.816
0.99	0.033

$$\left( \frac{h^2}{ab} = 0.04 \right),$$

$$(m=1, n=1)$$

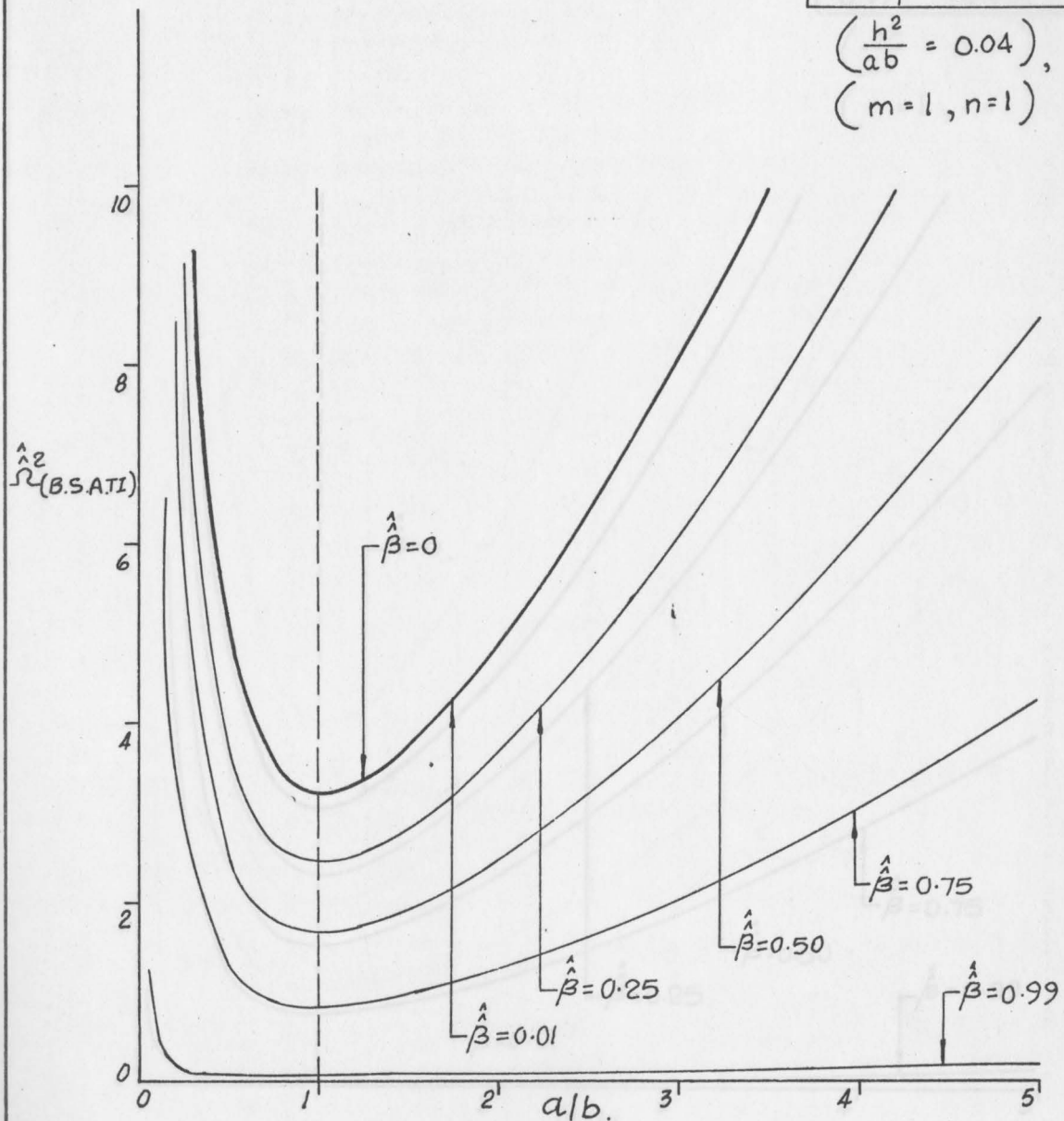


Figure 6. Natural Frequency Including Shear vs.  $a/b$ .



$$\hat{\Omega}_{(B.S.A.T.I.)}^2 = \frac{(1-\hat{\beta}) \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right)^2}{1 + k \frac{h^2}{ab} \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right)},$$

where

$k = \pi^2/5(1-\nu)$ , and

$\nu = 0.30$ .

$\hat{\beta}$	$\hat{\Omega}_{(B.S.A.T.I.)}^2$ Minimum
0.0	3.120
0.01	3.089
0.25	2.340
0.50	1.560
0.75	0.780
0.99	0.031

$$\left( \frac{h^2}{ab} = 0.05 \right),$$

$$(m=1, n=1)$$

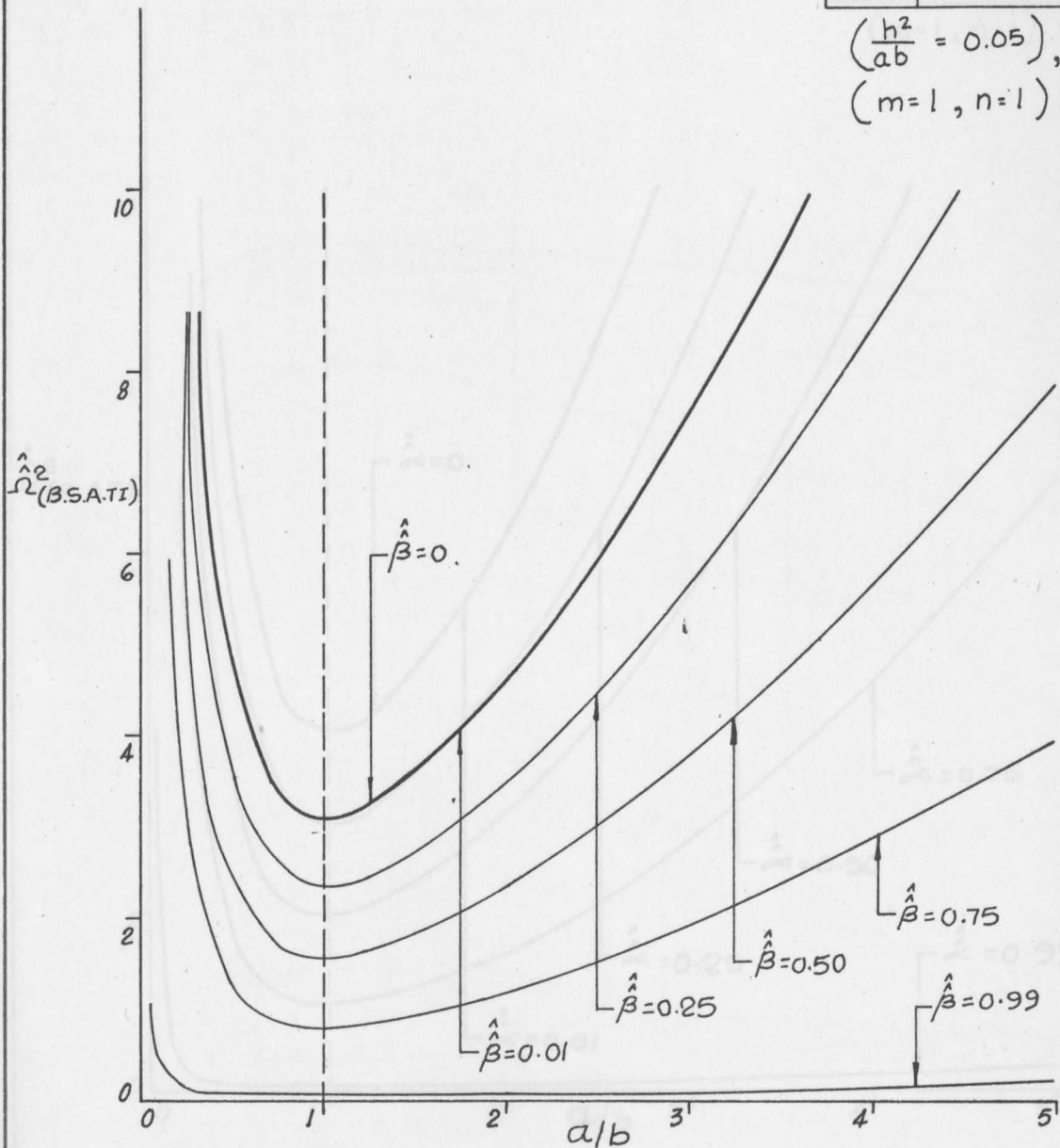


Figure 7. Natural Frequency Including Shear vs.  $a/b$ .

$$\hat{\Omega}^2_{(B.A.T.I.)} = (1 - \hat{\alpha})(m^2 \frac{b}{a} + n^2 \frac{a}{b})^2$$

$\hat{\alpha}$	$\hat{\Omega}^2_{(B.A.T.I.)}$ Minimum
0.0	4.000
0.01	3.960
0.25	3.000
0.50	2.000
0.75	1.000
0.99	0.040

(m=1, n=1)

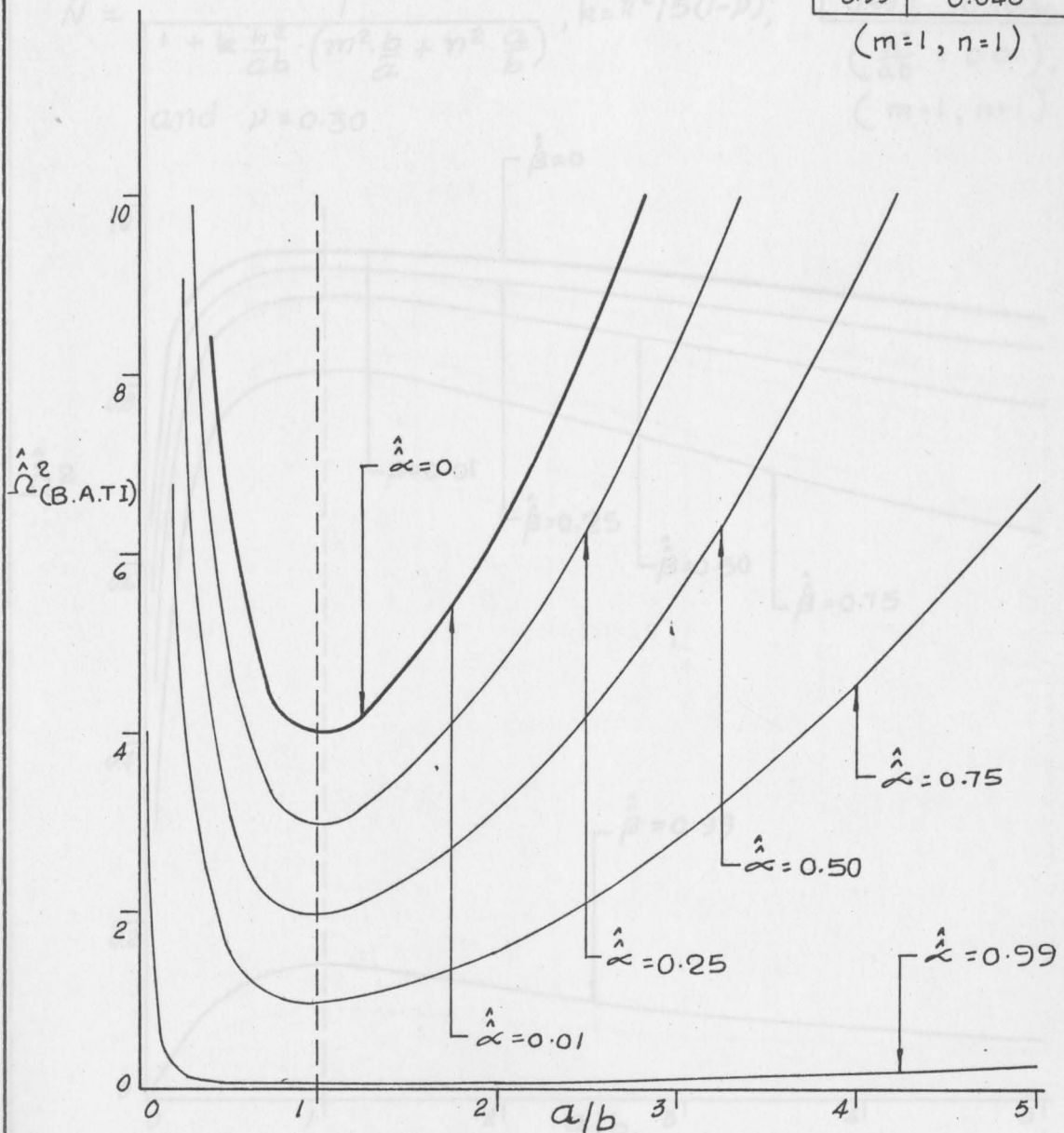


Figure 8. Natural Frequency Excluding Shear vs.  $a/b$ .

$$\hat{\Omega}^2 = \frac{\hat{\Omega}^2_{(B.S.A.T.I.)}}{\hat{\Omega}^2_{(B.A.T.I.)}} = \frac{1}{1 + \hat{\gamma}}, \text{ where}$$

where

$$\hat{\gamma} = \frac{1 - \hat{N}}{\hat{N}(1 - \hat{\beta})},$$

$$\hat{N} = \frac{1}{1 + k \cdot \frac{h^2}{ab} \cdot (m^2 \cdot \frac{b}{a} + n^2 \cdot \frac{a}{b})}, k = \pi^2 / 5(1 - \nu),$$

and  $\nu = 0.30$ .

$\hat{\beta}$	$\hat{\Omega}^2$ Maximum
0.0	0.9466
0.01	0.9461
0.25	0.9301
0.50	0.8986
0.75	0.8159
0.99	0.1506

( $\frac{h^2}{ab} = 0.01$ ),  
( $m=1, n=1$ )

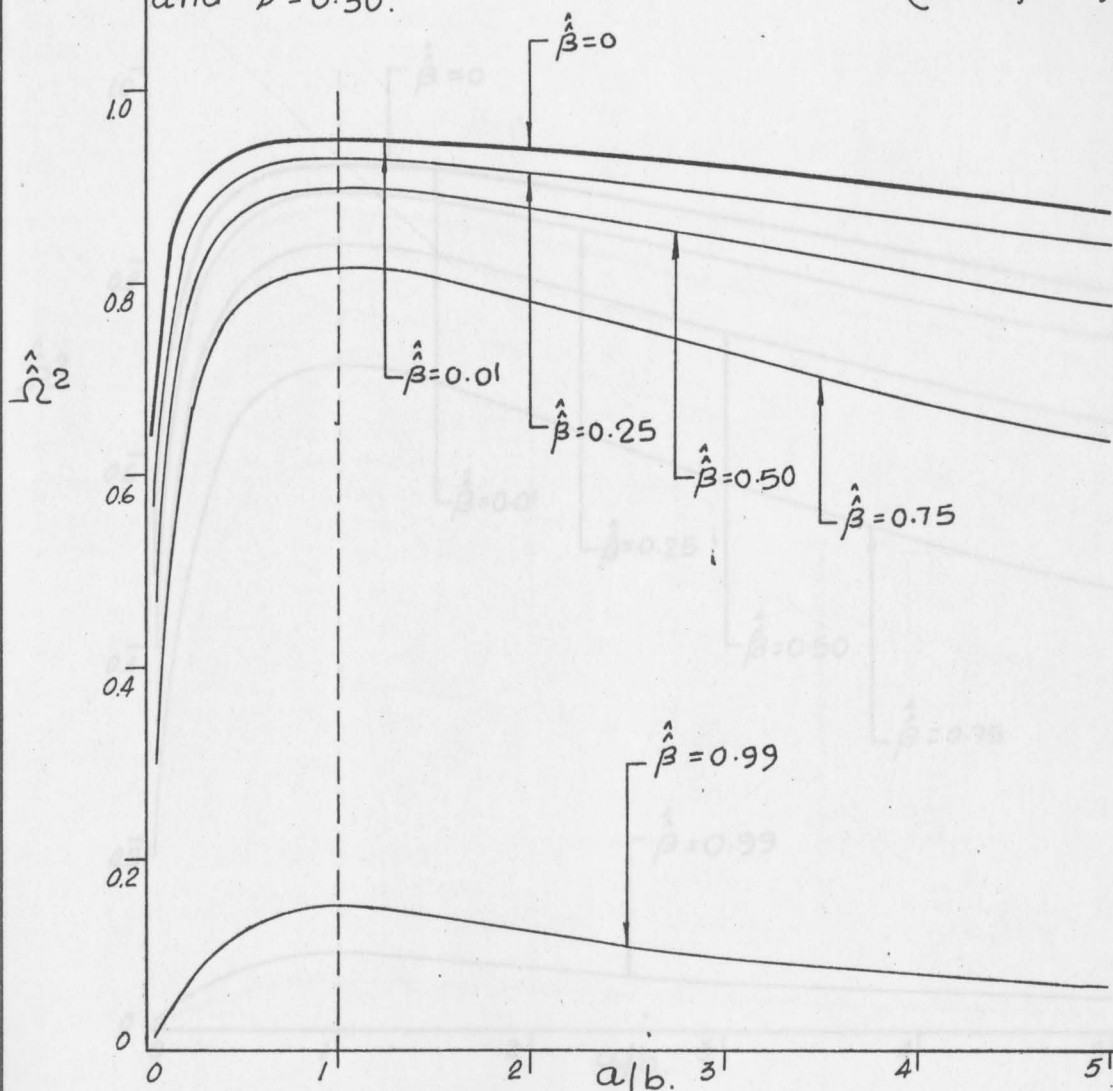


Figure 9. Natural Frequency Ratio vs. a/b.

$$\hat{\Omega}^2 = \frac{\hat{\Omega}^2(\text{B.S.A.T.I.})}{\hat{\Omega}^2(\text{B.A.T.I.})} = \frac{1}{1 + \hat{\gamma}}, \text{ where}$$

$$\hat{\gamma} = \frac{1 - \hat{N}}{\hat{N}(1 - \hat{\beta})}, \quad \hat{N} = \frac{1}{1 + k \cdot \frac{h^2}{ab} \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right)},$$

$$k = \pi^2/5(1 - \nu), \text{ and } \nu = 0.30.$$

$\hat{\beta}$	$\hat{\Omega}^2$ Maximum
0.0	0.8986
0.01	0.8977
0.25	0.8693
0.50	0.8159
0.75	0.6891
0.99	0.0814

( $h^2/ab = 0.02$ ),  
( $m=1, n=1$ ).

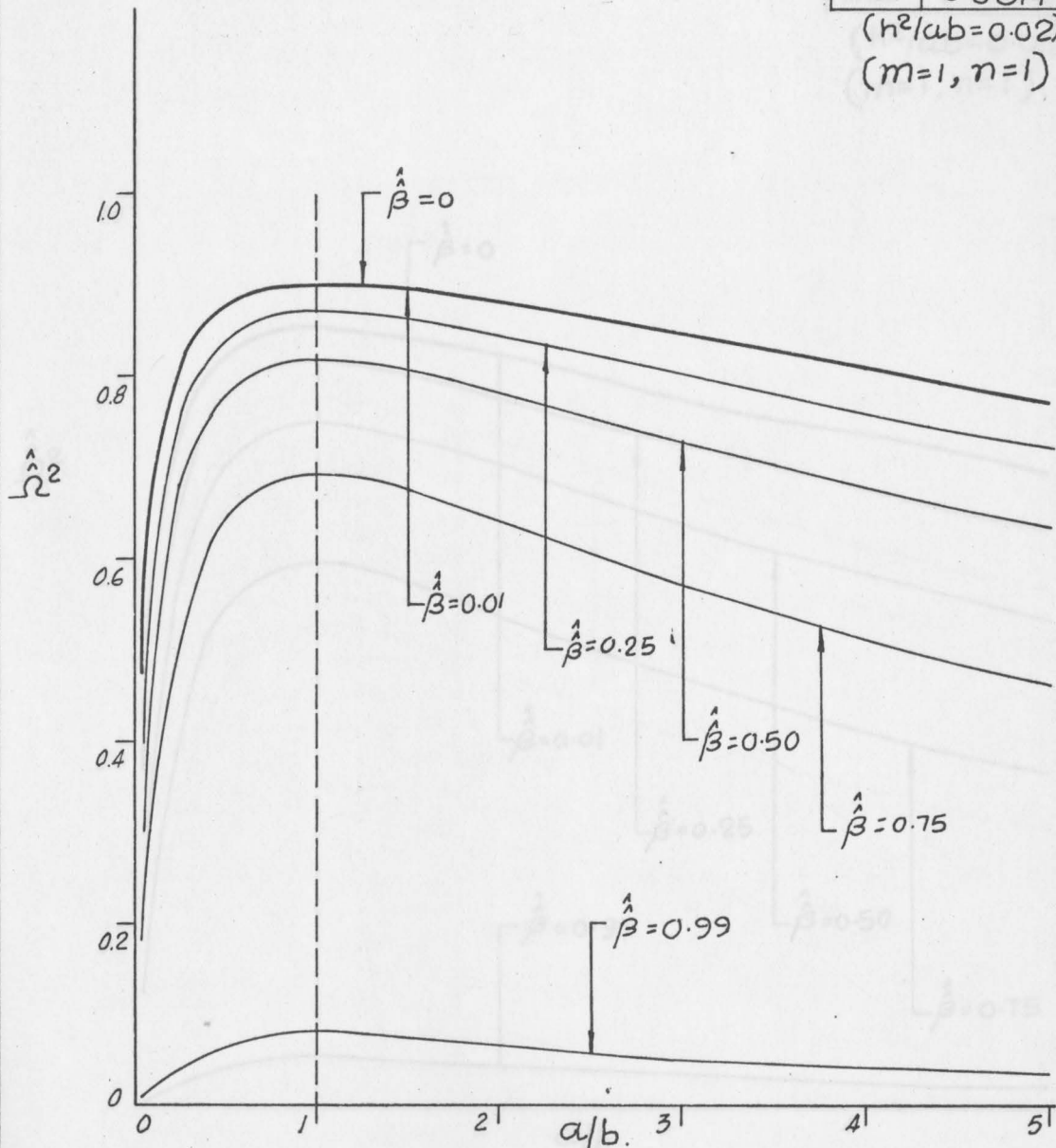


Figure 10. Natural Frequency Ratio vs.  $a/b$ .

$$\hat{\Omega}^2 = \frac{\hat{\Omega}^2_{(B.S.A.T.I)}}{\hat{\Omega}^2_{(B.A.T.I)}} = \frac{1}{1 + \hat{\gamma}}, \text{ where}$$

$$\hat{\gamma} = \frac{1 - \hat{N}}{\hat{N}(1 - \hat{\beta})}, \quad \hat{N} = \frac{1}{1 + k \cdot \frac{h^2}{ab} (m^2 \frac{b}{a} + n^2 \frac{a}{b})},$$

$$k = \pi^2/5(1 - \nu), \text{ and } \nu = 0.30.$$

$\hat{\beta}$	$\hat{\Omega}^2$ Maximum
0.0	0.8553
0.01	0.8540
0.25	0.8159
0.50	0.7472
0.75	0.5964
0.99	0.0558

( $h^2/ab = 0.03$ ),  
( $m=1, n=1$ ).

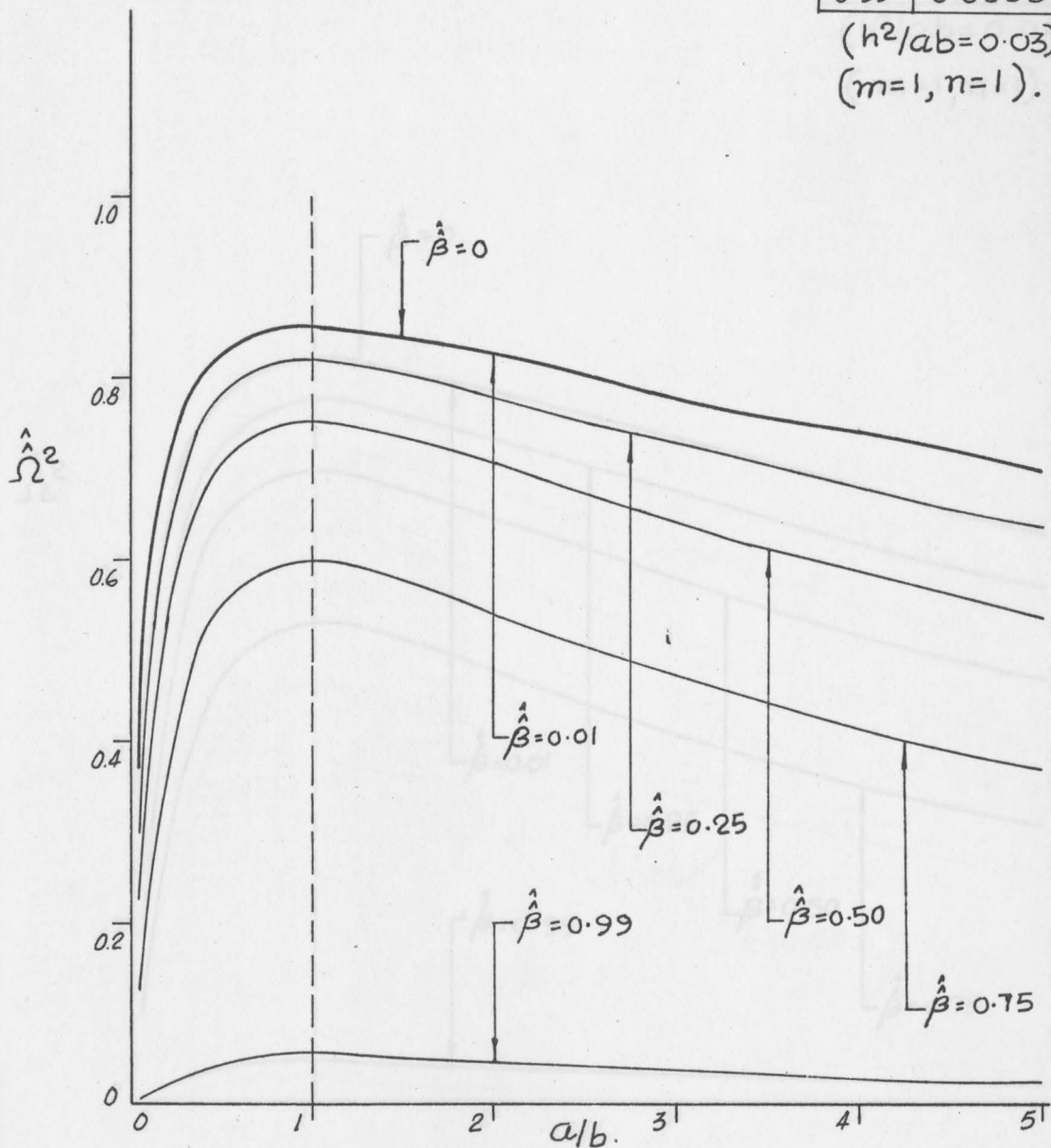


Figure 11. Natural Frequency Ratio vs.  $a/b$ .

$$\hat{\Omega}^2 = \frac{\hat{\Omega}_{(B.S.A.T.I.)}^2}{\hat{\Omega}_{(B.A.T.I.)}^2} = \frac{1}{1 + \hat{\gamma}}, \text{ where}$$

$$\hat{\gamma} = \frac{1 - \hat{N}}{\hat{N}(1 - \hat{\beta})}, \quad \hat{N} = \frac{1}{1 + k \cdot \frac{h^2}{ab} \cdot \left(m^2 \frac{b}{a} + n^2 \frac{a}{b}\right)},$$

$$k = \pi^2/5(1-\nu), \text{ and } \nu = 0.30$$

$\hat{\beta}$	$\hat{\Omega}_{\text{Maximum}}^2$
0.0	0.8159
0.01	0.8144
0.25	0.7688
0.50	0.6891
0.75	0.5257
0.99	0.0424

( $h^2/ab = 0.04$ ),

( $m=1, n=1$ ).

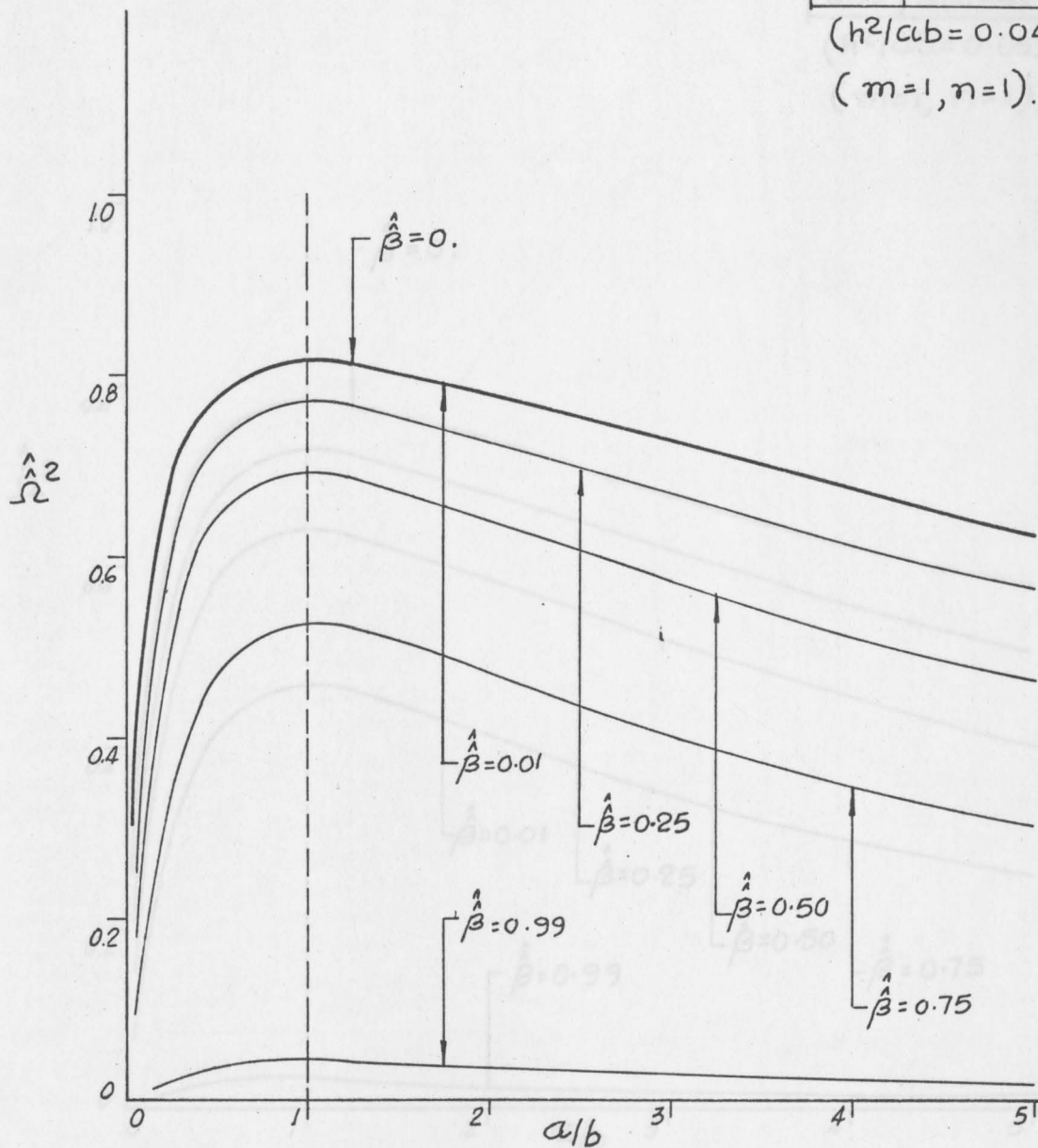


Figure 12. Natural Frequency Ratio vs.  $a/b$ .

$$\hat{\Omega}^2 = \frac{\hat{\Omega}^2_{(B.S.A.T.I.)}}{\hat{\Omega}^2_{(B.A.T.I.)}} = \frac{1}{1 + \hat{\gamma}}, \text{ where}$$

$$\hat{\gamma} = \frac{1 - \hat{N}}{\hat{N}(1 - \hat{\beta})}, \quad \hat{N} = \frac{1}{1 + k \cdot \frac{h^2}{ab} \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right)},$$

$$k = \pi^2/5(1 - \nu), \text{ and } \nu = 0.30.$$

$\hat{\beta}$	$\hat{\Omega}^2_{\text{Maximum}}$
0.0	0.7800
0.01	0.7783
0.25	0.7268
0.50	0.6394
0.75	0.4699
0.99	0.0342

$$(h^2/ab = 0.05),$$

$$(m=1, n=1).$$

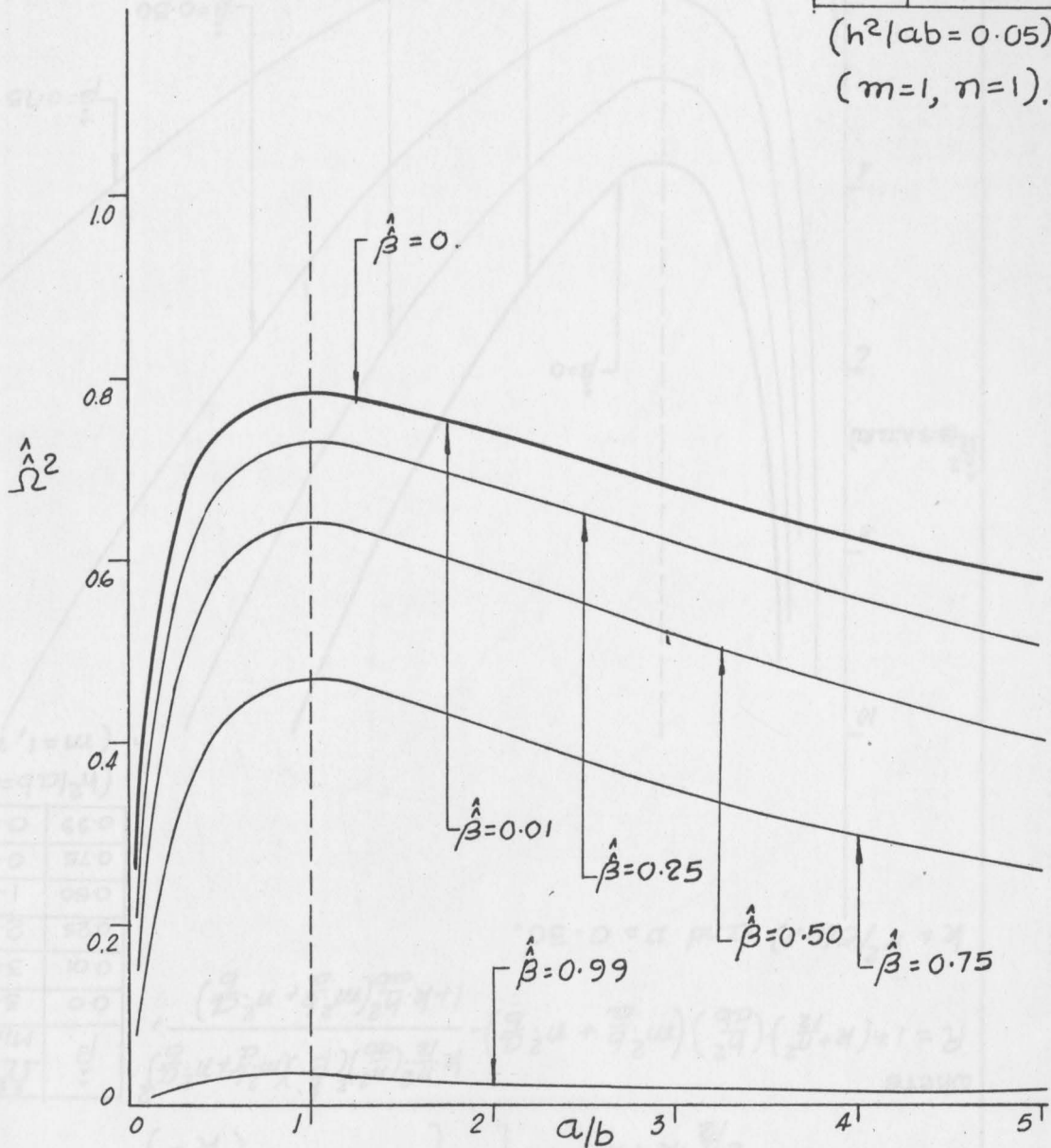


Figure 13. Natural Frequency Ratio vs.  $a/b$ .

$$\hat{\Omega}_{(B.S.A.T.I.R.I.)}^2 = \frac{1}{2 \cdot \frac{\pi^2}{12} k \left(\frac{ab}{h^2}\right)^2 (R)} \left[ 1 - \left[ 1 - \frac{4 \cdot \frac{\pi^2}{12} k \left(\frac{h^2}{ab}\right)^2 (1 - \hat{\beta}) (m^2 \frac{b}{a} + n^2 \frac{a}{b})^2}{(R^2)} \right]^{1/2} \right],$$

where

$$R = 1 + (k + \frac{\pi^2}{12}) \cdot \left(\frac{h^2}{ab}\right) \cdot (m^2 \frac{b}{a} + n^2 \frac{a}{b}) - \frac{k \cdot \frac{\pi^2}{12} \left(\frac{h^2}{ab}\right)^2 (\hat{\beta}) (m^2 \frac{b}{a} + n^2 \frac{a}{b})^2}{1 + k \cdot \frac{h^2}{ab} (m^2 \frac{b}{a} + n^2 \frac{a}{b})}$$

$$k = \frac{\pi^2}{5(1-\nu)} \text{ and } \nu = 0.30.$$

$\hat{\beta}$	$\hat{\Omega}_{(B.S.A.T.I.R.I.)}^2$ Minimum
0.0	3.732
0.01	3.694
0.25	2.799
0.50	1.866
0.75	0.933
0.99	0.037

$(h^2/ab=0.01),$   
 $(m=1, n=1).$

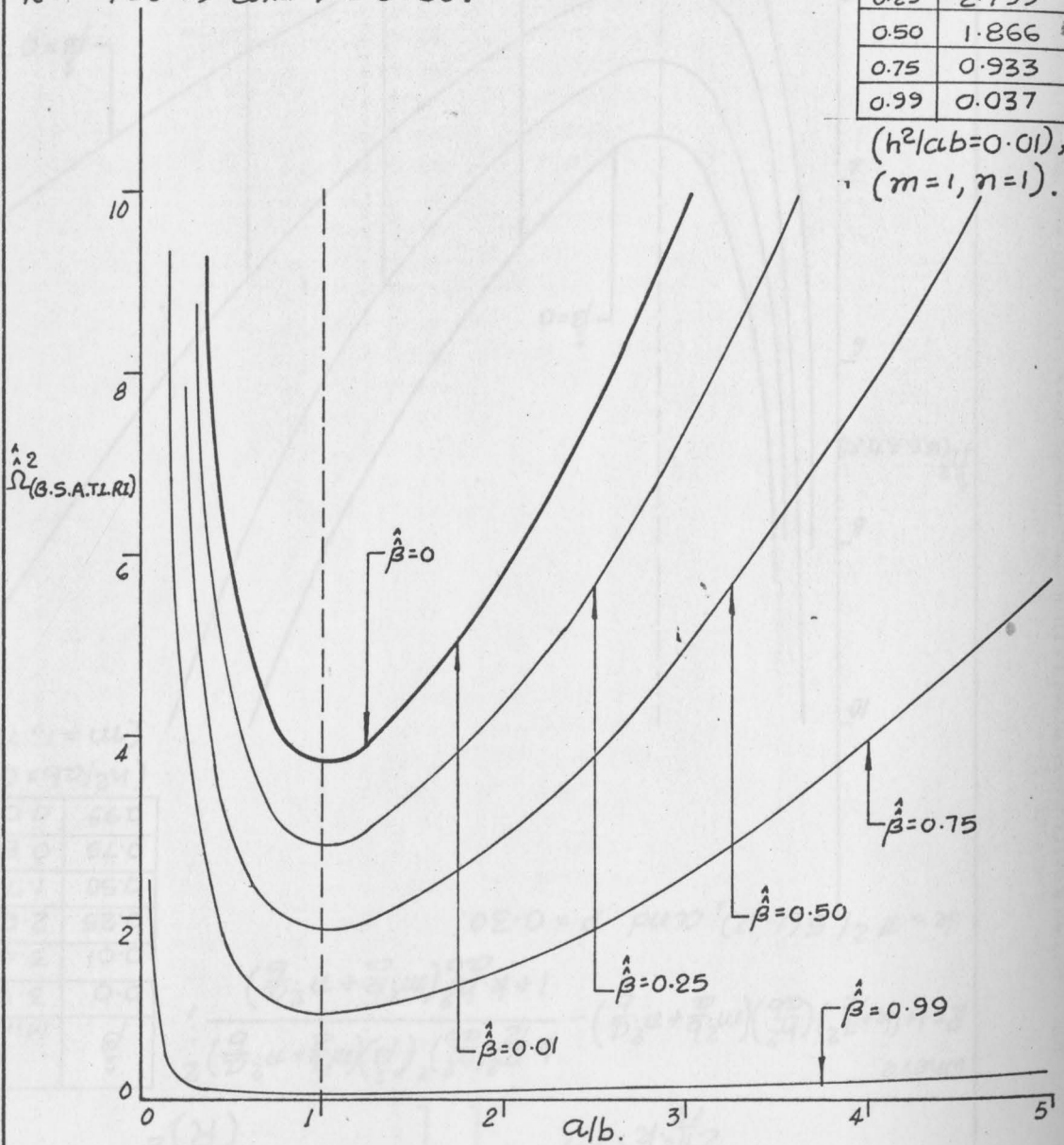


Figure 14. Lower Set of Natural Frequency Including Shear and Rotary Inertia vs. a/b.



$$\hat{\Omega}_{(B.S.A.T.I.R.I.)}^2 = \frac{1}{2 \cdot \frac{\pi^2}{12} \cdot k} \left( \frac{ab}{h^2} \right)^2 (R) \left[ 1 - \frac{4 \cdot \frac{\pi^2}{12} \cdot k \cdot \left( \frac{h^2}{ab} \right)^2 (1 - \hat{\beta}) \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right)^2}{(R)^2} \right]^{1/2}$$

where

$$R = 1 + \left( k + \frac{\pi^2}{12} \right) \left( \frac{h^2}{ab} \right) \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right) - \frac{k \cdot \frac{\pi^2}{12} \left( \frac{h^2}{ab} \right)^2 (\hat{\beta}) \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right)^2}{1 + k \cdot \frac{h^2}{ab} \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right)}$$

$k = \pi^2 / 5(1 - \nu)$ , and  $\nu = 0.30$ .

$\hat{\beta}$	$\hat{\Omega}_{(B.S.A.T.I.R.I.)}^2$ Minimum
0.0	3.501
0.01	3.466
0.25	2.626
0.50	1.751
0.75	0.875
0.99	0.035

$(h^2/ab = 0.02)$ ,  
 $(m=1, n=1)$ .

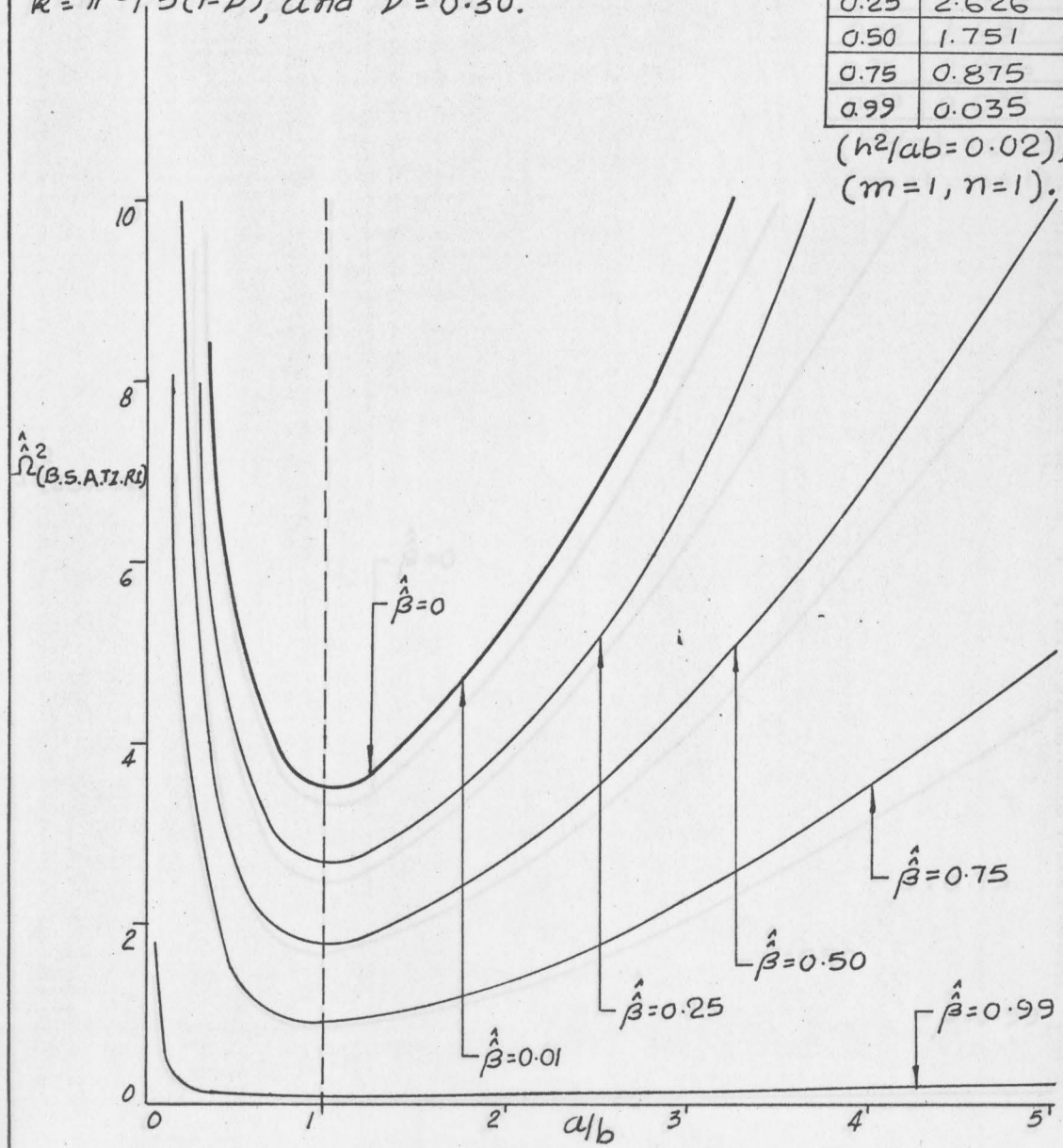


Figure 15. Lower Set Of Natural Frequency Including Shear And Rotary Inertia vs. a/b.

$$\hat{\Omega}_{(B.S.A.T.I.R.I.)}^2 = \frac{1}{2 \cdot \frac{\pi^2}{12} \cdot k} \left( \frac{ab}{h^2} \right)^2 (R) \left[ 1 - \left\{ 1 - \frac{4 \cdot \frac{\pi^2}{12} \cdot k \cdot \left( \frac{h^2}{ab} \right)^2 (1 - \hat{\beta}) \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right)^2}{(R^2)} \right\}^{1/2} \right]$$

where

$$R = 1 + \left( k + \frac{\pi^2}{12} \right) \left( \frac{h^2}{ab} \right) \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right) - \frac{k \cdot \frac{\pi^2}{12} \cdot \left( \frac{h^2}{ab} \right)^2 \cdot \hat{\beta} \cdot \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right)^2}{1 + k \cdot \frac{h^2}{ab} \cdot \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right)}$$

$k = \pi^2/5(1-\nu)$ , and  $\nu = 0.30$ .

$\hat{\beta}$	$\hat{\Omega}_{(B.S.A.T.I.R.I.)}^2$ Minimum
0.0	3.301
0.01	3.268
0.25	2.476
0.50	1.651
0.75	0.826
0.99	0.033

$(h^2/ab = 0.03)$ ,  
 $(m=1, n=1)$ .

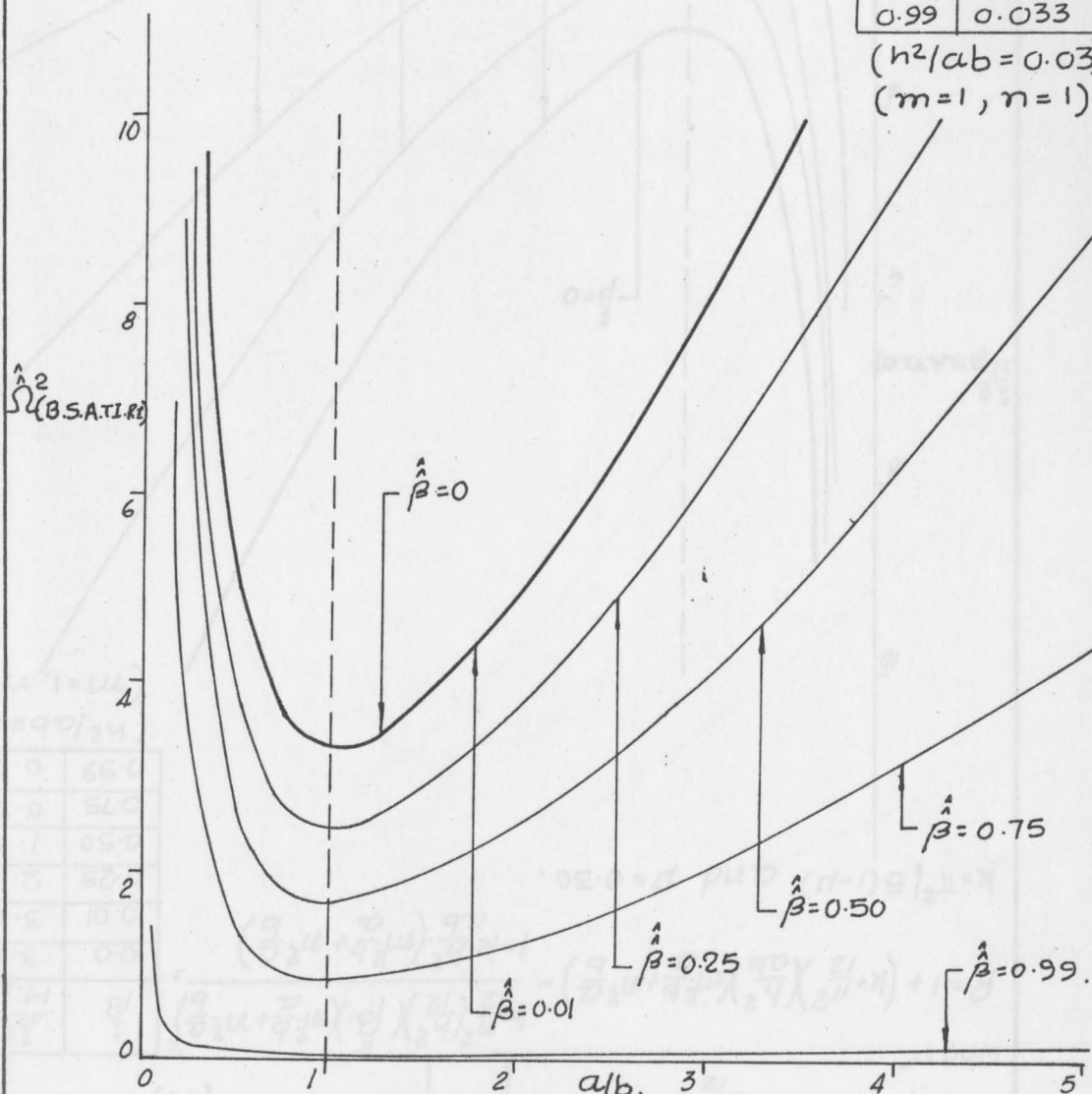


Figure 16. Lower Set Of Natural Frequency Including Shear And Rotary Inertia vs.  $a/b$ .

$$\hat{\Omega}^2_{(B.S.A.T.I.R.I.)} = \frac{1}{2 \cdot \frac{\pi^2 k}{12}} \left( \frac{ab}{h^2} \right)^2 (R) \left[ 1 - \left\{ 1 - \frac{4 \cdot \frac{\pi^2 k}{12} \cdot \left( \frac{h^2}{ab} \right)^2 (1 - \hat{\beta}) (m^2 \frac{b}{a} + n^2 \frac{a}{b})^2}{(R)^2} \right\}^{1/2} \right]$$

where

$$R = 1 + \left( k + \frac{\pi^2}{12} \right) \left( \frac{h^2}{ab} \right) \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right) - \frac{k \cdot \frac{\pi^2}{12} \left( \frac{h^2}{12} \right) \left( \hat{\beta} \right) \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right)^2}{1 + k \cdot \frac{h^2}{ab} \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right)}$$

$k = \pi^2 [5(1-\nu)]$  and  $\nu = 0.30$ .

$\hat{\beta}$	$\hat{\Omega}^2_{(B.S.A.T.I.R.I.)}$ Minimum
0.0	3.126
0.01	3.094
0.25	2.345
0.50	1.563
0.75	0.782
0.99	0.031

$(h^2/ab = 0.04)$   
 $(m=1, n=1)$ .

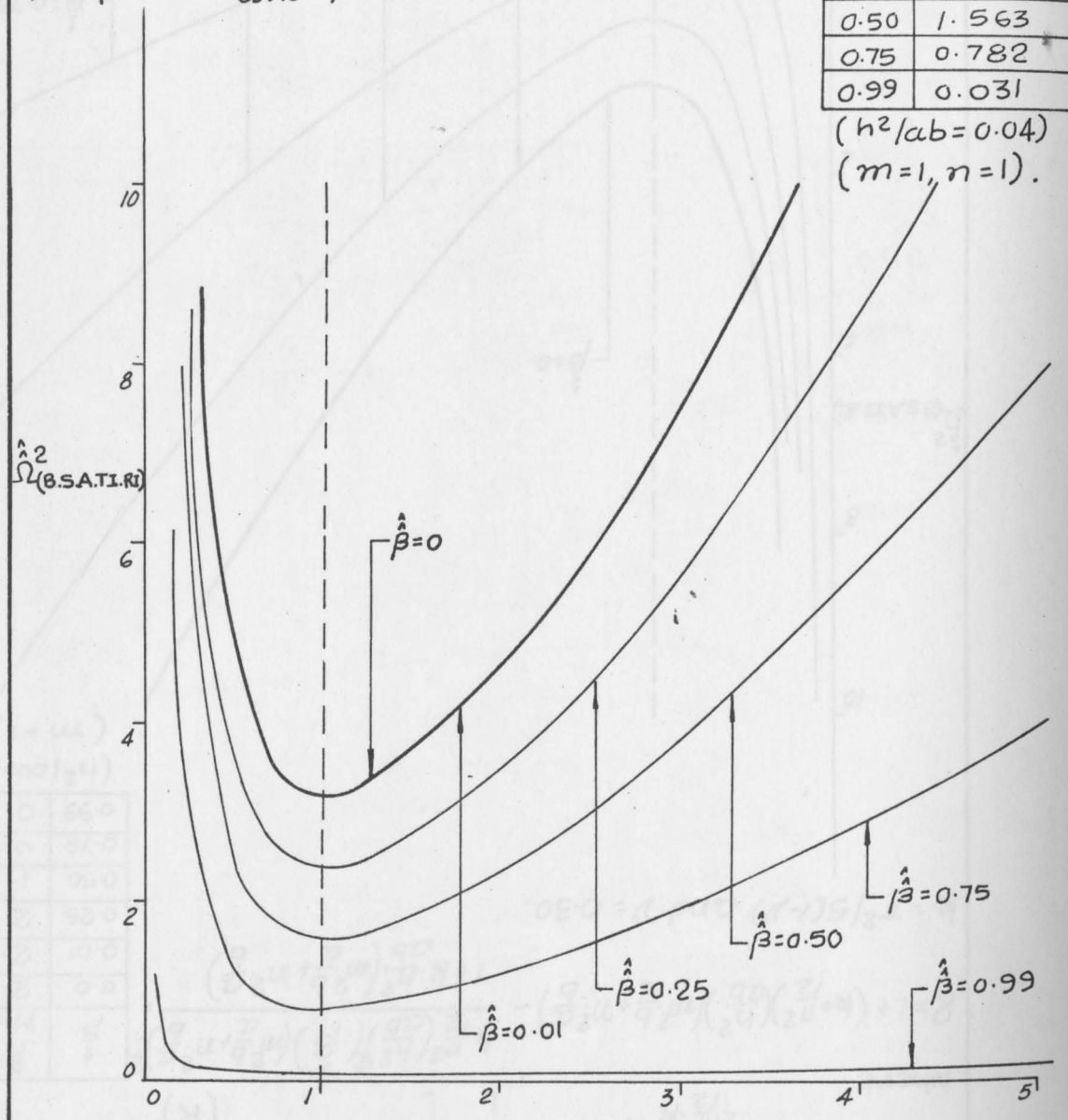


Figure 17. Lower Set Of Natural Frequency Including Shear And Rotary Inertia vs. a/b.

$$\hat{\Omega}_{(B.S.A.T.I.R.I.)}^2 = \frac{1}{2 \cdot \frac{\pi^2}{12} k} \left( \frac{ab}{h^2} \right)^2 (R) \left[ 1 - \left\{ 1 - \frac{4 \cdot \frac{\pi^2}{12} k \left( \frac{h^2}{ab} \right)^2 (1 - \hat{\beta}) \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right)^2}{(R)^2} \right\}^{1/2} \right]$$

where

$$R = 1 + \left( k + \frac{\pi^2}{12} \right) \left( \frac{h^2}{ab} \right) \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right) - \frac{k \frac{\pi^2}{12} \left( \frac{h^2}{ab} \right)^2 \hat{\beta} \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right)^2}{1 + k \cdot \frac{h^2}{ab} \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right)}$$

$k = \pi^2/5(1-\nu)$  and  $\nu = 0.30$ .

$\hat{\beta}$	$\hat{\Omega}_{(B.S.A.T.I.R.I.)}^2$ Minimum
0.0	2.970
0.01	2.940
0.25	2.228
0.50	1.485
0.75	0.743
0.99	0.030

$(h^2/ab = 0.05),$   
 $(m = 1, n = 1).$

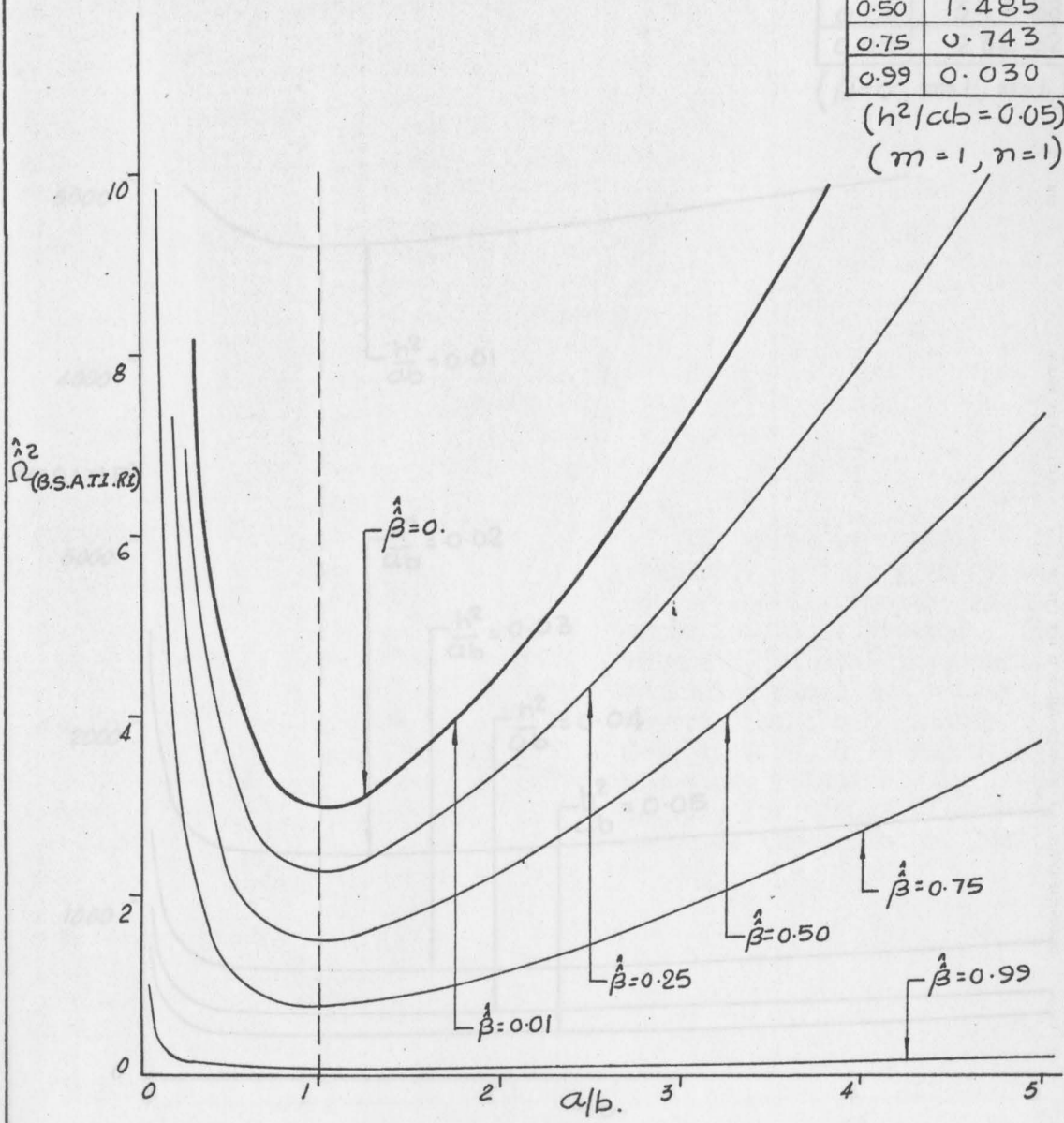


Figure 18. Lower Set Of Natural Frequency Including Shear and Rotary Inertia vs. a/b.

$$\hat{\Omega}_{(B.S.A.T.I.R.I.)}^2 = \frac{1}{2 \cdot \frac{\pi^2}{12} \cdot k} \left( \frac{ab}{h^2} \right)^2 (R) \cdot \left[ 1 + \left| 1 - \frac{4 \cdot \frac{\pi^2}{12} \cdot k \cdot \left( \frac{h^2}{ab} \right)^2 (1-\hat{\beta}) \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right)^2}{(R^2)} \right|^{1/2} \right]$$

where

$$R = 1 + \left( k + \frac{\pi^2}{12} \right) \frac{h^2}{ab} \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right) - \frac{k \cdot \frac{\pi^2}{12} \left( \frac{h^2}{ab} \right)^2 \hat{\beta} \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right)^2}{1 + k \cdot \frac{h^2}{ab} \left( m^2 \frac{b}{a} + n^2 \frac{a}{b} \right)}$$

$$k = \pi^2 / 5(1-\nu), \text{ and } \nu = 0.30.$$

$h^2/ab$	$\hat{\Omega}_{(B.S.A.T.I.R.I.)}^2$ Minimum
0.01	4622.08
0.02	1231.48
0.03	580.48
0.04	344.88
0.05	232.32

$$(\hat{\beta} = 0, m=1, n=1).$$

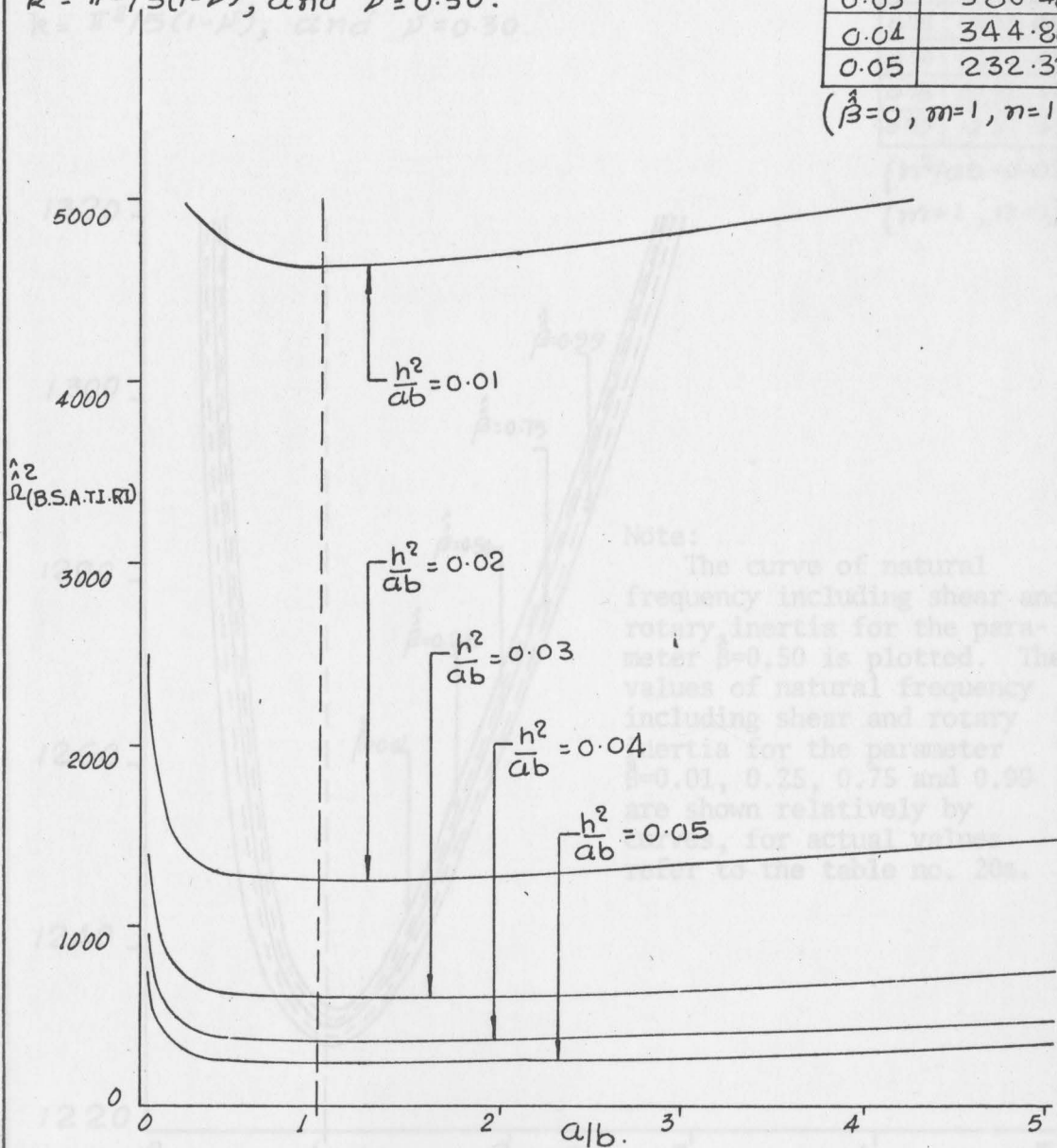


Figure 19. Higher Set Of Natural Frequency Including Shear And Rotary Inertia vs.  $a/b$ .

$$\hat{\Omega}_{(B.S.A.T.I.R.I.)}^2 = \frac{1}{2 \cdot \frac{\pi^2}{12} \cdot k} \cdot \left(\frac{ab}{h^2}\right)^2 \cdot (R) \cdot \left[ 1 + \left\{ 1 - \frac{4 \cdot \frac{\pi^2}{12} \cdot k \cdot \left(\frac{h^2}{ab}\right)^2 \cdot (1-\hat{\beta}) \cdot \left(m^2 \frac{b}{a} + n^2 \frac{a}{b}\right)^2}{(R^2)} \right\}^{\frac{1}{2}} \right]$$

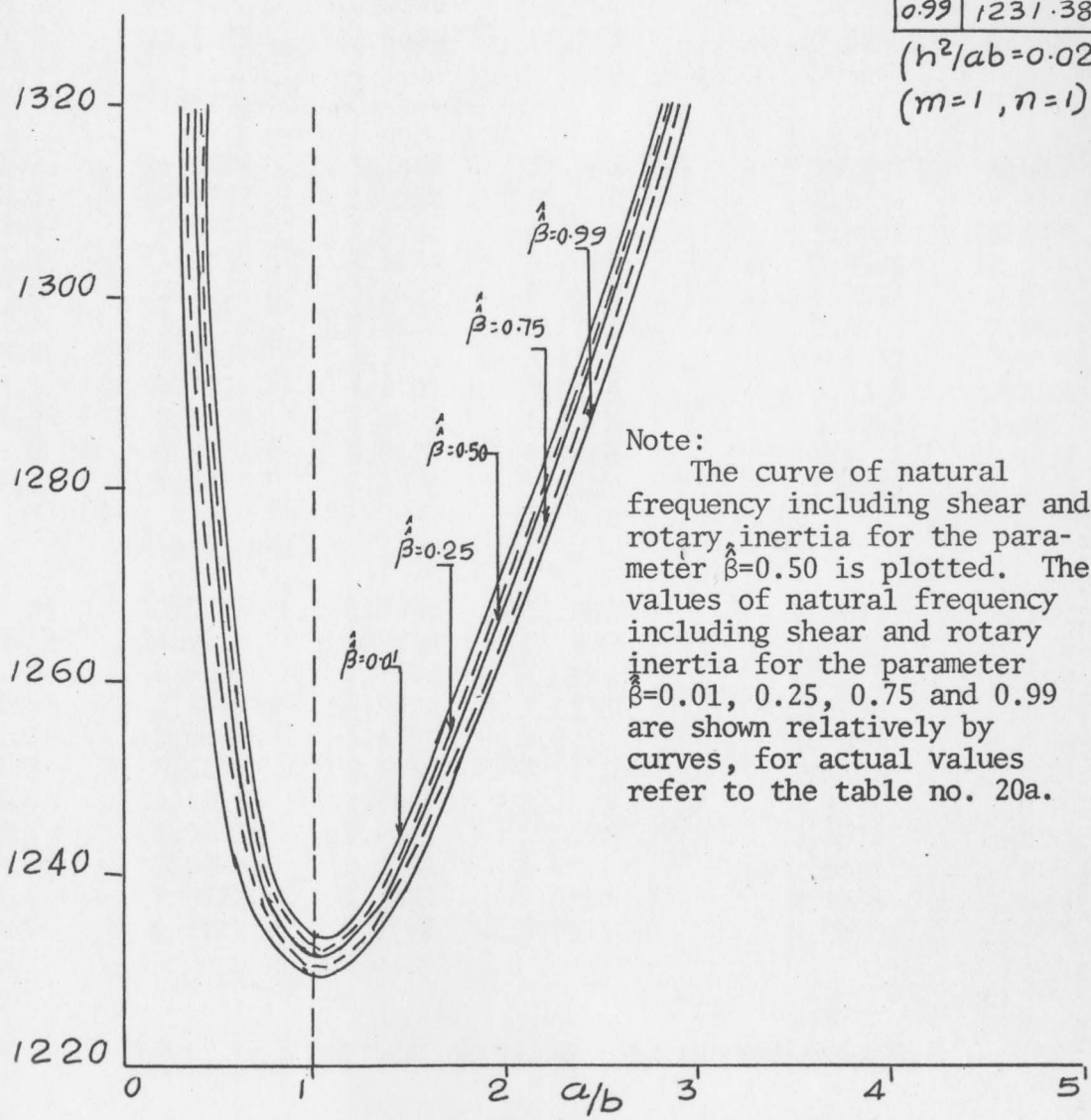
where

$$R = 1 + \left(k + \frac{\pi^2}{12}\right) \cdot \frac{h^2}{ab} \cdot \left(m^2 \frac{b}{a} + n^2 \frac{a}{b}\right) - \frac{k \cdot \frac{\pi^2}{12} \cdot \left(\frac{h^2}{ab}\right)^2 \cdot \hat{\beta} \cdot \left(m^2 \frac{b}{a} + n^2 \frac{a}{b}\right)^2}{1 + k \cdot \frac{h^2}{ab} \cdot \left(m^2 \frac{b}{a} + n^2 \frac{a}{b}\right)}$$

$k = \pi^2/5(1-\nu)$ , and  $\nu = 0.30$ .

$\hat{\beta}$	$\hat{\Omega}_{(B.S.A.T.I.R.I.)}^2$ Minimum
0.01	1231.48
0.25	1231.45
0.50	1231.43
0.75	1231.41
0.99	1231.38

$(h^2/ab=0.02)$ ,  
 $(m=1, n=1)$ .



Note:  
The curve of natural frequency including shear and rotary inertia for the parameter  $\hat{\beta}=0.50$  is plotted. The values of natural frequency including shear and rotary inertia for the parameter  $\hat{\beta}=0.01, 0.25, 0.75$  and  $0.99$  are shown relatively by curves, for actual values refer to the table no. 20a.

Figure 20. Higher Set Of Natural Frequency Including Shear And Rotary Inertia vs. a/b.

## APPENDIX B.

## TABULAR RESULTS

$\frac{Q}{D^2}$	$\frac{h^2}{ab}$	0.01	0.02	0.03	0.04	0.05
(m=1, n=1)						
0.25		16.1295	14.570	13.286	12.210	11.285
0.5		5.8384	5.4777	5.159	4.8752	4.6211
1.0		3.7864	3.5945	3.4212	3.2637	3.1201
1.5		4.4241	4.1832	3.967	3.7725	3.5950
2.0		5.8384	5.4776	5.1589	4.8752	4.6211
2.5		7.7741	7.2278	6.753	6.3370	5.9692
3.0		10.1563	9.3527	8.6670	8.0749	7.5195
3.5		12.9490	11.8099	10.855	10.042	9.3439
4.0		16.1291	14.5699	13.286	12.2091	11.284
4.5		19.6785	17.6092	15.934	14.549	13.386
5.0		23.5815	20.9078	18.779	17.0431	15.901
(m=2, n=1)						
0.25		45.2708	34.445	27.800	23.303	20.058
0.5		14.5702	12.210	10.507	9.2214	8.2160
1.0		5.4777	4.8752	4.3922	3.9962	3.6632
1.5		3.8539	3.5144	3.2091	2.9526	2.7341
2.0		3.5945	3.2637	2.9887	2.7564	2.5576
2.5		3.7670	3.4132	3.1202	2.8736	2.6630
3.0		4.1832	3.7725	3.4331	3.1532	2.9140
3.5		4.7651	4.2707	3.8692	3.5368	3.2508
4.0		5.4776	4.8752	4.3921	3.9962	3.6637
4.5		6.3022	5.5678	4.9666	4.5153	4.1254
5.0		7.2277	6.3370	5.6417	5.0839	4.6265
(m=3, n=1)						
0.25		72.2016	47.959	35.904	28.692	23.892
0.50		24.9606	18.610	14.825	12.320	10.539
1.0		8.6671	7.1044	6.0191	5.2215	4.6105
1.50		5.1590	4.3922	3.8239	3.3858	3.0377
2.0		3.9673	3.4352	3.0289	2.7086	2.4495
2.5		3.5277	3.0762	2.7271	2.4492	2.2227
3.0		3.4212	2.9887	2.6533	2.3855	2.1660
3.5		3.4971	3.0511	2.7059	2.4310	2.2067
4.0		3.6899	3.2091	2.8391	2.5456	2.3071
4.5		3.9672	3.4351	3.0289	2.7086	2.4495
5.0		4.3111	3.7136	3.2815	2.9076	2.6229

Table Ia. CRITICAL BUCKLING LOAD INCLUDING SHEAR

$\frac{a}{b} \backslash h^2/ab.$	$(\hat{N}_{B.S.A.})_{CR}$				
	0.01	0.02	0.03	0.04	0.05
(m=1, n=1)					
0.25	16.1295	14.570	13.286	12.210	11.295
0.5	5.8384	5.4777	5.159	4.8752	4.6211
1.0	3.7864	3.5945	3.4212	3.2637	3.1201
1.5	4.4241	4.1832	3.967	3.7725	3.5959
2.0	5.8384	5.4776	5.1589	4.8752	4.6211
2.5	7.7741	7.2278	6.753	6.3370	5.9692
3.0	10.1563	9.3527	8.6670	8.0749	7.5586
3.5	12.9490	11.8099	10.855	10.043	9.3439
4.0	16.1291	14.5699	13.286	12.2093	11.294
4.5	19.6785	17.6092	15.934	14.549	13.386
5.0	23.5815	20.9078	18.779	17.0431	15.601
(m=2, n=1)					
0.25	45.2708	34.446	27.800	23.303	20.058
0.5	14.5702	12.210	10.507	9.2214	8.2160
1.0	5.4777	4.8752	4.3922	3.9962	3.6657
1.5	3.8839	3.5144	3.2091	2.9526	2.7341
2.0	3.5945	3.2637	2.9887	2.7564	2.5576
2.5	3.7670	3.4132	3.1202	2.8736	2.6630
3.0	4.1832	3.7725	3.4351	3.1532	2.9140
3.5	4.7651	4.2707	3.8692	3.5368	3.2569
4.0	5.4776	4.8752	4.3921	3.9962	3.6657
4.5	6.3022	5.5678	4.9866	4.5153	4.1254
5.0	7.2277	6.3370	5.6417	5.0839	4.6265
(m=3, n=1)					
0.25	72.2016	47.959	35.904	28.692	23.892
0.50	24.9906	18.610	14.825	12.320	10.539
1.0	8.6671	7.1044	6.0191	5.2215	4.6105
1.50	5.1590	4.3922	3.8239	3.3858	3.0377
2.0	3.9673	3.4352	3.0289	2.7086	2.4495
2.5	3.5277	3.0762	2.7271	2.4492	2.2227
3.0	3.4212	2.9887	2.6533	2.3855	2.1669
3.5	3.4971	3.0511	2.7059	2.4310	2.2067
4.0	3.6899	3.2091	2.8391	2.5456	2.3071
4.5	3.9672	3.4351	3.0289	2.7086	2.4495
5.0	4.3111	3.7136	3.2615	2.9076	2.6229

Table 1a. CRITICAL BUCKLING LOAD INCLUDING SHEAR



$\hat{N}$   
 (N.B.S.A.)CR.

$\frac{a}{b} \cdot \frac{h^2}{ab}$	0.01	0.02	0.03	0.04	0.05
(m=1, n=2)					
0.25	21.911	19.501	17.569	15.985	14.663
0.5	14.378	13.055	11.955	11.026	10.230
1.0	21.911	19.501	17.569	15.985	14.663
1.5	37.411	32.300	28.417	25.368	22.910
2.0	58.280	48.837	42.028	36.885	32.864
2.5	83.632	68.173	57.537	49.772	43.854
3.0	112.86	89.709	74.441	63.613	55.536
3.5	145.47	113.02	92.405	78.150	67.706
4.0	181.08	137.78	111.20	93.210	80.232
4.5	219.34	163.75	130.65	108.67	93.029
5.0	259.96	190.74	150.63	124.46	106.04
(m=2, n=2)					
0.25	48.838	36.885	29.633	24.764	21.269
0.5	19.501	15.985	13.543	11.748	10.374
1.0	13.055	11.026	9.5421	8.4106	7.5189
1.5	15.090	12.613	10.834	9.4954	8.4510
2.0	19.501	15.985	13.543	11.748	10.374
2.5	25.348	20.336	16.978	14.573	12.764
3.0	32.300	25.368	20.886	17.750	15.432
3.5	40.172	30.920	25.131	21.169	18.285
4.0	48.837	36.885	29.633	24.764	21.269
4.5	58.197	43.188	34.333	28.492	24.349
5.0	68.172	49.772	39.193	32.323	27.502
(m=3, n=2)					
0.25	74.442	49.279	36.830	29.402	24.468
0.5	28.418	20.886	16.510	13.650	11.635
1.0	13.741	10.834	8.9428	7.6136	6.6284
1.5	11.955	9.5421	7.9398	6.798	5.9437
2.0	12.836	10.183	8.4381	7.2039	6.2847
2.5	14.854	11.6303	9.5563	8.1100	7.0440
3.0	17.569	13.543	11.018	9.2869	8.0258
3.5	20.795	15.772	12.7034	10.634	9.1450
4.0	24.433	18.236	14.547	12.099	10.357
4.5	28.417	20.886	16.510	13.650	11.635
5.0	32.701	23.684	18.565	15.266	12.962

Table 1b. CRITICAL BUCKLING LOAD INCLUDING SHEAR

$\hat{N}$   
 (N.B.S.A.)CR.

$\frac{a}{b} \cdot \frac{h^2}{ab}$	0.01	0.02	0.03	0.04	0.05
(m=1, n=3)					
0.25	33.210	28.882	25.552	22.911	20.765
0.5	35.705	30.916	27.260	24.377	22.046
1.0	78.004	63.939	54.172	46.993	41.495
1.5	143.40	111.56	91.288	77.251	66.955
2.0	224.91	167.49	133.43	110.88	94.847
2.5	318.64	228.85	178.53	146.36	124.01
3.0	421.91	293.95	225.55	182.97	153.92
3.5	532.77	361.78	273.87	220.34	184.31
4.0	649.80	431.62	323.13	258.22	215.03
4.5	771.89	503.02	373.07	296.48	245.98
5.0	898.18	575.63	423.53	335.01	277.09
(m=2, n=3)					
0.25	54.974	41.032	32.732	27.224	23.303
0.5	28.882	22.911	18.986	16.209	14.141
1.0	30.92	24.377	20.121	17.131	14.914
1.5	44.880	34.178	27.597	23.141	19.924
2.0	63.939	46.993	37.148	30.713	26.179
2.5	86.450	61.547	47.783	39.049	33.015
3.0	111.56	77.250	59.081	47.831	40.180
3.5	138.71	93.765	70.819	56.896	47.548
4.0	167.49	110.88	82.866	66.154	55.052
4.5	197.61	128.44	95.138	75.550	62.650
5.0	228.84	146.36	107.58	85.047	70.318
(m=3, n=3)					
0.25	78.207	51.489	38.378	30.589	25.428
0.5	34.415	24.791	19.374	15.899	13.482
1.0	23.879	17.865	14.270	11.880	10.176
1.5	27.260	20.121	15.946	13.205	11.268
2.0	34.414	24.791	19.374	15.899	13.481
2.5	43.600	30.619	23.594	19.191	16.173
3.0	54.171	37.148	28.265	22.811	19.121
3.5	65.781	44.149	33.223	26.632	22.224
4.0	78.206	51.488	38.377	30.588	25.428
4.5	91.287	59.081	43.673	34.639	28.702
5.0	104.91	66.867	49.072	38.758	32.027

Table 1c. CRITICAL BUCKLING LOAD INCLUDING SHEAR

m	n	$a/b$	$h^2/ab$	(N.B.A.S.) CR Minimum				
				0.01	0.02	0.03	0.04	0.05
1	1	1.0	1.0	3.786	3.595	3.421	3.264	3.120
2	1	2.0	2.0	3.595	3.264	2.989	2.756	2.558
3	1	3.0	3.0	3.421	2.989	2.653	2.386	2.167
1	2	0.5	0.5	14.378	13.055	11.955	11.026	10.230
2	2	1.0	1.0	13.055	11.026	9.542	8.4106	7.519
3	2	1.5	1.5	11.955	9.542	7.940	6.798	5.944
1	3	0.33	0.33	30.790	26.898	23.879	21.470	19.502
2	3	0.67	0.67	26.898	21.470	17.865	15.296	13.373
3	3	1.0	1.0	23.879	17.865	14.270	11.880	10.176

TABLE 1d. SUMMARY SHEET FOR CRITICAL BUCKLING-LOAD INCLUDING SHEAR.

( $\hat{N}$ B.A.) CR.

$\frac{b}{a} m$ (n=1)	1	2	3
0.25	18.063	66.015	146.007
0.5	6.250	18.063	38.028
1.0	4.000	6.250	11.111
1.5	4.694	4.340	6.250
2.0	6.250	4.000	4.695
2.5	8.410	4.203	4.135
3.0	11.111	4.694	4.000
3.5	14.331	5.389	4.096
4.0	18.062	6.250	4.340
4.5	22.299	7.260	4.694
5.0	27.039	8.410	5.138
(n=2)			
0.25	25.000	72.250	152.111
0.5	16.000	25.000	44.444
1.0	25.000	16.000	18.778
1.5	44.444	18.778	16.000
2.0	72.249	25.000	17.361
2.5	108.158	33.640	20.551
3.0	152.108	44.444	25.000
3.5	204.077	57.325	30.512
4.0	264.056	72.248	37.006
4.5	332.041	89.195	44.444
5.0	408.029	108.157	52.803
(n=3)			
0.25	39.063	83.265	162.563
0.5	42.250	39.063	56.250
1.0	100.000	42.250	36.000
1.5	200.692	65.340	42.250
2.0	342.244	99.999	56.249
2.5	524.399	145.200	75.689
3.0	747.095	200.690	99.998
3.5	1010.308	266.383	128.982
4.0	1314.032	342.242	162.559
4.5	1658.258	428.250	200.690
5.0	2042.989	525.397	243.354

TABLE 1e. CRITICAL BUCKLING LOAD EXCLUDING SHEAR.

$\frac{a}{b} \sqrt{h^2/ab}$	0.01	0.02	$\hat{N}$ 0.03	0.04	0.05
(m=1, n=1)					
0.25	0.8930	0.8067	0.7355	0.6760	0.6253
0.5	0.9341	0.8764	0.8254	0.7800	0.7394
1.0	0.9466	0.8986	0.8553	0.8159	0.7800
1.5	0.9424	0.8911	0.8451	0.8036	0.7660
2.0	0.9341	0.8764	0.8254	0.7800	0.7394
2.5	0.9244	0.8594	0.8030	0.7535	0.7098
3.0	0.9141	0.8418	0.7800	0.7268	0.6803
3.5	0.9035	0.8241	0.7574	0.7008	0.6520
4.0	0.8930	0.8067	0.7355	0.6760	0.6523
4.5	0.8825	0.7897	0.7146	0.6525	0.6003
5.0	0.8721	0.7732	0.6945	0.6303	0.5770
(m=2, n=1)					
0.25	0.6858	0.5218	0.4211	0.3530	0.3038
0.5	0.8067	0.6760	0.5817	0.5105	0.4549
1.0	0.8764	0.7800	0.7027	0.6394	0.5865
1.5	0.8949	0.8097	0.7394	0.6803	0.6299
2.0	0.8986	0.8159	0.7472	0.6891	0.6394
2.5	0.8964	0.8122	0.7425	0.6838	0.6337
3.0	0.8911	0.8036	0.7318	0.6717	0.6207
3.5	0.8842	0.7925	0.7180	0.6563	0.6044
4.0	0.8764	0.7800	0.7027	0.6394	0.5865
4.5	0.8681	0.7669	0.6869	0.6220	0.5682
5.0	0.8594	0.7535	0.6708	0.6045	0.5501
(m=3, n=1)					
0.25	0.4945	0.3285	0.2459	0.1965	0.1636
0.5	0.6572	0.4849	0.3899	0.3240	0.2771
1.0	0.7800	0.6394	0.5417	0.4699	0.4149
1.5	0.8254	0.7027	0.6118	0.5417	0.4860
2.0	0.8451	0.7317	0.6452	0.5770	0.5218
2.5	0.8532	0.7440	0.6596	0.5924	0.5376
3.0	0.8553	0.7472	0.6633	0.5964	0.5417
3.5	0.8538	0.7449	0.6607	0.5935	0.5388
4.0	0.8502	0.7394	0.6541	0.5865	0.5316
4.5	0.8451	0.7318	0.6452	0.5770	0.5218
5.0	0.8391	0.7228	0.6348	0.5659	0.5105

Table 2a. CRITICAL BUCKLING LOAD RATIO

$\frac{a}{b} \cdot \frac{h^2}{ab}$	$\hat{N}$				
	0.01	0.02	0.03	0.04	0.05
(m=1, n=2)					
0.25	0.8764	0.7800	0.7027	0.6394	0.5865
0.5	0.8986	0.8159	0.7472	0.6891	0.6394
1.0	0.8764	0.7800	0.7027	0.6394	0.5865
1.5	0.8418	0.7268	0.6394	0.5708	0.5155
2.0	0.8067	0.6760	0.5817	0.5105	0.4549
2.5	0.7732	0.6303	0.5320	0.4602	0.4055
3.0	0.7420	0.5898	0.4894	0.4182	0.3651
3.5	0.7128	0.5538	0.4528	0.3829	0.3318
4.0	0.6858	0.5218	0.4211	0.3530	0.3038
4.5	0.6606	0.4932	0.3935	0.3273	0.2802
5.0	0.6371	0.4675	0.3692	0.3050	0.2599
(m=2, n=2)					
0.25	0.6760	0.5105	0.4101	0.3428	0.2944
0.5	0.7800	0.6394	0.5417	0.4699	0.4149
1.0	0.8159	0.6891	0.5964	0.5257	0.4699
1.5	0.8036	0.6717	0.5770	0.5057	0.4501
2.0	0.7800	0.6394	0.5417	0.4699	0.4149
2.5	0.7535	0.6045	0.5047	0.4332	0.3794
3.0	0.7268	0.5708	0.4699	0.3994	0.3472
3.5	0.7008	0.5394	0.4384	0.3693	0.3190
4.0	0.6760	0.5105	0.4101	0.3428	0.2944
4.5	0.6525	0.4842	0.3849	0.3194	0.2730
5.0	0.6303	0.4602	0.3624	0.2989	0.2543
(m=3, n=2)					
0.25	0.4894	0.3240	0.2421	0.1933	0.1609
0.5	0.6394	0.4699	0.3715	0.3071	0.2618
1.0	0.7318	0.5770	0.4762	0.4055	0.3530
1.5	0.7472	0.5964	0.4962	0.4249	0.3715
2.0	0.7394	0.5865	0.4860	0.4149	0.3620
2.5	0.7228	0.5659	0.4650	0.3946	0.3428
3.0	0.7027	0.5417	0.4407	0.3715	0.3210
3.5	0.6815	0.5169	0.4163	0.3485	0.2997
4.0	0.6602	0.4928	0.3931	0.3270	0.2799
4.5	0.6394	0.4699	0.3715	0.3071	0.2618
5.0	0.6193	0.4485	0.3516	0.2891	0.2455

Table 2b. CRITICAL BUCKLING LOAD RATIO

$\frac{a}{b} \frac{h^2}{ab}$ (m=1, n=3)	$\hat{N}$				
	0.01	0.02	0.03	0.04	0.05
0.25	0.8502	0.7394	0.6541	0.5865	0.5316
0.5	0.8451	0.7318	0.6452	0.5770	0.5218
1.0	0.7800	0.6394	0.5417	0.4699	0.4149
1.5	0.7145	0.5559	0.4549	0.3849	0.3336
2.0	0.6572	0.4894	0.3899	0.3240	0.2771
2.5	0.6076	0.4364	0.3405	0.2791	0.2365
3.0	0.5647	0.3935	0.3019	0.2449	0.2060
3.5	0.5273	0.3581	0.2711	0.2181	0.1824
4.0	0.4945	0.3285	0.2459	0.1965	0.1636
4.5	0.4655	0.3033	0.2250	0.1788	0.1483
5.0	0.4396	0.2818	0.2073	0.1640	0.1356
(m=2, n=3)					
0.25	0.6602	0.4928	0.3931	0.3270	0.2799
0.5	0.7394	0.5865	0.4860	0.4149	0.3620
1.0	0.7318	0.5770	0.4762	0.4055	0.3530
1.5	0.6869	0.5231	0.4224	0.3542	0.3049
2.0	0.6394	0.4699	0.3715	0.3071	0.2618
2.5	0.5954	0.4239	0.3291	0.2689	0.2274
3.0	0.5559	0.3849	0.2944	0.2383	0.2002
3.5	0.5207	0.3520	0.2659	0.2136	0.1785
4.0	0.4894	0.3240	0.2421	0.1933	0.1609
4.5	0.4614	0.2999	0.2222	0.1764	0.1463
5.0	0.4364	0.2791	0.2051	0.1622	0.1341
(m=3, n=3)					
0.25	0.4811	0.3167	0.2361	0.1882	0.1564
0.5	0.6118	0.4407	0.3444	0.2827	0.2397
1.0	0.6633	0.4962	0.3964	0.3300	0.2827
1.5	0.6452	0.4762	0.3774	0.3125	0.2667
2.0	0.6118	0.4407	0.3444	0.2827	0.2397
2.5	0.5760	0.4045	0.3117	0.2536	0.2137
3.0	0.5417	0.3715	0.2827	0.2281	0.1912
3.5	0.5100	0.3423	0.2576	0.2065	0.1723
4.0	0.4811	0.3167	0.2361	0.1882	0.1564
4.5	0.4549	0.2944	0.2176	0.1726	0.1430
5.0	0.4311	0.2748	0.2016	0.1593	0.1316

Table 2c. CRITICAL BUCKLING LOAD RATIO

m	n	$a/b$	$\hat{N}$ Maximum				
			0.01	0.02	0.03	0.04	0.05
1	1	1.0	0.9466	0.8986	0.8553	0.8159	0.7800
2	1	2.0	0.8986	0.8159	0.7472	0.6891	0.6394
3	1	3.0	0.8538	0.7449	0.6607	0.5935	0.5388
1	2	0.5	0.8986	0.8159	0.7472	0.6891	0.6394
2	2	1.0	0.8159	0.6891	0.5964	0.5257	0.4699
3	2	1.5	0.7472	0.5964	0.4962	0.4249	0.3715
1	3	0.33	0.8553	0.7472	0.6633	0.5964	0.5417
2	3	0.67	0.7472	0.5964	0.4962	0.4249	0.3715
3	3	1.0	0.6633	0.4962	0.3964	0.3300	0.2827

TABLE 2d. SUMMARY SHEET FOR CRITICAL BUCKLING-LOAD RATIO.



$\frac{a/b}{\beta}$	0.01	0.25	$\hat{\Omega}^2$ (B.S.A. TI) 0.50	0.75	$(h^2/ab=0.01)$ 0.99
(m=1, n=1)					
0.25	15.968	12.097	8.065	4.0324	0.1613
0.5	5.7800	4.3788	2.919	1.460	0.0584
1.0	3.7486	2.8398	1.893	0.947	0.0379
1.5	4.3799	3.3181	2.212	1.106	0.0442
2.0	5.7800	4.3788	2.919	1.460	0.0584
2.5	7.6964	5.8306	3.887	1.944	0.0777
3.0	10.055	7.6172	5.078	2.539	0.1016
3.5	12.820	9.7118	6.475	3.237	0.1295
4.0	15.968	12.097	8.065	4.032	0.1613
4.5	19.482	14.759	9.839	4.920	0.1968
5.0	23.346	17.686	11.791	5.895	0.2358
(m=2, n=1)					
0.25	179.27	135.81	90.542	45.271	1.8108
0.5	57.698	43.711	29.140	14.570	0.5828
1.0	21.692	16.433	10.955	5.478	0.2191
1.5	15.380	11.652	7.768	3.884	0.1554
2.0	14.234	10.784	7.189	3.595	0.1438
2.5	14.917	11.301	7.534	3.767	0.1507
3.0	16.566	12.550	8.367	4.183	0.1673
3.5	18.870	14.295	9.530	4.765	0.1906
4.0	21.691	16.433	10.955	5.478	0.2191
4.5	24.957	18.907	12.605	6.302	0.2521
5.0	28.622	21.683	14.456	7.228	0.2891
(m=3, n=1)					
0.25	643.32	487.36	324.907	162.454	6.4982
0.5	222.66	168.69	112.458	56.229	2.2492
1.0	77.224	58.503	39.002	19.501	0.7800
1.5	45.966	34.823	23.215	11.608	0.4643
2.0	35.349	26.779	17.853	8.926	0.3571
2.5	31.431	23.812	15.874	7.937	0.3175
3.0	30.483	23.093	15.395	7.698	0.3079
3.5	31.159	23.605	15.737	7.868	0.3147
4.0	32.877	24.907	16.605	8.302	0.3321
4.5	35.348	26.779	17.853	8.926	0.3571
5.0	38.412	29.100	19.400	9.700	0.3880

TABLE 3a. NATURAL FREQUENCY INCLUDING SHEAR.

$\frac{a/b}{\beta}$	$\hat{\Omega}^2$ (B.S.A. II)				
	0.01	0.025	0.50	0.75	( $h^2/ab=0.01$ ) 0.99
(m=1, n=2)					
0.25	21.692	16.433	10.955	5.478	0.2191
0.5	14.234	10.784	7.189	3.595	0.1438
1.0	21.692	16.433	10.955	5.478	0.2191
1.5	37.037	28.058	18.706	9.353	0.3741
2.0	57.697	43.710	29.140	14.570	0.5828
2.5	82.795	62.724	41.816	20.908	0.8363
3.0	111.73	84.643	56.429	28.215	1.1286
3.5	144.02	109.11	72.737	36.369	1.4548
4.0	179.27	135.81	90.540	45.270	1.8108
4.5	217.14	164.50	109.67	54.834	2.1934
5.0	257.36	194.97	129.978	64.989	2.5996
(m=2, n=2)					
0.25	193.40	146.51	97.676	48.838	1.9535
0.5	77.224	58.503	39.002	19.501	0.7800
1.0	51.698	39.165	26.110	13.055	0.5222
1.5	59.756	45.270	30.180	15.090	0.6036
2.0	77.223	58.502	39.002	19.501	0.7800
2.5	100.38	76.044	50.696	25.348	1.0139
3.0	127.91	96.899	64.599	32.300	1.2920
3.5	159.08	120.52	80.344	40.172	1.4769
4.0	193.39	146.51	97.674	48.837	1.9535
4.5	230.46	174.59	116.394	58.197	2.3279
5.0	269.96	204.52	136.344	68.172	2.7269
(m=3, n=2)					
0.25	663.27	502.48	334.987	167.494	6.6998
0.5	253.20	191.82	127.879	63.939	2.5576
1.0	122.43	92.749	61.833	30.916	1.2367
1.5	106.52	80.694	53.796	26.898	1.0759
2.0	144.37	86.646	57.764	28.882	1.1553
2.5	132.35	100.27	66.844	33.422	1.3369
3.0	156.54	118.59	79.058	39.529	1.5812
3.5	185.28	140.36	93.576	46.788	1.8715
4.0	217.69	164.92	109.947	54.973	2.1989
4.5	253.20	191.81	127.877	63.938	2.5575
5.0	291.37	220.73	147.154	73.577	2.9431

TABLE 3b. NATURAL FREQUENCY INCLUDING SHEAR.

a/b \ β	$\Omega^2$ (B.S.A. II)				
	0.01	0.25	0.50	0.75	( $h^2/ab=0.01$ ) 0.99
(m=1, n=3)					
0.25	32.878	24.907	16.605	8.3024	0.3321
0.5	35.348	26.779	17.853	8.9264	0.3571
1.0	77.224	58.503	39.002	19.501	0.7800
1.5	141.97	107.55	71.702	35.851	1.4341
2.0	222.66	168.68	112.456	56.228	2.2491
2.5	315.45	238.98	159.319	79.660	3.1864
3.0	417.69	316.43	210.953	105.476	4.2191
3.5	527.45	399.58	266.387	133.194	5.3278
4.0	643.30	487.35	324.901	162.451	6.4981
4.5	764.17	578.92	385.944	192.972	7.7189
5.0	889.20	673.64	449.092	224.546	8.9819
(m=2, n=3)					
0.25	217.70	164.92	109.948	54.974	2.1990
0.5	114.37	86.646	57.764	28.882	1.1553
1.0	122.43	92.749	61.833	30.916	1.2367
1.5	177.72	134.639	89.760	44.880	1.7952
2.0	253.20	191.816	127.878	63.939	2.5576
2.5	342.34	259.348	172.899	86.450	3.4580
3.0	441.77	334.677	223.118	111.559	4.4624
3.5	549.28	416.118	277.412	138.706	5.5483
4.0	663.26	502.472	334.981	167.491	6.6997
4.5	782.55	592.839	395.226	197.613	7.9046
5.0	906.23	686.534	457.690	228.845	9.1538
(m=3, n=3)					
0.25	696.83	527.899	351.933	175.966	7.0387
0.5	306.63	232.899	154.866	77.433	3.0973
1.0	212.76	161.185	107.457	53.728	2.1491
1.5	242.89	184.005	122.670	61.335	2.4534
2.0	306.63	232.297	154.865	77.432	3.0973
2.5	388.47	294.299	196.199	98.100	3.9240
3.0	482.67	365.657	243.771	121.886	4.8754
3.5	586.11	444.024	296.016	148.008	5.9204
4.0	696.82	527.891	351.927	175.964	7.0386
4.5	813.37	616.189	410.793	205.396	8.2159
5.0	934.73	708.128	472.085	236.043	9.4417

TABLE 3c. NATURAL FREQUENCY INCLUDING SHEAR.

		$\hat{\Omega}^2$ (B.S.A. TI) ( $h^2/ab=0.01$ ), ( $\beta=0$ )		
$\frac{b/a}{n}$	$n$	1	2	3
		(m=1)		
0.25		16.130	21.911	33.210
0.5		5.838	14.378	35.705
1.0		3.786	21.911	78.004
1.5		4.424	37.411	143.405
2.0		5.838	58.280	224.913
2.5		7.774	83.632	318.638
3.0		10.156	112.858	421.906
3.5		12.949	145.475	532.774
4.0		16.129	181.080	649.803
4.5		19.679	219.337	771.889
5.0		23.582	259.956	898.183
	(m=2)			
0.25		181.083	195.351	219.896
0.5		58.281	78.004	115.528
1.0		21.911	52.220	123.666
1.5		15.536	60.360	179.519
2.0		14.378	78.003	255.755
2.5		15.068	101.392	345.798
3.0		16.733	129.198	446.236
3.5		19.060	160.687	554.825
4.0		21.910	195.348	669.962
4.5		25.209	232.789	790.452
5.0		28.911	272.689	915.379
	(m=3)			
0.25		649.814	669.974	703.866
0.5		224.915	255.757	309.732
1.0		78.004	123.666	214.913
1.5		46.431	107.592	245.341
2.0		35.706	115.528	309.729
2.5		31.749	133.688	392.398
3.0		30.791	158.117	487.542
3.5		31.474	187.152	592.032
4.0		33.209	219.893	703.854
4.5		35.705	255.753	821.585
5.0		38.800	294.309	944.170

TABLE 3d. NATURAL FREQUENCY INCLUDING SHEAR.

$\hat{\Omega}^2$  (B.S.A. TI. RI.) MINIMUM

m	n	$\frac{a}{b} \hat{\beta}$	$(h^2/ab=0.01)$					
			0.0	0.01	0.25	0.50	0.75	0.99
1	1	1.0	3.786	3.749	2.840	1.893	0.947	0.038
2	1	2.0	14.378	14.234	10.784	7.189	3.595	0.144
3	1	3.0	30.791	30.483	23.093	15.395	7.698	0.308
1	2	0.5	14.378	14.234	10.784	7.189	3.595	0.144
2	2	1.0	52.220	51.698	39.165	26.110	13.055	0.522
3	2	1.5	107.592	106.516	80.694	53.796	26.898	1.076
1	3	0.33	30.791	30.483	23.093	15.395	7.698	0.308
2	3	0.67	107.592	106.516	80.694	53.796	26.898	1.076
3	3	1.0	214.913	212.764	161.185	107.457	53.728	2.149

TABLE 3e. SUMMARY SHEET FOR NATURAL FREQUENCY INCLUDING SHEAR.

$a/b$	$\Omega^2$ (B.S.A. TT)				
	0.01	0.25	0.50	0.75	0.99
(m=1, n=1)					
0.25	14.425	10.928	7.285	3.643	0.146
0.5	5.423	4.108	2.739	1.369	0.055
1.0	3.559	2.696	1.797	0.899	0.036
1.5	4.141	3.137	2.092	1.046	0.042
2.0	5.423	4.108	2.739	1.369	0.055
2.5	7.156	5.421	3.614	1.807	0.072
3.0	9.259	7.015	4.676	2.338	0.094
3.5	11.692	8.857	5.905	2.953	0.118
4.0	14.424	10.927	7.285	3.643	0.146
4.5	17.433	14.174	8.805	4.402	0.176
5.0	20.699	15.681	10.454	5.227	0.209
(m=2, n=1)					
0.25	136.408	103.339	68.893	34.446	1.378
0.5	48.350	36.629	24.419	12.210	0.488
1.0	19.306	14.626	9.751	4.875	0.195
1.5	13.917	10.543	7.029	3.514	0.141
2.0	12.924	9.791	6.528	3.264	0.131
2.5	13.516	10.240	6.827	3.413	0.137
3.0	14.939	11.317	7.545	3.773	0.151
3.5	16.912	12.812	8.541	4.271	0.171
4.0	19.306	14.626	9.750	4.875	0.195
4.5	22.048	16.703	11.136	5.568	0.223
5.0	25.095	19.011	12.674	6.337	0.254
(m=3, n=1)					
0.25	427.313	323.722	215.815	107.908	4.316
0.5	165.818	125.620	83.747	41.873	1.675
1.0	63.300	47.955	31.970	15.985	0.639
1.5	39.135	29.647	19.765	9.883	0.395
2.0	30.607	23.187	15.458	7.729	0.309
2.5	27.409	20.764	13.843	6.921	0.277
3.0	26.629	20.174	13.449	6.725	0.269
3.5	27.185	20.595	13.730	6.865	0.275
4.0	28.593	21.661	14.441	7.221	0.289
4.5	30.607	23.187	15.459	7.729	0.309
5.0	33.088	25.067	16.711	8.356	0.334

TABLE 4a. NATURAL FREQUENCY INCLUDING SHEAR.

$\beta$ a/b	$\Omega^2$ (B.S.A. TI)					$(h^2/ab=0.02)$ 0.99
	0.01	0.25	0.50	0.75		
(m=1, n=2)						
0.25	19.306	14.626	9.751	4.875	0.195	
0.5	12.924	9.791	6.528	3.264	0.131	
1.0	19.306	14.626	9.751	4.875	0.195	
1.5	31.977	24.225	16.150	8.075	0.323	
2.0	48.349	36.628	24.419	12.209	0.488	
2.5	67.491	51.129	34.086	17.043	0.682	
3.0	88.812	67.282	44.855	22.427	0.897	
3.5	111.890	84.765	56.510	28.255	1.130	
4.0	136.406	103.338	68.892	34.446	1.378	
4.5	162.116	122.816	81.877	40.938	1.638	
5.0	188.830	143.053	95.369	47.684	1.907	
(m=2, n=2)						
0.25	146.066	110.656	73.771	36.885	1.475	
0.5	63.300	47.955	31.970	15.985	0.639	
1.0	43.661	33.076	22.051	11.026	0.441	
1.5	49.947	37.838	25.226	12.613	0.505	
2.0	63.300	47.954	31.970	15.985	0.639	
2.5	80.529	61.007	40.671	20.336	0.813	
3.0	100.457	76.104	50.736	25.368	1.015	
3.5	122.442	92.759	61.839	30.920	1.237	
4.0	146.064	110.655	73.770	36.885	1.475	
4.5	171.024	129.564	86.376	43.188	1.728	
5.0	197.096	149.315	99.544	49.772	1.991	
(m=3, n=2)						
0.25	439.077	332.634	221.756	110.878	4.435	
0.5	186.094	140.980	93.987	46.993	1.880	
1.0	96.534	73.132	48.755	24.377	0.975	
1.5	85.020	64.409	42.940	21.470	0.859	
2.0	90.727	68.733	45.822	22.911	0.916	
2.5	103.626	78.505	52.337	26.168	1.047	
3.0	120.667	91.415	60.943	30.472	1.219	
3.5	140.527	106.460	70.973	35.587	1.420	
4.0	162.486	123.096	82.064	41.032	1.641	
4.5	186.091	140.978	93.985	46.993	1.880	
5.0	211.027	159.869	106.579	53.290	2.132	

TABLE 4b. NATURAL FREQUENCY INCLUDING SHEAR.

$\frac{a}{b}$	$\Omega^2$ (B.S.A. II)					$(h^2/ab=0.02)$
	0.01	0.25	0.50	0.75	0.99	
(m=1, n=3)						
0.25	28.593	21.662	14.441	7.221	0.289	
0.5	30.607	23.187	15.458	7.729	0.309	
1.0	63.300	47.955	31.970	15.985	0.639	
1.5	110.444	83.670	55.780	27.890	1.116	
2.0	165.816	125.619	83.746	41.873	1.675	
2.5	226.557	171.634	114.423	57.211	2.289	
3.0	291.015	220.466	146.978	73.489	2.940	
3.5	358.159	271.332	180.888	90.444	3.618	
4.0	427.306	323.717	215.811	107.906	4.316	
4.5	497.987	377.263	251.509	125.754	5.030	
5.0	569.870	431.720	287.813	143.907	5.756	
(m=2, n=3)						
0.25	162.488	123.097	82.065	41.0324	1.641	
0.5	90.727	68.733	45.822	22.9109	0.916	
1.0	96.534	73.132	48.754	24.377	0.975	
1.5	135.344	102.533	68.355	34.178	1.367	
2.0	186.092	140.979	93.986	46.993	1.880	
2.5	243.725	184.640	123.093	61.547	2.462	
3.0	305.911	231.751	154.501	77.250	3.090	
3.5	371.309	281.294	187.530	93.765	3.751	
4.0	439.070	332.629	221.753	110.876	4.435	
4.5	508.625	385.322	256.881	128.441	5.138	
5.0	579.574	439.071	292.714	146.357	5.854	
(m=3, n=3)						
0.25	458.767	347.551	231.701	115.851	4.634	
0.5	220.889	167.340	111.560	55.780	2.231	
1.0	159.173	120.586	80.390	40.195	1.608	
1.5	179.281	135.819	90.546	45.273	1.811	
2.0	220.887	167.339	111.559	55.780	2.231	
2.5	272.813	206.677	137.784	68.892	2.756	
3.0	330.984	250.746	167.164	83.582	3.343	
3.5	393.365	298.003	198.669	99.335	3.973	
4.0	458.761	347.546	231.698	115.849	4.634	
4.5	526.408	398.794	265.862	132.921	5.317	
5.0	595.782	451.350	300.900	150.450	6.018	

TABLE 4c. NATURAL FREQUENCY INCLUDING SHEAR.

TABLE 4d. NATURAL FREQUENCY INCLUDING SHEAR.



$\frac{a}{b} \backslash n$	$\hat{\Omega}^2$ (B.S.A. TI. RI) $(h^2/ab=0.02), (\hat{\beta}=0)$		
	1	2	3
	(m=1)		
0.25	14.570	19.501	28.882
0.5	5.478	13.055	30.916
1.0	3.595	19.501	63.939
1.5	4.183	32.299	111.560
2.0	5.478	48.837	167.491
2.5	7.228	68.173	228.845
3.0	9.353	89.709	293.955
3.5	11.810	113.020	361.777
4.0	14.570	137.784	431.622
4.5	17.609	163.754	503.017
5.0	20.908	190.737	575.627
	(m=2)		
0.25	137.786	147.542	164.130
0.5	48.838	63.940	91.644
1.0	19.501	44.102	97.509
1.5	14.058	50.451	136.711
2.0	13.055	63.939	187.972
2.5	13.653	81.343	246.187
3.0	15.090	101.472	309.001
3.5	17.083	123.678	375.059
4.0	19.501	147.540	443.505
4.5	22.271	172.752	513.763
5.0	25.348	199.087	585.429
	(m=3)		
0.25	431.629	443.512	463.402
0.5	167.493	187.973	223.120
1.0	63.940	97.509	160.781
1.5	39.530	85.879	181.091
2.0	30.917	91.643	223.119
2.5	27.686	104.673	275.569
3.0	26.898	121.886	334.327
3.5	27.450	141.946	397.338
4.0	28.882	164.128	463.395
4.5	30.916	187.971	531.725
5.0	33.422	213.159	601.800

TABLE 4d. NATURAL FREQUENCY INCLUDING SHEAR.

$\hat{\Omega}^2$  (B.S.A. TI) MINIMUM

m	n	$\frac{a}{b} / \hat{\beta}$	$(h^2/ab=0.02)$					
			0.0	0.01	0.25	0.50	0.75	0.99
1	1	1.0	3.595	3.559	2.696	1.797	0.899	0.036
2	1	2.0	13.055	12.924	9.791	6.528	3.264	0.131
3	1	3.0	26.898	26.629	20.174	13.449	6.725	0.269
1	2	0.5	13.055	12.924	9.791	6.528	3.264	0.131
2	2	1.0	44.102	43.661	33.076	22.051	11.026	0.441
3	2	1.5	85.879	85.020	64.409	42.940	21.470	0.859
1	3	0.33	26.898	26.629	20.174	13.449	6.725	0.269
2	3	0.67	85.879	85.020	64.409	42.940	21.470	0.859
3	3	1.0	160.781	159.173	120.586	80.390	40.195	1.811

TABLE 4e. SUMMARY SHEET FOR NATURAL FREQUENCY INCLUDING SHEAR.

$\frac{a/b}{\beta}$	$\hat{\Omega}^2$ (B.S.A. TI)					$(h^2/ab=0.03)$
	0.01	0.25	0.50	0.75	0.99	
(m=1, n=1)						
0.25	13.153	9.964	6.643	3.321	0.133	
0.5	5.107	3.869	2.580	1.290	0.052	
1.0	3.387	2.566	1.711	0.855	0.034	
1.5	3.928	2.975	1.984	0.992	0.040	
2.0	5.107	3.869	2.579	1.290	0.052	
2.5	6.686	5.065	3.377	1.688	0.070	
3.0	8.580	6.500	4.334	2.167	0.087	
3.5	10.746	8.141	5.428	2.714	0.109	
4.0	13.153	9.964	6.643	3.321	0.133	
4.5	15.774	11.950	7.967	3.983	0.159	
5.0	18.591	14.084	9.389	4.695	0.188	
(m=2, n=1)						
0.25	110.086	82.399	55.599	27.800	1.112	
0.5	41.608	31.521	21.014	10.507	0.420	
1.0	17.393	13.177	8.784	4.392	0.176	
1.5	12.708	9.627	6.418	3.209	0.128	
2.0	11.845	8.966	5.977	2.989	0.120	
2.5	12.356	9.361	6.241	3.120	0.125	
3.0	13.603	10.305	6.870	3.435	0.137	
3.5	15.322	11.608	7.739	3.869	0.155	
4.0	17.393	13.176	8.784	4.392	0.176	
4.5	19.747	14.960	9.973	4.987	0.200	
5.0	22.341	16.925	11.283	5.642	0.226	
(m=3, n=1)						
0.25	319.901	242.349	161.566	80.783	3.231	
0.5	132.094	100.071	66.714	33.357	1.334	
1.0	53.630	40.629	27.086	13.543	0.542	
1.5	34.071	25.811	17.207	8.604	0.344	
2.0	26.988	20.445	13.630	6.815	0.273	
2.5	24.299	18.408	12.272	6.136	0.245	
3.0	23.641	17.910	11.940	5.970	0.239	
3.5	24.110	18.265	12.177	6.088	0.244	
4.0	25.297	19.164	12.776	6.388	0.256	
4.5	26.987	20.445	13.630	6.815	0.277	
5.0	29.060	22.015	14.677	7.338	0.294	

TABLE 5a. NATURAL FREQUENCY INCLUDING SHEAR.

$\frac{a}{b}$	$\beta$		$\Omega^2$ (B.S.A. II)	$(h^2/ab=0.03)$	
	0.25	0.01		0.50	0.75
(m=1, n=2)					
0.25	13.177	17.393	8.784	4.392	0.176
0.5	8.966	11.835	5.977	2.989	0.120
1.0	13.177	17.393	8.784	4.392	0.176
1.5	21.313	28.133	14.209	7.104	0.284
2.0	31.521	41.608	21.014	10.507	0.420
2.5	43.153	56.962	28.769	14.384	0.575
3.0	55.830	73.696	37.220	18.610	0.744
3.5	69.304	91.481	46.202	23.101	0.924
4.0	83.397	110.084	55.598	27.799	1.112
4.5	97.985	129.340	65.323	32.662	1.307
5.0	112.97	149.123	75.315	37.657	1.506
(m=2, n=2)					
0.25	88.899	117.347	59.266	29.633	1.185
0.5	40.629	53.630	27.086	13.543	0.542
1.0	28.626	37.787	19.594	9.542	0.382
1.5	32.503	42.904	21.669	10.834	0.433
2.0	40.629	53.630	27.086	13.543	0.542
2.5	50.935	67.234	33.957	16.978	0.679
3.0	62.657	82.707	41.771	20.886	0.835
3.5	75.394	99.520	50.263	25.131	1.005
4.0	88.898	117.345	59.265	29.633	1.185
4.5	103.000	135.960	68.667	34.333	1.373
5.0	117.580	155.205	78.386	39.193	1.568
(m=3, n=2)					
0.25	248.603	328.155	165.736	82.868	3.315
0.5	111.444	147.106	74.296	37.148	1.486
1.0	60.364	79.861	40.243	20.121	0.805
1.5	53.594	70.744	35.729	17.865	0.715
2.0	56.957	75.183	37.971	18.986	0.759
2.5	64.505	85.147	43.003	21.502	0.860
3.0	74.373	98.172	49.582	24.791	0.992
3.5	85.748	113.187	57.165	28.583	1.143
4.0	98.193	129.615	65.462	32.731	1.309
4.5	111.442	147.104	74.295	37.147	1.486
5.0	125.316	165.417	83.544	41.772	1.671

TABLE 5b. NATURAL FREQUENCY INCLUDING SHEAR.

$\frac{a}{b} \backslash \hat{\beta}$	$\hat{\Omega}^2$ (B.S.A. TI)					$(h^2/ab=0.03)$ 0.99
	0.01	0.25	0.50	0.75		
(m=1, n=3)						
0.25	25.297	19.164	12.776	6.388	0.256	
0.5	26.988	20.445	13.630	6.815	0.273	
1.0	53.630	40.629	27.086	13.543	0.542	
1.5	90.375	68.466	45.644	22.822	0.913	
2.0	132.093	100.070	66.714	33.357	1.334	
2.5	176.749	133.901	89.267	44.634	1.785	
3.0	223.297	169.164	112.776	56.388	2.256	
3.5	271.136	205.406	136.937	68.469	2.739	
4.0	319.896	242.346	161.564	80.782	3.231	
4.5	369.336	279.800	186.534	93.267	3.731	
5.0	419.293	317.646	211.764	105.882	4.235	
(m=2, n=3)						
0.25	129.617	98.195	65.463	32.732	1.309	
0.5	75.184	56.957	37.972	18.986	0.759	
1.0	79.681	60.364	40.243	20.121	0.805	
1.5	109.284	82.791	55.194	27.597	1.104	
2.0	147.104	111.443	74.295	37.148	1.486	
2.5	189.219	143.347	95.565	47.783	1.911	
3.0	233.959	177.242	118.161	59.081	2.363	
3.5	280.444	212.452	141.638	70.819	2.833	
4.0	328.151	248.599	165.733	82.866	3.315	
4.5	376.748	285.415	190.277	95.138	3.806	
5.0	426.016	322.740	215.160	107.580	4.303	
(m=3, n=3)						
0.25	341.947	259.051	172.701	86.350	3.454	
0.5	172.619	130.772	87.181	43.591	1.744	
1.0	127.147	96.324	64.216	32.108	1.284	
1.5	142.074	107.632	71.755	35.877	1.435	
2.0	172.618	130.771	87.181	43.590	1.745	
2.5	210.223	159.260	106.173	53.087	2.124	
3.0	251.841	190.789	127.192	63.596	2.544	
3.5	296.017	224.256	149.504	74.752	2.990	
4.0	341.943	259.047	172.698	86.349	3.454	
4.5	389.123	294.790	196.527	98.263	3.931	
5.0	437.235	331.238	220.826	110.413	4.417	

TABLE 5c. NATURAL FREQUENCY INCLUDING SHEAR.

$\frac{a}{b} \backslash n$	$\hat{\Omega}^2$ (B.S.A. TI. RI) $(h^2/ab=0.03), (\beta=0)$		
	1	2	3
(m=1)			
0.25	13.286	17.569	25.552
0.5	5.159	11.955	27.260
1.0	3.421	17.569	54.172
1.5	3.967	28.417	91.288
2.0	5.159	42.028	133.427
2.5	6.753	57.537	178.534
3.0	8.667	74.441	225.552
3.5	10.855	92.405	273.875
4.0	13.286	111.196	323.127
4.5	15.934	130.646	373.067
5.0	18.779	150.629	423.529
(m=2)			
0.25	111.198	118.532	130.926
0.5	42.029	54.172	75.843
1.0	17.569	38.169	80.485
1.5	12.837	43.337	110.388
2.0	11.955	54.172	148.590
2.5	12.481	67.914	191.130
3.0	13.741	83.543	236.323
3.5	15.477	100.526	283.277
4.0	17.569	118.531	331.465
4.5	19.947	137.333	380.554
5.0	22.567	156.773	430.319
(m=3)			
0.25	323.132	331.470	345.401
0.5	133.428	148.592	174.363
1.0	54.172	80.485	128.431
1.5	34.415	71.458	143.510
2.0	27.260	75.943	174.361
2.5	24.544	86.007	212.346
3.0	23.879	99.164	254.385
3.5	24.354	114.330	299.007
4.0	25.552	130.925	345.397
4.5	27.260	148.590	393.053
5.0	29.354	167.088	441.651

TABLE 5d. NATURAL FREQUENCY INCLUDING SHEAR.

$\hat{\Omega}^2$  (B.S.A. TI) MINIMUM

m	n	$\frac{h^2}{ab}$	$(h^2/ab=0.03)$					
			0.0	0.01	0.25	0.50	0.75	0.99
1	1	1.0	3.421	3.387	2.566	1.711	0.855	0.034
2	1	2.0	11.955	11.835	8.966	5.977	2.989	0.120
3	1	3.0	23.879	23.641	17.910	11.940	5.970	0.239
1	2	0.5	11.955	11.835	8.966	5.977	2.989	0.120
2	2	1.0	38.169	37.787	28.626	19.084	9.542	0.382
3	2	1.5	71.458	70.744	53.594	35.729	17.865	0.715
1	3	0.33	23.879	23.641	17.910	11.940	5.970	0.239
2	3	0.67	71.458	70.744	53.594	35.729	17.865	0.715
3	3	1.0	128.431	127.147	96.324	64.216	32.108	1.284

TABLE 5e. SUMMARY SHEET FOR NATURAL FREQUENCY INCLUDING SHEAR.

$\frac{a}{b} \backslash \beta$	$\Omega^2$ (B.S.A. II)					$(h^2/ab=0.04)$
	0.01	0.25	0.50	0.75	0.99	
(m=1, n=1)						
0.25	12.087	9.157	6.105	3.052	0.122	
0.5	4.827	3.656	2.438	1.219	0.049	
1.0	3.231	2.448	1.632	0.816	0.033	
1.5	3.735	2.829	1.886	0.943	0.038	
2.0	4.826	3.656	2.438	1.219	0.049	
2.5	6.274	4.753	3.169	1.584	0.063	
3.0	7.994	6.056	4.037	2.019	0.081	
3.5	9.943	7.532	5.022	2.511	0.100	
4.0	12.087	9.157	6.105	3.052	0.122	
4.5	14.404	10.912	7.275	3.637	0.146	
5.0	16.873	12.782	8.522	4.261	0.170	
(m=2, n=1)						
0.25	92.279	69.909	46.606	23.303	0.932	
0.5	36.517	27.664	18.443	9.221	0.369	
1.0	15.825	11.989	7.992	3.996	0.160	
1.5	11.692	8.858	5.905	2.953	0.118	
2.0	10.915	8.269	5.513	2.756	0.110	
2.5	11.379	8.621	5.747	2.874	0.115	
3.0	12.487	9.460	6.306	3.153	0.126	
3.5	14.006	10.610	7.074	3.537	0.142	
4.0	15.825	11.989	7.992	3.996	0.160	
4.5	17.881	13.546	9.031	4.515	0.181	
5.0	20.132	15.252	10.168	5.084	0.203	
(m=3, n=1)						
0.25	255.642	193.668	129.112	64.556	2.582	
0.5	109.769	83.158	55.439	27.720	1.109	
1.0	46.524	35.245	23.497	11.748	0.470	
1.5	30.167	22.854	15.236	7.618	0.305	
2.0	24.134	18.283	12.189	6.094	0.244	
2.5	21.823	16.532	11.022	5.511	0.220	
3.0	21.255	16.102	10.735	5.367	0.215	
3.5	21.660	16.409	10.939	5.470	0.219	
4.0	22.682	17.183	11.455	5.728	0.229	
4.5	24.133	18.283	12.189	6.094	0.244	
5.0	25.907	19.626	13.084	6.542	0.262	

TABLE 6a. NATURAL FREQUENCY INCLUDING SHEAR.



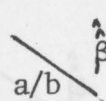
$\frac{a}{b}$ 	$\hat{\Omega}^2$ (B.S.A. TI)					$(h^2/ab=0.04)$ 0.99
	0.01	0.25	0.50	0.75		
(m=1, n=2)						
0.25	15.825	11.989	7.992	3.996	0.160	
0.5	10.915	8.269	5.513	2.756	0.110	
1.0	15.825	11.989	7.992	3.996	0.160	
1.5	25.114	19.026	12.684	6.342	0.254	
2.0	36.516	27.664	18.443	9.221	0.369	
2.5	49.274	37.329	24.886	12.443	0.498	
3.0	62.977	47.710	31.807	15.903	0.636	
3.5	77.369	58.613	39.075	19.538	0.782	
4.0	92.278	69.908	46.605	23.303	0.932	
4.5	107.588	81.506	54.337	27.169	1.087	
5.0	123.214	93.344	62.229	31.115	1.245	
(m=2, n=2)						
0.25	98.066	74.292	49.528	24.764	0.991	
0.5	46.524	35.245	23.497	11.748	0.470	
1.0	33.306	25.232	16.821	8.411	0.336	
1.5	37.602	28.486	18.991	9.495	0.380	
2.0	46.523	35.245	23.497	11.748	0.470	
2.5	57.707	43.718	29.145	14.573	0.583	
3.0	70.288	53.249	35.499	17.750	0.710	
3.5	83.828	63.506	42.337	21.169	0.847	
4.0	98.064	74.291	49.527	24.764	0.991	
4.5	112.827	85.475	56.984	28.492	1.140	
5.0	128.000	96.969	64.646	32.323	1.293	
(m=3, n=2)						
0.25	261.974	198.465	132.310	66.155	2.646	
0.5	121.625	92.140	61.427	30.713	1.229	
1.0	67.837	51.392	34.261	17.131	0.685	
1.5	60.572	45.888	30.592	15.296	0.612	
2.0	64.187	48.626	32.418	16.209	0.648	
2.5	72.260	54.743	36.495	18.248	0.730	
3.0	82.746	62.687	41.791	20.896	0.836	
3.5	94.753	71.782	47.855	23.927	0.957	
4.0	107.806	81.671	54.448	27.224	1.089	
4.5	121.623	92.139	61.426	30.713	1.229	
5.0	136.018	103.044	68.696	34.348	1.374	

TABLE 6b. NATURAL FREQUENCY INCLUDING SHEAR.

$\frac{a}{b} \backslash \hat{\beta}$	$\hat{\Omega}^2$ (B.S.A. TI)					$(h^2/ab=0.04)$
	0.01 (m=1, n=3)	0.25	0.50	0.75	0.99	
0.25	22.682	17.183	11.456	5.728	0.229	
0.5	24.134	18.283	12.189	6.094	0.244	
1.0	46.523	35.245	23.497	11.748	0.470	
1.5	76.478	57.938	38.625	19.313	0.773	
2.0	109.768	83.158	55.438	27.719	1.109	
2.5	144.894	109.768	73.179	36.589	1.464	
3.0	181.145	137.231	91.487	45.744	1.830	
3.5	218.135	165.254	110.169	55.085	2.203	
4.0	255.638	193.665	129.110	64.555	2.582	
4.5	293.510	222.356	148.238	74.119	2.965	
5.0	331.659	251.257	167.504	83.752	3.350	
	(m=2, n=3)					
0.25	107.807	81.672	54.448	27.224	1.089	
0.5	64.187	48.627	32.418	16.209	0.648	
1.0	67.837	51.392	34.261	17.131	0.685	
1.5	91.639	69.424	46.282	23.141	0.926	
2.0	121.624	92.139	61.426	30.713	1.229	
2.5	154.636	117.148	78.099	39.049	1.562	
3.0	189.409	143.492	95.661	47.831	1.913	
3.5	225.308	170.688	113.792	56.896	2.276	
4.0	261.971	198.463	132.308	66.154	2.646	
4.5	299.177	226.649	151.100	75.550	3.022	
5.0	336.785	255.140	170.093	85.047	3.402	
	(m=3, n=3)					
0.25	272.546	206.474	137.650	68.825	2.753	
0.5	141.662	107.320	71.546	35.773	1.431	
1.0	105.850	80.189	53.460	26.730	1.069	
1.5	117.657	89.134	59.423	29.711	1.189	
2.0	141.661	107.319	71.546	35.773	1.431	
2.5	170.993	129.540	86.360	43.180	1.727	
3.0	203.243	153.972	102.648	51.324	2.053	
3.5	237.293	179.768	119.845	59.923	2.397	
4.0	272.543	206.472	137.648	68.824	2.753	
4.5	308.632	233.813	155.875	77.938	3.118	
5.0	345.335	261.617	174.412	87.206	3.488	

TABLE 6c. NATURAL FREQUENCY INCLUDING SHEAR.

TABLE 6c. NATURAL FREQUENCY INCLUDING SHEAR.

$\frac{b/a}{n}$	$\hat{\Omega}^2$ (B.S.A. TI. RI)		
	$(h^2/ab=0.04), (\hat{\beta}=0)$		
$(m=1)$	1	2	3
0.25	12.210	15.985	22.911
0.5	4.875	11.026	24.377
1.0	3.264	15.985	46.993
1.5	3.773	25.368	77.251
2.0	4.875	36.885	110.877
2.5	6.337	49.772	146.358
3.0	8.075	63.613	182.975
3.5	10.043	78.150	220.338
4.0	12.209	93.210	258.220
4.5	14.459	108.675	296.475
5.0	17.043	124.458	335.009
$(m=2)$			
0.25	93.212	99.056	108.896
0.5	36.886	46.993	64.835
1.0	15.985	33.642	68.522
1.5	11.810	37.982	92.565
2.0	11.026	46.993	122.852
2.5	11.494	58.290	156.198
3.0	12.613	70.998	191.323
3.5	14.147	84.675	227.584
4.0	15.985	99.055	264.646
4.5	18.061	113.967	302.199
5.0	20.336	129.293	340.187
$(m=3)$			
0.25	258.224	264.620	275.299
0.5	110.878	122.853	143.093
1.0	46.993	68.522	106.919
1.5	30.472	61.869	118.846
2.0	24.377	64.835	143.092
2.5	22.043	72.990	172.720
3.0	21.470	83.582	205.296
3.5	21.879	95.711	239.690
4.0	22.911	108.895	275.296
4.5	24.377	122.852	311.750
5.0	26.168	137.392	348.823

TABLE 6d. NATURAL FREQUENCY INCLUDING SHEAR.

$\hat{\Omega}^2$  (B.S.A. TI) MINIMUM

m	n	$\frac{h^2}{ab}$	$(h^2/ab=0.04)$					
			0.0	0.01	0.25	0.50	0.75	0.99
1	1	1.0	3.264	3.231	2.448	1.632	0.816	0.033
2	1	2.0	11.026	10.915	8.269	5.513	2.756	0.110
3	1	3.0	21.470	21.255	16.102	10.735	5.367	0.215
1	2	0.5	11.026	10.915	8.269	5.513	2.756	0.110
2	2	1.0	33.462	33.306	25.232	16.821	8.411	0.336
3	2	1.5	61.184	60.572	45.888	30.592	15.296	0.612
1	3	0.33	21.470	21.255	16.102	10.735	5.367	0.215
2	3	0.67	61.184	60.572	45.888	30.592	15.296	0.612
3	3	1.0	106.919	105.850	80.189	53.460	26.730	1.069

TABLE 6e. SUMMARY SHEET FOR NATURAL FREQUENCY INCLUDING SHEAR.

$\frac{b/a}{\beta}$	$\hat{\Omega}^2$ (B.S.A. II)					$(h^2/ab=0.05)$
	0.01	0.25	0.50	0.75	0.99	
	(m=1, n=1)					
0.25	11.182	8.471	5.647	2.824	0.114	
0.5	4.575	3.466	2.311	1.155	0.046	
1.0	3.089	2.340	1.560	0.780	0.031	
1.5	3.560	2.697	1.798	0.899	0.036	
2.0	4.575	3.466	2.311	1.155	0.046	
2.5	5.910	4.477	2.985	1.492	0.060	
3.0	7.483	5.669	3.779	1.890	0.076	
3.5	9.251	7.008	4.672	2.336	0.093	
4.0	11.181	8.471	5.647	2.824	0.113	
4.5	13.252	10.040	6.693	3.347	0.134	
5.0	15.445	11.701	7.801	3.900	0.156	
	(m=2, n=1)					
0.25	79.431	60.175	40.117	20.058	0.802	
0.5	32.535	24.648	16.432	8.216	0.329	
1.0	14.516	10.997	7.332	3.666	0.147	
1.5	10.827	8.202	5.468	2.734	0.109	
2.0	10.128	7.673	5.115	2.558	0.102	
2.5	10.546	7.989	5.326	2.663	0.107	
3.0	11.540	8.742	5.828	2.914	0.117	
3.5	12.897	9.771	6.514	3.257	0.130	
4.0	14.516	10.997	7.331	3.666	0.147	
4.5	16.337	12.376	8.251	4.125	0.165	
5.0	18.321	13.879	9.253	4.627	0.185	
	(m=3, n=1)					
0.25	212.880	161.273	107.515	53.758	2.150	
0.5	93.899	71.136	47.424	23.712	0.949	
1.0	41.080	31.121	20.747	10.374	0.415	
1.5	27.066	20.505	13.670	6.835	0.273	
2.0	21.825	16.534	11.023	5.512	0.221	
2.5	19.805	15.004	10.002	5.001	0.200	
3.0	19.307	14.627	9.751	4.876	0.195	
3.5	19.662	14.896	9.930	4.965	0.199	
4.0	20.557	15.573	10.382	5.191	0.208	
4.5	21.825	16.534	11.023	5.511	0.221	
5.0	23.370	17.705	11.803	5.902	0.236	

TABLE 7a. NATURAL FREQUENCY INCLUDING SHEAR.

$\frac{b}{a} \sqrt{\frac{\rho}{E}}$	$\Omega^2$ (B.S.A. TI)					$(h^2/ab=0.05)$
	0.01	0.25	0.50	0.75	0.99	
(m=1, n=2)						
0.25	14.516	10.997	7.332	3.666	0.147	
0.5	10.128	7.673	5.115	2.558	0.102	
1.0	14.516	10.997	7.332	3.666	0.147	
1.5	22.681	17.182	11.455	5.727	0.229	
2.0	32.535	24.648	16.432	8.216	0.329	
2.5	43.415	32.890	21.927	10.963	0.439	
3.0	54.980	41.652	27.768	13.884	0.555	
3.5	67.028	50.779	33.853	16.926	0.677	
4.0	79.430	60.174	40.116	20.058	0.802	
4.5	92.099	69.772	46.515	23.257	0.930	
5.0	104.975	79.526	53.018	26.509	1.050	
(m=2, n=2)						
0.25	84.226	63.808	42.539	21.269	0.851	
0.5	41.080	31.121	20.747	10.374	0.415	
1.0	29.775	22.557	15.038	7.519	0.301	
1.5	33.466	25.353	16.902	8.451	0.338	
2.0	41.079	31.121	20.747	10.374	0.415	
2.5	50.545	38.292	25.528	12.764	0.511	
3.0	61.112	46.297	30.865	15.433	0.617	
3.5	72.410	54.856	36.571	18.285	0.731	
4.0	84.225	63.807	42.538	21.269	0.851	
4.5	96.422	73.047	48.698	24.349	0.974	
5.0	108.909	82.507	55.005	27.502	1.100	
(m=3, n=2)						
0.25	218.007	165.157	110.105	55.052	2.202	
0.5	103.668	78.536	52.357	26.179	1.047	
1.0	59.059	44.747	29.828	14.914	0.597	
1.5	52.958	40.120	26.747	13.373	0.535	
2.0	55.997	42.422	28.281	14.141	0.566	
2.5	62.762	47.547	31.698	15.849	0.634	
3.0	71.510	54.174	36.116	18.058	0.722	
3.5	81.482	61.729	41.152	20.576	0.823	
4.0	92.279	69.908	46.606	23.303	0.932	
4.5	103.666	78.535	52.357	26.178	1.047	
5.0	115.493	87.494	58.330	29.165	1.167	

TABLE 7b. NATURAL FREQUENCY INCLUDING SHEAR.

$\frac{a}{b} \beta$	$\hat{\Omega}^2$ (B.S.A. II)				
	0.01	0.25	0.50	0.75	( $h^2/ab=0.05$ ) 0.99
(m=1, n=3)					
0.25	20.557	15.573	10.382	5.919	0.208
0.5	21.825	16.534	11.023	5.511	0.221
1.0	41.080	31.121	20.747	10.374	0.415
1.5	66.286	50.216	33.478	16.739	0.670
2.0	93.898	71.135	47.423	23.712	0.949
2.5	122.768	93.006	62.004	31.002	1.240
3.0	152.380	115.439	76.959	38.480	1.539
3.5	182.467	138.232	92.155	46.078	1.843
4.0	212.877	161.270	107.514	53.757	2.150
4.5	243.516	184.482	122.988	61.494	2.460
5.0	274.324	207.821	138.547	69.274	2.771
(m=2, n=3)					
0.25	92.280	69.909	46.606	23.303	0.932
0.5	55.997	42.422	28.281	14.141	0.566
1.0	59.059	44.742	29.828	14.914	0.597
1.5	78.900	59.773	39.848	19.924	0.797
2.0	103.667	78.535	52.357	26.179	1.047
2.5	130.741	99.046	66.031	33.015	1.321
3.0	159.112	120.539	80.360	40.180	1.607
3.5	188.289	142.643	95.096	47.548	1.902
4.0	218.004	165.155	110.103	55.052	2.202
4.5	248.095	187.951	125.301	62.650	2.506
5.0	278.460	210.955	140.636	70.318	2.813
(m=3, n=3)					
0.25	226.563	171.639	114.426	57.213	2.289
0.5	120.120	91.000	60.667	30.333	1.213
1.0	90.664	68.685	45.790	22.895	0.916
1.5	100.402	76.062	50.708	25.354	1.014
2.0	120.119	90.999	60.666	30.333	1.213
2.5	144.102	109.168	72.779	36.389	1.456
3.0	170.366	129.066	86.044	43.022	1.721
3.5	198.012	150.009	100.006	50.003	2.000
4.0	226.560	171.637	114.424	57.212	2.289
4.5	255.734	193.738	129.159	64.579	2.583
5.0	285.358	216.180	144.120	72.060	2.882

TABLE 7c. NATURAL FREQUENCY INCLUDING SHEAR.

TABLE 7d. NATURAL FREQUENCY INCLUDING SHEAR.

$\frac{b/a}{n}$	$\hat{\Omega}^2$ (B.S.A. TI. RI) $(h^2/ab=0.05), (\hat{\beta}=0)$		
	1	2	3
	(m=1)		
0.25	11.295	14.663	20.765
0.5	4.621	10.230	22.046
1.0	3.120	14.663	41.495
1.5	3.596	22.910	66.955
2.0	4.621	32.864	94.847
2.5	5.969	43.854	124.008
3.0	7.559	55.536	153.919
3.5	9.344	67.706	184.310
4.0	11.294	80.232	215.027
4.5	13.386	93.029	245.976
5.0	15.601	106.035	277.095
	(m=2)		
0.25	80.234	85.077	93.212
0.5	32.864	41.495	56.562
1.0	14.663	30.076	59.655
1.5	10.936	33.804	79.697
2.0	10.230	41.494	104.714
2.5	10.652	51.055	132.061
3.0	11.656	61.729	160.719
3.5	13.028	73.141	190.191
4.0	14.663	85.076	220.206
4.5	16.502	97.396	250.601
5.0	18.506	110.009	281.273
	(m=3)		
0.25	215.030	220.209	228.852
0.5	94.848	104.715	121.333
1.0	41.495	59.656	91.579
1.5	27.340	53.493	101.416
2.0	22.046	56.562	121.333
2.5	20.005	63.396	145.557
3.0	19.502	72.232	172.087
3.5	19.861	82.305	200.012
4.0	20.764	93.211	228.849
4.5	22.046	104.713	258.317
5.0	23.607	116.659	288.240

TABLE 7d. NATURAL FREQUENCY INCLUDING SHEAR.



$\hat{\Omega}^2$  (B.S.A. TI) MINIMUM

m	n	$\frac{a}{b}$	$(h^2/ab=0.05)$					
			0.0	0.01	0.25	0.50	0.75	0.99
1	1	1.0	3.120	3.089	2.340	1.560	0.780	0.031
2	1	2.0	10.230	10.128	7.673	5.115	2.558	0.102
3	1	3.0	19.502	19.307	14.627	9.751	4.876	0.195
1	2	0.5	10.230	10.128	7.673	5.115	2.558	0.102
2	2	1.0	30.076	29.775	22.557	15.038	7.519	0.301
3	2	1.5	53.493	52.958	40.120	26.747	13.373	0.535
1	3	0.33	19.502	19.307	14.627	9.751	4.876	0.195
2	3	0.67	53.493	52.958	40.120	26.741	13.373	0.5349
3	3	1.0	91.579	90.664	68.685	45.790	22.895	0.916

TABLE 7e. SUMMARY SHEET FOR NATURAL FREQUENCY INCLUDING SHEAR.

$\frac{a/b}{\hat{\alpha}}$	$\hat{\Omega}^2(\text{B.A. TI})$				
	0.01	0.25	0.50	0.75	0.99
(m=1, n=1)					
0.25	17.882	13.547	9.031	4.516	0.181
0.5	6.188	4.688	3.125	1.563	0.063
1.0	3.960	3.000	2.000	1.000	0.040
1.5	4.648	3.521	2.347	1.174	0.047
2.0	6.187	4.688	3.125	1.563	0.063
2.5	8.326	6.307	4.205	2.103	0.084
3.0	11.000	8.333	5.556	2.778	0.111
3.5	14.188	10.749	7.166	3.583	0.143
4.0	17.882	13.547	9.031	4.516	0.181
4.5	22.076	16.724	11.149	5.575	0.223
5.0	26.769	20.280	13.520	6.760	0.270
(m=2, n=1)					
0.25	261.421	198.046	132.031	66.015	2.641
0.5	71.528	54.188	36.125	18.063	0.723
1.0	24.750	18.750	12.500	6.250	0.250
1.5	17.188	13.021	8.681	4.340	0.174
2.0	15.840	12.000	8.000	4.000	0.160
2.5	16.642	12.607	8.405	4.203	0.168
3.0	18.590	14.083	9.389	4.694	0.188
3.5	21.340	16.167	10.778	5.389	0.216
4.0	24.750	18.750	12.500	6.250	0.250
4.5	28.749	21.780	14.520	7.260	0.290
5.0	33.303	25.230	16.820	8.410	0.336
(m=3, n=1)					
0.25	1300.922	985.547	657.031	328.516	13.141
0.5	338.827	256.687	171.125	85.562	3.423
1.0	99.001	75.000	50.000	25.000	1.000
1.5	55.688	42.188	28.125	14.063	0.563
2.0	41.828	31.688	21.125	10.563	0.423
2.5	36.838	27.908	18.605	9.303	0.372
3.0	35.640	27.000	18.000	9.000	0.360
3.5	36.494	27.647	18.431	9.216	0.369
4.0	38.672	29.297	19.531	9.766	0.391
4.5	41.827	31.687	21.125	10.562	0.423
5.0	45.777	34.680	23.120	11.560	0.462

TABLE 8a. NATURAL FREQUENCY EXCLUDING SHEAR.

$\frac{a}{b} \hat{\omega}$	$\hat{\Omega}^2$ (B.A. TI)				
	0.01	0.25	0.50	0.75	0.99
(m=1, n=2)					
0.25	24.750	18.750	12.500	6.250	0.250
0.5	15.840	12.000	8.000	4.000	0.160
1.0	24.750	18.750	12.500	6.250	0.250
1.5	44.000	33.333	22.222	11.111	0.444
2.0	71.527	54.187	36.125	18.062	0.723
2.5	107.077	81.119	54.079	27.040	1.082
3.0	150.587	114.081	76.054	38.027	1.521
3.5	202.037	153.058	102.039	51.019	2.041
4.0	261.415	198.042	132.028	66.014	2.641
4.5	328.720	249.030	166.020	83.010	3.320
5.0	403.949	306.022	204.015	102.007	4.080
(m=2, n=2)					
0.25	286.109	216.750	144.500	72.250	2.890
0.5	99.000	75.000	50.000	25.000	1.000
1.0	63.360	48.000	32.000	16.000	0.640
1.5	74.360	56.333	37.555	18.778	0.751
2.0	98.999	74.999	50.000	25.000	1.000
2.5	133.213	100.919	67.279	33.640	1.346
3.0	175.997	133.331	88.887	44.444	1.778
3.5	227.009	171.976	114.651	57.325	2.293
4.0	286.104	216.745	144.497	72.248	2.890
4.5	353.213	267.586	178.391	89.195	3.568
5.0	428.303	324.572	216.315	108.157	4.326
(m=3, n=2)					
0.25	1355.310	1026.750	684.500	342.250	13.690
0.5	395.999	299.999	200.000	100.000	4.000
1.0	167.310	126.750	84.500	42.240	1.690
1.5	142.560	108.000	72.000	36.000	1.440
2.0	154.687	117.187	78.125	39.062	1.563
2.5	183.109	138.719	92.479	46.240	1.850
3.0	222.747	168.748	112.499	56.249	2.250
3.5	271.861	205.956	137.304	68.652	2.746
4.0	329.726	249.792	166.528	83.264	3.331
4.5	395.992	299.993	199.996	99.998	4.000
5.0	470.477	356.422	237.615	118.807	4.752

TABLE 8b. NATURAL FREQUENCY EXCLUDING SHEAR.

$\frac{a}{b} \sqrt{\frac{E}{G}}$	$\hat{\Omega}^2$ (B.A. TI)				
	0.01 (m=1, n=3)	0.25	0.50	0.75	0.99
0.25	38.672	29.297	19.531	9.766	0.391
0.5	41.828	31.688	21.125	10.563	0.423
1.0	99.000	75.000	50.000	25.000	1.000
1.5	198.685	150.519	100.346	50.173	2.007
2.0	338.822	256.683	171.122	85.561	3.423
2.5	519.155	393.299	262.200	131.100	5.244
3.0	739.624	560.321	373.547	186.774	7.471
3.5	1000.204	757.731	505.154	252.577	10.103
4.0	1300.891	985.523	657.016	328.508	13.140
4.5	1641.675	1243.693	829.129	414.564	16.583
5.0	2022.559	1532.241	1021.494	510.747	20.430
	(m=2, n=3)				
0.25	329.731	249.796	166.531	83.265	3.331
0.5	154.688	117.188	78.125	39.063	1.563
1.0	167.310	126.750	84.500	42.250	1.690
1.5	258.745	196.019	130.679	65.340	2.614
2.0	395.994	299.996	199.997	99.999	4.000
2.5	574.991	435.599	290.399	145.200	5.808
3.0	794.734	602.071	401.381	200.690	8.028
3.5	1054.877	799.149	532.766	266.383	10.655
4.0	1355.279	1026.726	684.484	342.242	13.690
4.5	1695.868	1284.749	856.499	428.250	17.130
5.0	2076.612	1573.191	1048.794	524.397	20.976
	(m=3, n=3)				
0.25	1448.432	1097.297	731.531	365.766	14.631
0.5	501.187	379.687	253.125	126.562	5.063
1.0	320.759	243.000	162.000	81.000	3.240
1.5	376.446	285.186	190.124	95.062	3.803
2.0	501.182	379.683	253.122	126.561	5.063
2.5	674.387	510.899	340.599	170.300	6.812
3.0	890.984	674.988	449.992	224.996	9.000
3.5	1149.231	870.629	580.419	290.210	11.608
4.0	1448.400	1097.273	731.515	365.758	14.630
4.5	1788.146	1354.656	903.104	451.552	18.062
5.0	2168.287	1642.641	1095.094	547.547	21.902

TABLE 8c. NATURAL FREQUENCY EXCLUDING SHEAR.

TABLE 8L. NATURAL FREQUENCY INCLUDING SHEAR.

$\frac{b}{a} n$	$\hat{\Omega}^2$ (B.A. TI)	$(\hat{\alpha}=0)$		
		1	2	3
	(m=1)			
0.25	18.063	25.000	39.063	
0.5	6.250	16.000	42.250	
1.0	4.000	25.000	100.000	
1.5	4.694	44.444	200.692	
2.0	6.250	72.249	342.244	
2.5	8.410	108.158	524.399	
3.0	11.111	152.108	747.095	
3.5	14.331	204.077	1010.308	
4.0	18.063	264.056	1314.032	
4.5	22.299	332.041	1658.258	
5.0	27.039	408.029	2042.989	
	(m=2)			
0.25	264.062	288.999	333.062	
0.5	72.250	100.000	156.250	
1.0	25.000	64.000	169.000	
1.5	17.361	75.111	261.359	
2.0	16.000	100.000	399.995	
2.5	16.810	134.558	580.799	
3.0	18.778	177.775	802.762	
3.5	21.556	229.302	1065.533	
4.0	25.000	288.994	1368.968	
4.5	29.040	356.781	1712.998	
5.0	33.639	432.629	2097.588	
	(m=3)			
0.25	1314.063	1369.000	1463.063	
0.5	342.249	399.999	506.249	
1.0	100.000	169.000	324.000	
1.5	56.250	144.000	380.248	
2.0	42.250	156.249	506.244	
2.5	37.210	184.958	681.199	
3.0	36.000	224.997	899.984	
3.5	36.862	274.607	1160.839	
4.0	39.062	333.056	1463.031	
4.5	42.250	399.992	1806.209	
5.0	46.240	475.229	2190.189	

TABLE 8d. NATURAL FREQUENCY EXCLUDING SHEAR.

m	n	$\frac{a}{b} \sqrt{\frac{E}{G}}$	$\frac{\Delta^2}{\Omega^2}$ (B.A. TI) MINIMUM					
			0.0	0.01	0.25	0.50	0.75	0.99
1	1	1.0	4.000	3.960	3.000	2.000	1.000	0.040
2	1	2.0	16.000	15.840	12.000	8.000	4.000	0.160
3	1	3.0	36.000	35.640	27.000	18.000	9.000	0.360
1	2	0.50	16.000	15.840	12.000	8.000	4.000	0.160
2	2	1.0	64.000	63.360	48.000	32.000	16.000	0.640
3	2	1.50	144.000	142.560	108.000	72.000	36.000	1.440
1	3	0.33	36.000	35.640	27.000	18.000	9.000	0.360
2	3	0.67	144.000	142.560	108.000	72.000	36.000	1.440
3	3	1.00	324.000	320.759	243.00	162.000	81.000	3.240

TABLE 8e. SUMMARY SHEET FOR NATURAL FREQUENCY EXCLUDING SHEAR.

$\frac{\hat{\alpha}}{\hat{\beta}}$	0.01	0.25	$\hat{\Omega}^2$ 0.50	0.75	$(h^2/ab=0.01)$ 0.99
	(m=1, n=2)				
0.25	0.8920	0.8622	0.8067	0.6760	0.0770
0.5	0.9335	0.9141	0.8764	0.7800	0.1242
1.0	0.9461	0.9301	0.8986	0.8159	0.1506
1.5	0.9419	0.9247	0.8911	0.8036	0.1407
2.0	0.9335	0.9141	0.8764	0.7800	0.1242
2.5	0.9237	0.9017	0.8594	0.7535	0.1090
3.0	0.9133	0.8886	0.8418	0.7268	0.0962
3.5	0.9027	0.8754	0.8241	0.7008	0.0857
4.0	0.8920	0.8622	0.8067	0.6760	0.0770
4.5	0.8814	0.8492	0.7897	0.6525	0.0699
5.0	0.8710	0.8365	0.7732	0.6303	0.0638
	(m=2, n=1)				
0.25	0.6836	0.6207	0.5218	0.3530	0.0214
0.5	0.8051	0.7578	0.6760	0.5105	0.0400
1.0	0.8753	0.8418	0.7800	0.6394	0.0662
1.5	0.8939	0.8646	0.8097	0.6803	0.0784
2.0	0.8977	0.8693	0.8159	0.6891	0.0814
2.5	0.8954	0.8664	0.8122	0.6838	0.0796
3.0	0.8901	0.8599	0.8036	0.6717	0.0756
3.5	0.8832	0.8514	0.7925	0.6563	0.0710
4.0	0.8753	0.8418	0.7800	0.6394	0.0662
4.5	0.8669	0.8315	0.7669	0.6220	0.0617
5.0	0.8582	0.8210	0.7535	0.6045	0.0576
	(m=3, n=1)				
0.25	0.4920	0.4232	0.3285	0.1965	0.0097
0.5	0.6549	0.5898	0.4894	0.3240	0.0188
1.0	0.7783	0.7268	0.6394	0.4699	0.0342
1.5	0.8240	0.7800	0.7027	0.5417	0.0451
2.0	0.8438	0.8036	0.7318	0.5770	0.0517
2.5	0.8520	0.8134	0.7440	0.5924	0.0549
3.0	0.8540	0.8159	0.7472	0.5964	0.0558
3.5	0.8526	0.8141	0.7449	0.5935	0.0552
4.0	0.8489	0.8097	0.7394	0.5865	0.0537
4.5	0.8438	0.8036	0.7318	0.5770	0.0517
5.0	0.8377	0.7964	0.7228	0.5659	0.0496

TABLE 9a. NATURAL FREQUENCY RATIO

$\frac{a/b}{\hat{\beta}}$	0.01	0.25	$\hat{\Omega}^2$ 0.50	0.75	$(h^2/ab=0.01)$ 0.99
(m=1, n=2)					
0.25	0.8763	0.8418	0.7800	0.6394	0.0662
0.5	0.8977	0.8693	0.8159	0.6891	0.0814
1.0	0.8753	0.8418	0.7800	0.6394	0.0662
1.5	0.8404	0.7996	0.7268	0.5708	0.0505
2.0	0.8051	0.7578	0.6760	0.5105	0.0401
2.5	0.7715	0.7189	0.6303	0.4602	0.0330
3.0	0.7400	0.6832	0.5898	0.4182	0.0279
3.5	0.7108	0.6506	0.5538	0.3829	0.0242
4.0	0.6836	0.6207	0.5218	0.3530	0.0214
4.5	0.6583	0.5934	0.4932	0.3273	0.0191
5.0	0.6348	0.5683	0.4675	0.3050	0.0173
(m=2, n=2)					
0.25	0.6738	0.6101	0.5105	0.3428	0.0204
0.5	0.7783	0.7268	0.6394	0.4699	0.0342
1.0	0.8144	0.7688	0.6891	0.5257	0.0424
1.5	0.8020	0.7542	0.6717	0.5057	0.0393
2.0	0.7783	0.7268	0.6394	0.4699	0.0342
2.5	0.7516	0.6963	0.6045	0.4332	0.0297
3.0	0.7248	0.6661	0.5708	0.3994	0.0259
3.5	0.6987	0.6372	0.5394	0.3693	0.0229
4.0	0.6738	0.6101	0.5105	0.3428	0.0204
4.5	0.6502	0.5847	0.4842	0.3194	0.0184
5.0	0.6280	0.5612	0.4602	0.2989	0.0168
(m=3, n=2)					
0.25	0.4869	0.4182	0.3240	0.1933	0.0095
0.5	0.6371	0.5708	0.4699	0.3071	0.0174
1.0	0.7298	0.6717	0.5770	0.4055	0.0266
1.5	0.7453	0.6891	0.5964	0.4249	0.0287
2.0	0.7374	0.6803	0.5865	0.4149	0.0276
2.5	0.7208	0.6617	0.5659	0.3946	0.0254
3.0	0.7006	0.6394	0.5417	0.3715	0.0231
3.5	0.6793	0.6161	0.5769	0.3485	0.0210
4.0	0.6580	0.5931	0.4928	0.3270	0.0191
4.5	0.6371	0.5708	0.4699	0.3071	0.0174
5.0	0.6169	0.5496	0.4485	0.2891	0.0160

TABLE 9b. NATURAL FREQUENCY RATIO.



$\frac{a}{b}$	$\hat{\Omega}^2$					$(h^2/ab=0.01)$ 0.99
	0.01 (m=1, n=3)	0.25	0.50	0.75		
0.25	0.8489	0.8097	0.7394	0.5865	0.0537	
0.5	0.8438	0.8036	0.7318	0.5770	0.0517	
1.0	0.7783	0.7268	0.6394	0.4699	0.0342	
1.5	0.7125	0.6525	0.5559	0.3849	0.0244	
2.0	0.6549	0.5898	0.4894	0.3240	0.0188	
2.5	0.6052	0.5373	0.4364	0.2791	0.0152	
3.0	0.5623	0.4932	0.3935	0.2449	0.0128	
3.5	0.5248	0.4556	0.3581	0.2181	0.0110	
4.0	0.4920	0.4232	0.3285	0.1965	0.0097	
4.5	0.4630	0.3951	0.3033	0.1788	0.0086	
5.0	0.4372	0.3704	0.2818	0.1640	0.0078	
(m=2, n=3)						
0.25	0.6580	0.5931	0.4928	0.3270	0.0191	
0.5	0.7374	0.6803	0.5865	0.4149	0.0276	
1.0	0.7298	0.6717	0.5770	0.4055	0.0266	
1.5	0.6847	0.6220	0.5231	0.3542	0.0215	
2.0	0.6371	0.5708	0.4699	0.3071	0.0174	
2.5	0.5930	0.5246	0.4239	0.2689	0.0145	
3.0	0.5534	0.4892	0.3849	0.2383	0.0124	
3.5	0.5182	0.4490	0.3520	0.2136	0.0107	
4.0	0.4869	0.4182	0.3240	0.1933	0.0095	
4.5	0.4589	0.3912	0.2999	0.1764	0.0085	
5.0	0.4339	0.3674	0.2791	0.1622	0.0077	
(m=3, n=3)						
0.25	0.4786	0.4101	0.3167	0.1882	0.0092	
0.5	0.6094	0.5417	0.4407	0.2827	0.0155	
1.0	0.6611	0.5964	0.4962	0.3300	0.0193	
1.5	0.6429	0.5770	0.4762	0.3125	0.0179	
2.0	0.6094	0.5417	0.4407	0.2827	0.0155	
2.5	0.5736	0.5047	0.4045	0.2536	0.0134	
3.0	0.5392	0.4699	0.3715	0.2281	0.0117	
3.5	0.5075	0.4384	0.3423	0.2065	0.0103	
4.0	0.4786	0.4101	0.3167	0.1882	0.0092	
4.5	0.4524	0.3849	0.2944	0.1726	0.0083	
5.0	0.4286	0.3624	0.2748	0.1593	0.0075	

TABLE 9c. NATURAL FREQUENCY RATIO.

$\frac{a}{b} \cdot n$	$\hat{\Omega}^2$	$(h^2/ab=0.01), (\hat{\beta}=0)$		
	(m=1)	1	2	3
0.25	0.8930	0.8930	0.8764	0.8502
0.5	0.9341	0.9341	0.8966	0.8451
1.0	0.9466	0.9466	0.8764	0.7800
1.5	0.9424	0.9424	0.8418	0.7145
2.0	0.9341	0.9341	0.8067	0.6572
2.5	0.9244	0.9244	0.7732	0.6076
3.0	0.9141	0.9141	0.7420	0.5647
3.5	0.9035	0.9035	0.7128	0.5273
4.0	0.8930	0.8930	0.6858	0.4945
4.5	0.8825	0.8825	0.6606	0.4655
5.0	0.8721	0.8721	0.6371	0.4396
(m=2)				
0.25	0.6858	0.6858	0.6760	0.6602
0.5	0.8067	0.8067	0.7800	0.7394
1.0	0.8764	0.8764	0.8159	0.7318
1.5	0.8949	0.8949	0.8036	0.6869
2.0	0.8986	0.8986	0.7800	0.6394
2.5	0.8964	0.8964	0.7535	0.5954
3.0	0.8911	0.8911	0.7268	0.5559
3.5	0.8842	0.8842	0.7008	0.5207
4.0	0.8764	0.8764	0.6760	0.4894
4.5	0.8681	0.8681	0.6525	0.4614
5.0	0.8594	0.8594	0.6303	0.4364
(m=3)				
0.25	0.4945	0.4945	0.4894	0.4811
0.5	0.6572	0.6572	0.6394	0.6118
1.0	0.7800	0.7800	0.7318	0.6633
1.5	0.8254	0.8254	0.7472	0.6452
2.0	0.8451	0.8451	0.7394	0.6118
2.5	0.8532	0.8532	0.7228	0.5760
3.0	0.8553	0.8553	0.7027	0.5417
3.5	0.8538	0.8538	0.6815	0.5100
4.0	0.8502	0.8502	0.6602	0.4811
4.5	0.8451	0.8451	0.6394	0.4549
5.0	0.8391	0.8391	0.6193	0.4311

TABLE 9d. NATURAL FREQUENCY RATIO.

m	n	$\frac{h^2}{ab}$	$\hat{\Omega}^2$ MAXIMUM					
			( $h^2/ab=0.01$ ) 0.0	0.01	0.25	0.50	0.75	0.99
1	1	1.0	0.9466	0.9461	0.9301	0.8986	0.8159	0.1506
2	1	2.0	0.8986	0.8977	0.8693	0.8159	0.6891	0.0814
3	1	3.0	0.8553	0.8540	0.8159	0.7472	0.5964	0.0558
1	2	0.5	0.8986	0.8977	0.8693	0.8159	0.6891	0.0814
2	2	1.0	0.8159	0.8144	0.7268	0.6891	0.5257	0.0424
3	2	1.5	0.7472	0.7453	0.6891	0.5964	0.4249	0.0287
1	3	0.33	0.8553	0.8540	0.8159	0.7472	0.5964	0.0558
2	3	0.67	0.7472	0.7453	0.6891	0.5964	0.4249	0.0287
3	3	1.0	0.6633	0.6611	0.5964	0.4962	0.3300	0.0193

TABLE 9e. SUMMARY SHEET FOR NATURAL FREQUENCY SQUARE RATIO.

$\frac{a}{b} \backslash \beta$	$\hat{\Omega}^2$					$(h^2/ab=0.02)$ 0.99
	0.01	0.25	0.50	0.75	0.99	
(m=1, n=1)						
0.25	0.8051	0.7578	0.6760	0.5105	0.0770	
0.5	0.8753	0.8418	0.7800	0.6394	0.1242	
1.0	0.8977	0.8693	0.8159	0.6891	0.1506	
1.5	0.8901	0.8599	0.8036	0.6717	0.1407	
2.0	0.8752	0.8418	0.7800	0.6394	0.1242	
2.5	0.8582	0.8210	0.7535	0.6045	0.1090	
3.0	0.8404	0.7996	0.7268	0.5708	0.0962	
3.5	0.8226	0.7784	0.7008	0.5394	0.0857	
4.0	0.8051	0.7578	0.6760	0.5105	0.0770	
4.5	0.7880	0.7380	0.6525	0.4842	0.0699	
5.0	0.7715	0.7189	0.6303	0.4602	0.0638	
(m=2, n=1)						
0.25	0.5193	0.4501	0.3530	0.2143	0.0214	
0.5	0.6738	0.6101	0.5105	0.3428	0.0400	
1.0	0.7783	0.7268	0.6394	0.4699	0.0662	
1.5	0.8082	0.7614	0.6803	0.5155	0.0784	
2.0	0.8144	0.7688	0.6891	0.5257	0.0814	
2.5	0.8107	0.7643	0.6838	0.5195	0.0796	
3.0	0.8020	0.7542	0.6717	0.5057	0.0756	
3.5	0.7908	0.7412	0.6563	0.4884	0.0710	
4.0	0.7783	0.7268	0.6394	0.4699	0.0662	
4.5	0.7651	0.7116	0.6220	0.4513	0.0617	
5.0	0.7516	0.6963	0.6045	0.4332	0.0576	
(m=3, n=1)						
0.25	0.3263	0.2684	0.1965	0.1090	0.0097	
0.5	0.4869	0.4182	0.3240	0.1933	0.0188	
1.0	0.6371	0.5708	0.4699	0.3071	0.0342	
1.5	0.7006	0.6394	0.5417	0.3715	0.0451	
2.0	0.7298	0.6717	0.5770	0.4055	0.0517	
2.5	0.7421	0.6855	0.5924	0.4209	0.0549	
3.0	0.7453	0.6891	0.5964	0.4249	0.0558	
3.5	0.7430	0.6866	0.5935	0.4220	0.0552	
4.0	0.7374	0.6803	0.5865	0.4149	0.0537	
4.5	0.7298	0.6717	0.5770	0.4055	0.0517	
5.0	0.7208	0.6617	0.5659	0.3946	0.0496	

TABLE 10a. NATURAL FREQUENCY RATIO.

$\frac{a/b}{\hat{\beta}}$	0.01	0.25	$\hat{\Omega}^2$ 0.50	0.75	$(h^2/ab=0.02)$ 0.99
(m=1, n=2)					
0.25	0.7783	0.7268	0.6394	0.4699	0.0662
0.5	0.8144	0.7688	0.6891	0.5257	0.0814
1.0	0.7783	0.7268	0.6394	0.4699	0.0662
1.5	0.7248	0.6661	0.5708	0.3994	0.0505
2.0	0.6738	0.6101	0.5105	0.3428	0.0401
2.5	0.6280	0.5612	0.4602	0.2989	0.0330
3.0	0.5873	0.5188	0.4182	0.2644	0.0279
3.5	0.5513	0.4821	0.3829	0.2368	0.0242
4.0	0.5193	0.4501	0.3530	0.2143	0.0214
4.5	0.4907	0.4219	0.3273	0.1957	0.0191
5.0	0.4650	0.3970	0.3050	0.1800	0.0173
(m=2, n=2)					
0.25	0.5080	0.4389	0.3428	0.2068	0.0204
0.5	0.6371	0.5708	0.4699	0.3071	0.0342
1.0	0.6869	0.6244	0.5257	0.3565	0.0424
1.5	0.6695	0.6054	0.5057	0.3384	0.0393
2.0	0.6371	0.5708	0.4699	0.3071	0.0342
2.5	0.6021	0.5341	0.4332	0.2765	0.0297
3.0	0.5684	0.4993	0.3994	0.2495	0.0259
3.5	0.5369	0.4676	0.3693	0.2264	0.0229
4.0	0.5080	0.4389	0.3428	0.2068	0.0204
4.5	0.4817	0.4132	0.3194	0.1901	0.0184
5.0	0.4577	0.3900	0.2989	0.1757	0.0168
(m=3, n=2)					
0.25	0.3218	0.2644	0.1933	0.1070	0.0095
0.5	0.4674	0.3994	0.3071	0.1814	0.0174
1.0	0.5745	0.5057	0.4055	0.2543	0.0266
1.5	0.5940	0.5257	0.4249	0.2698	0.0287
2.0	0.5841	0.5155	0.4149	0.2618	0.0276
2.5	0.5635	0.4944	0.3946	0.2458	0.0254
3.0	0.5392	0.4699	0.3715	0.2281	0.0231
3.5	0.5144	0.4452	0.3485	0.2110	0.0210
4.0	0.4903	0.4215	0.3270	0.1954	0.0191
4.5	0.4674	0.3994	0.3071	0.1814	0.0174
5.0	0.4461	0.3789	0.2891	0.1690	0.0160

TABLE 10b. NATURAL FREQUENCY RATIO.

$\frac{a/b}{\hat{\beta}}$	$\hat{\Omega}^2$				
	0.01	0.25	0.50	0.75	$(h^2/ab=0.02)$ 0.99
(m=1, n=3)					
0.25	0.7374	0.6803	0.5865	0.4149	0.0537
0.5	0.7298	0.6717	0.5770	0.4055	0.0517
1.0	0.6371	0.5708	0.4699	0.3071	0.0342
1.5	0.5534	0.4842	0.3849	0.2383	0.0244
2.0	0.4869	0.4182	0.3240	0.1933	0.0188
2.5	0.4339	0.3674	0.2791	0.1622	0.0152
3.0	0.3911	0.3273	0.2449	0.1395	0.0128
3.5	0.3558	0.2950	0.2181	0.1224	0.0110
4.0	0.3263	0.2684	0.1965	0.1090	0.0097
4.5	0.3012	0.2462	0.1788	0.0982	0.0083
5.0	0.2797	0.2273	0.1640	0.0893	0.0078
(m=2, n=3)					
0.25	0.4903	0.4215	0.3270	0.1954	0.0191
0.5	0.5841	0.5155	0.4149	0.2618	0.0276
1.0	0.5735	0.5057	0.4055	0.2543	0.0266
1.5	0.5206	0.4513	0.3542	0.2152	0.0215
2.0	0.4674	0.3994	0.3071	0.1814	0.0174
2.5	0.4214	0.3556	0.2689	0.1554	0.0145
3.0	0.3825	0.3194	0.2383	0.1353	0.0124
3.5	0.3497	0.2895	0.2136	0.1196	0.0107
4.0	0.3218	0.2644	0.1933	0.1070	0.0095
4.5	0.2978	0.2432	0.1764	0.0967	0.0085
5.0	0.2771	0.2250	0.1622	0.0882	0.0077
(m=3, n=3)					
0.25	0.3146	0.2580	0.1882	0.1039	0.0092
0.5	0.4383	0.3715	0.2827	0.1646	0.0155
1.0	0.4937	0.4249	0.3300	0.1976	0.0193
1.5	0.4737	0.4055	0.3125	0.1852	0.0179
2.0	0.4383	0.3715	0.2827	0.1646	0.0155
2.5	0.4021	0.3375	0.2536	0.1452	0.0134
3.0	0.3691	0.3071	0.2281	0.1287	0.0117
3.5	0.3400	0.2807	0.2065	0.1151	0.0103
4.0	0.3146	0.2580	0.1882	0.1039	0.0092
4.5	0.2933	0.2383	0.1726	0.0945	0.0083
5.0	0.2728	0.2213	0.1593	0.0865	0.0075

TABLE 10c. NATURAL FREQUENCY RATIO.

TABLE 10d. NATURAL FREQUENCY RATIO.

$\frac{cL}{b}$	$n$	$\hat{\Omega}^2$ ( $h^2/ab=0.02, \hat{\beta}=0$ )		
		1	2	3
(m=1)				
0.25		0.8067	0.7800	0.7394
0.5		0.8764	0.8159	0.7318
1.0		0.8986	0.7800	0.6394
1.5		0.8911	0.7268	0.5550
2.0		0.8764	0.6760	0.4894
2.5		0.8594	0.6303	0.4364
3.0		0.8418	0.5898	0.3935
3.5		0.8241	0.5538	0.3581
4.0		0.8067	0.5218	0.3285
4.5		0.7897	0.4932	0.3033
5.0		0.7732	0.4675	0.2818
(m=2)				
0.25		0.5218	0.5105	0.4928
0.5		0.6760	0.6394	0.5865
1.0		0.7800	0.6891	0.5770
1.5		0.8097	0.6717	0.5231
2.0		0.8159	0.6394	0.4699
2.5		0.8122	0.6045	0.4239
3.0		0.8036	0.5708	0.3849
3.5		0.7925	0.5394	0.3520
4.0		0.7800	0.5105	0.3240
4.5		0.7669	0.4842	0.2000
5.0		0.7535	0.4602	0.2791
(m=3)				
0.25		0.3285	0.3240	0.3167
0.5		0.4849	0.4699	0.4407
1.0		0.6394	0.5770	0.4962
1.5		0.7027	0.5964	0.4762
2.0		0.7317	0.5865	0.4407
2.5		0.7440	0.5659	0.4045
3.0		0.7472	0.5417	0.3715
3.5		0.7449	0.5169	0.3423
4.0		0.7394	0.4928	0.3167
4.5		0.7319	0.4699	0.2944
5.0		0.7228	0.4485	0.2748

TABLE 10d. NATURAL FREQUENCY RATIO.

$\hat{\Omega}^2$  MAXIMUM

m	n	$\frac{a}{b}$	$(h^2/ab=0.02)$					
			0.0	0.01	0.25	0.50	0.75	0.99
1	1	1.0	0.8986	0.8977	0.8693	0.8159	0.6891	0.0814
2	1	2.0	0.8159	0.8144	0.7688	0.6891	0.5257	0.0424
3	1	3.0	0.7472	0.7453	0.6891	0.5964	0.4249	0.0287
1	2	0.5	0.8159	0.8144	0.7688	0.6891	0.5257	0.0424
2	2	1.0	0.6891	0.6869	0.6244	0.5257	0.3565	0.0217
3	2	1.5	0.5964	0.5940	0.5257	0.4249	0.2698	0.0146
1	3	0.33	0.7472	0.7453	0.6891	0.5964	0.4249	0.0287
2	3	0.67	0.5964	0.5940	0.5257	0.4249	0.2698	0.0146
3	3	1.0	0.4962	0.4937	0.4249	0.3300	0.1976	0.0098

TABLE 10e. SUMMARY SHEET FOR NATURAL FREQUENCY SQUARE RATIO.



$\frac{\hat{\beta}}{a/b}$	0.01	0.25	$\hat{\Omega}^2$ 0.50	0.75	$(h^2/ab=0.03)$ 0.99
(m=1, n=1)					
0.25	0.7336	0.6760	0.5817	0.4101	0.0271
0.5	0.8240	0.7800	0.7027	0.5417	0.0451
1.0	0.8540	0.8195	0.7472	0.5964	0.0558
1.5	0.8438	0.8036	0.7318	0.5770	0.0517
2.0	0.8240	0.7800	0.7027	0.5417	0.0451
2.5	0.8014	0.7535	0.6708	0.5047	0.0392
3.0	0.7783	0.7268	0.6394	0.4699	0.0342
3.5	0.7556	0.7008	0.6096	0.4384	0.0303
4.0	0.7336	0.6760	0.5817	0.4102	0.0271
4.5	0.7125	0.6525	0.5559	0.3849	0.0244
5.0	0.6924	0.6303	0.5320	0.3624	0.0222
(m=2, n=1)					
0.25	0.4187	0.3530	0.2667	0.1539	0.0072
0.5	0.5793	0.5105	0.4101	0.2580	0.0137
1.0	0.7006	0.6394	0.5417	0.3715	0.0231
1.5	0.7374	0.6803	0.5865	0.4149	0.0276
2.0	0.7453	0.6891	0.5964	0.4229	0.0287
2.5	0.7405	0.6838	0.5904	0.4189	0.0280
3.0	0.7298	0.6717	0.5770	0.4055	0.0266
3.5	0.7160	0.6563	0.5601	0.3889	0.0248
4.0	0.7006	0.6394	0.5417	0.3715	0.0231
4.5	0.6847	0.6220	0.5231	0.3542	0.0215
5.0	0.6686	0.6045	0.5047	0.3375	0.0200
(m=3, n=1)					
0.25	0.2440	0.1965	0.1402	0.0754	0.0033
0.5	0.3875	0.3240	0.2421	0.1377	0.0063
1.0	0.5392	0.4699	0.3715	0.2281	0.0117
1.5	0.6094	0.5417	0.4407	0.2827	0.0155
2.0	0.6429	0.5770	0.4762	0.3125	0.0179
2.5	0.6574	0.5924	0.4921	0.3264	0.0190
3.0	0.6611	0.5964	0.4962	0.3300	0.0193
3.5	0.6584	0.5935	0.4933	0.3274	0.0191
4.0	0.6519	0.5865	0.4860	0.3210	0.0186
4.5	0.6429	0.5770	0.4762	0.3125	0.0179
5.0	0.6325	0.5659	0.4650	0.3029	0.0171

TABLE 11a. NATURAL FREQUENCY RATIO.

$\frac{a}{b} \backslash \hat{\beta}$	$\hat{\Omega}^2$					$(h^2/ab=0.03)$ 0.99
	0.01	0.25	0.50	0.75		
(m=1, n=2)						
0.25	0.7006	0.6394	0.5417	0.3715	0.0231	
0.5	0.7453	0.6891	0.5964	0.4249	0.0287	
1.0	0.7006	0.6394	0.5417	0.3715	0.0231	
1.5	0.6371	0.5708	0.4699	0.3071	0.0175	
2.0	0.5793	0.5105	0.4101	0.2580	0.0137	
2.5	0.5295	0.4602	0.3264	0.2213	0.0112	
3.0	0.4869	0.4182	0.3240	0.1933	0.0095	
3.5	0.4503	0.3829	0.2927	0.1714	0.0082	
4.0	0.4187	0.3530	0.2667	0.1539	0.0072	
4.5	0.3911	0.3273	0.2449	0.1395	0.0064	
5.0	0.3668	0.3050	0.2264	0.1276	0.0058	
(m=2, n=2)						
0.25	0.4077	0.3428	0.2580	0.1481	0.0069	
0.5	0.5392	0.4699	0.3715	0.2281	0.0117	
1.0	0.5940	0.5257	0.4249	0.2698	0.0146	
1.5	0.5745	0.5057	0.4055	0.2543	0.0135	
2.0	0.5392	0.4699	0.3715	0.2281	0.0117	
2.5	0.5022	0.4332	0.3375	0.2030	0.0101	
3.0	0.4674	0.3994	0.3071	0.1814	0.0088	
3.5	0.4359	0.3693	0.2807	0.1633	0.0077	
4.0	0.4077	0.3428	0.2580	0.1481	0.0069	
4.5	0.3825	0.3194	0.2383	0.1353	0.0062	
5.0	0.3601	0.2989	0.2213	0.1244	0.0057	
(m=3, n=2)						
0.25	0.2403	0.1933	0.1377	0.0740	0.0032	
0.5	0.3691	0.3071	0.2281	0.1287	0.0059	
1.0	0.4737	0.4055	0.3125	0.1852	0.0090	
1.5	0.4937	0.4249	0.3300	0.1976	0.0098	
2.0	0.4835	0.4149	0.3210	0.1912	0.0094	
2.5	0.4625	0.3946	0.3029	0.1785	0.0086	
3.0	0.4383	0.3715	0.2827	0.1646	0.0078	
3.5	0.4139	0.3485	0.2629	0.1513	0.0071	
4.0	0.3907	0.3270	0.2446	0.1394	0.0064	
4.5	0.3691	0.3071	0.2281	0.1287	0.0059	
5.0	0.3493	0.2891	0.2133	0.1194	0.0054	

TABLE 11c. NATURAL FREQUENCY RATIO.

TABLE 11b. NATURAL FREQUENCY RATIO.

$\frac{a/b}{\hat{\beta}}$	$\hat{\Omega}^2$					$(h^2/ab=0.03)$ 0.99
	0.01	0.25	0.50	0.75		
(m=1, n=3)						
0.25	0.6519	0.5865	0.4860	0.3210	0.0186	
0.5	0.6429	0.5770	0.4762	0.3125	0.0179	
1.0	0.5392	0.4699	0.3715	0.2281	0.0117	
1.5	0.4524	0.3849	0.2944	0.1726	0.0083	
2.0	0.3875	0.3240	0.2421	0.1377	0.0063	
2.5	0.3382	0.2791	0.2051	0.1143	0.0051	
3.0	0.2998	0.2449	0.1778	0.0976	0.0043	
3.5	0.2691	0.2181	0.1568	0.0851	0.0037	
4.0	0.2440	0.1965	0.1402	0.0754	0.0033	
4.5	0.2232	0.1788	0.1267	0.0677	0.0029	
5.0	0.2057	0.1640	0.1156	0.0614	0.0026	
(m=2, n=3)						
0.25	0.3907	0.3270	0.2546	0.1394	0.0064	
0.5	0.4835	0.4149	0.3210	0.1912	0.0094	
1.0	0.4737	0.4055	0.3125	0.1852	0.0090	
1.5	0.4199	0.3542	0.2677	0.1545	0.0073	
2.0	0.3691	0.3071	0.2281	0.1287	0.0059	
2.5	0.3269	0.2689	0.1969	0.1092	0.0049	
3.0	0.2923	0.2383	0.1726	0.0945	0.0042	
3.5	0.2639	0.2136	0.1533	0.0830	0.0036	
4.0	0.2403	0.1933	0.1377	0.0740	0.0032	
4.5	0.2204	0.1764	0.1250	0.0666	0.0028	
5.0	0.2035	0.1622	0.1143	0.0606	0.0026	
(m=3, n=3)						
0.25	0.2343	0.1882	0.1338	0.0717	0.0031	
0.5	0.3422	0.2827	0.2080	0.1161	0.0052	
1.0	0.3940	0.3300	0.2472	0.1410	0.0065	
1.5	0.3751	0.3125	0.2326	0.1316	0.0060	
2.0	0.3422	0.2827	0.2080	0.1161	0.0052	
2.5	0.3096	0.2536	0.1846	0.1017	0.0045	
3.0	0.2806	0.2281	0.1646	0.0897	0.0039	
3.5	0.2557	0.2065	0.1478	0.0798	0.0035	
4.0	0.2343	0.1882	0.1338	0.0717	0.0031	
4.5	0.2159	0.1726	0.1221	0.0650	0.0028	
5.0	0.2000	0.1593	0.1121	0.0594	0.0025	

TABLE 11c. NATURAL FREQUENCY RATIO.

TABLE 11d. NATURAL FREQUENCY RATIO.

		$\hat{\Omega}^2$ ( $h^2/ab=0.03, \hat{\beta}=0$ )		
$b/a \backslash n$		1	2	3
(m=1)				
0.25		0.7355	0.7027	0.6541
0.5		0.8254	0.7472	0.6452
1.0		0.8553	0.7027	0.5417
1.5		0.8451	0.6394	0.4549
2.0		0.8254	0.5817	0.3899
2.5		0.8030	0.5320	0.3405
3.0		0.7800	0.4894	0.3019
3.5		0.7574	0.4528	0.2711
4.0		0.7355	0.4211	0.2459
4.5		0.7146	0.3935	0.2250
5.0		0.6945	0.3692	0.2073
(m=2)				
0.25		0.4211	0.4101	0.3931
0.5		0.5817	0.5417	0.4860
1.0		0.7027	0.5964	0.4762
1.5		0.7394	0.5770	0.4224
2.0		0.7472	0.5417	0.3715
2.5		0.7425	0.5047	0.3291
3.0		0.7318	0.4699	0.2944
3.5		0.7180	0.4384	0.2659
4.0		0.7027	0.4101	0.2421
4.5		0.6869	0.3849	0.2222
5.0		0.6708	0.3624	0.2051
(m=3)				
0.25		0.2459	0.2421	0.2361
0.5		0.3899	0.3715	0.3444
1.0		0.5417	0.4762	0.3964
1.5		0.6118	0.4962	0.3774
2.0		0.6452	0.4860	0.3444
2.5		0.6596	0.4650	0.3117
3.0		0.6633	0.4407	0.2827
3.5		0.6607	0.4163	0.2576
4.0		0.6541	0.3931	0.2361
4.5		0.6452	0.3715	0.2176
5.0		0.6348	0.3516	0.2016

TABLE 11.d. NATURAL FREQUENCY RATIO.

$\hat{\Omega}^2$  MAXIMUM

m	n	$\frac{a}{b}$	$(h^2/ab=0.03)$					
			0.0	0.01	0.25	0.50	0.75	0.99
1	1	1.0	0.8553	0.8540	0.8159	0.7472	0.5964	0.0558
2	1	2.0	0.7472	0.7453	0.6891	0.5964	0.4249	0.0287
3	1	3.0	0.6633	0.6611	0.5964	0.4962	0.3300	0.0193
1	2	0.5	0.7472	0.7453	0.6891	0.5964	0.4249	0.0287
2	2	1.0	0.5964	0.5940	0.5257	0.4249	0.2698	0.0146
3	2	1.5	0.4962	0.4937	0.4249	0.4055	0.1976	0.0098
1	3	0.33	0.6633	0.6611	0.5964	0.4962	0.3300	0.0193
2	3	0.67	0.4962	0.4937	0.4249	0.3300	0.1976	0.0098
3	3	1.0	0.3964	0.3940	0.3300	0.2472	0.1410	0.0065

TABLE 11e. SUMMARY SHEET FOR NATURAL FREQUENCY SQUARE RATIO.

$\frac{a/b}{\beta}$	$\hat{\Omega}^2$					$(h^2/ab=0.04)$
	0.01	0.25	0.50	0.75	0.99	
	(m=1, n=1)					
0.25	0.6738	0.6101	0.5105	0.3428	0.0204	
0.5	0.7783	0.7268	0.6394	0.4699	0.0342	
1.0	0.8144	0.7688	0.6891	0.5257	0.0424	
1.5	0.8020	0.7542	0.6717	0.5057	0.0393	
2.0	0.7783	0.7268	0.6394	0.4699	0.0342	
2.5	0.7516	0.6963	0.6045	0.4332	0.0297	
3.0	0.7248	0.6661	0.5708	0.3994	0.0259	
3.5	0.6987	0.6372	0.5394	0.3693	0.0229	
4.0	0.6738	0.6101	0.5105	0.3428	0.0204	
4.5	0.6502	0.5847	0.4842	0.3194	0.0184	
5.0	0.6280	0.5612	0.4602	0.2989	0.0168	
	(m=2, n=1)					
0.25	0.3507	0.2904	0.2143	0.1200	0.0054	
0.5	0.5080	0.4389	0.3428	0.2068	0.0103	
1.0	0.6371	0.5708	0.4699	0.3071	0.0174	
1.5	0.6781	0.6148	0.5155	0.3472	0.0208	
2.0	0.6869	0.6244	0.5257	0.3565	0.0217	
2.5	0.6816	0.6186	0.5195	0.3509	0.0212	
3.0	0.6695	0.6054	0.5057	0.3384	0.0200	
3.5	0.6540	0.5888	0.4884	0.3231	0.0187	
4.0	0.6371	0.5708	0.4699	0.3071	0.0174	
4.5	0.6196	0.5523	0.4513	0.2914	0.0162	
5.0	0.6021	0.5341	0.4332	0.2765	0.0151	
	(m=3, n=1)					
0.25	0.1949	0.1550	0.1090	0.0576	0.0024	
0.5	0.3218	0.2644	0.1933	0.1070	0.0048	
1.0	0.4674	0.3994	0.3071	0.1814	0.0088	
1.5	0.5392	0.4699	0.3715	0.2281	0.0117	
2.0	0.5745	0.5057	0.4055	0.2543	0.0135	
2.5	0.5900	0.5215	0.4209	0.2665	0.0143	
3.0	0.5940	0.5257	0.4249	0.2698	0.0146	
3.5	0.5911	0.5227	0.4220	0.2674	0.0144	
4.0	0.5841	0.5155	0.4149	0.2618	0.0140	
4.5	0.5745	0.5057	0.4055	0.2543	0.0135	
5.0	0.5635	0.4944	0.3946	0.2458	0.0129	

TABLE 12a. NATURAL FREQUENCY RATIO.

$\frac{\hat{\alpha}}{b}$	0.01	0.25	$\hat{\Omega}^2$ 0.50	0.75	$(h^2/ab=0.04)$ 0.99
(m=1, n=2)					
0.25	0.6371	0.5708	0.4699	0.3071	0.0174
0.5	0.6869	0.6244	0.5257	0.3565	0.0217
1.0	0.6371	0.5708	0.4699	0.3071	0.0174
1.5	0.5683	0.4993	0.3994	0.2495	0.0131
2.0	0.5080	0.4389	0.3428	0.2068	0.0103
2.5	0.4577	0.3900	0.2989	0.1757	0.0085
3.0	0.4158	0.3503	0.2644	0.1523	0.0071
3.5	0.3806	0.3176	0.2368	0.1343	0.0062
4.0	0.3507	0.2904	0.2143	0.1200	0.0054
4.5	0.3251	0.2673	0.1957	0.1084	0.0048
5.0	0.3029	0.2477	0.1800	0.0989	0.0044
(m=2, n=2)					
0.25	0.3405	0.2812	0.2068	0.1153	0.0052
0.5	0.4674	0.3994	0.3071	0.1814	0.0088
1.0	0.5232	0.4539	0.3565	0.2169	0.0110
1.5	0.5032	0.4341	0.3384	0.2037	0.0101
2.0	0.4674	0.3994	0.3071	0.1814	0.0088
2.5	0.4307	0.3644	0.2765	0.1604	0.0076
3.0	0.3970	0.3328	0.2495	0.1425	0.0066
3.5	0.3669	0.3051	0.2264	0.1277	0.0058
4.0	0.3405	0.2812	0.2068	0.1153	0.0052
4.5	0.3173	0.2604	0.1901	0.1050	0.0047
5.0	0.2968	0.2422	0.1757	0.0963	0.0042
(m=3, n=2)					
0.25	0.1917	0.1523	0.1070	0.0565	0.0024
0.5	0.3050	0.2495	0.1814	0.0998	0.0044
1.0	0.4030	0.3384	0.2543	0.1457	0.0068
1.5	0.4224	0.3565	0.2698	0.1559	0.0073
2.0	0.4125	0.3472	0.2618	0.1506	0.0070
2.5	0.3922	0.3284	0.2458	0.1401	0.0065
3.0	0.3691	0.3071	0.2281	0.1287	0.0059
3.5	0.3463	0.2863	0.2110	0.1180	0.0053
4.0	0.3247	0.2670	0.1954	0.1083	0.0048
4.5	0.3050	0.2495	0.1814	0.0998	0.0044
5.0	0.2870	0.2337	0.1690	0.0923	0.0041

TABLE 12b. NATURAL FREQUENCY RATIO.

$\frac{a/b}{\hat{\beta}}$	0.01 (m=1, n=3)	0.25	$\hat{\Omega}^2$ 0.50	0.75	$(h^2/ab=0.04)$ 0.99
0.25	0.5841	0.5155	0.4149	0.2618	0.0140
0.5	0.5745	0.5057	0.4055	0.2543	0.0135
1.0	0.4674	0.3994	0.3071	0.1814	0.0088
1.5	0.3825	0.3194	0.2383	0.1353	0.0062
2.0	0.3218	0.2644	0.1933	0.1070	0.0048
2.5	0.2771	0.2250	0.1622	0.0882	0.0039
3.0	0.2431	0.1957	0.1395	0.0750	0.0032
3.5	0.2164	0.1720	0.1224	0.0652	0.0028
4.0	0.1949	0.1550	0.1090	0.0576	0.0024
4.5	0.1773	0.1404	0.0982	0.0516	0.0022
5.0	0.1626	0.1282	0.0893	0.0467	0.0020
(m=2, n=3)					
0.25	0.3247	0.2670	0.1954	0.1083	0.0048
0.5	0.4125	0.3472	0.2618	0.1506	0.0070
1.0	0.4030	0.3384	0.2543	0.1457	0.0068
1.5	0.3519	0.2914	0.2152	0.1206	0.0055
2.0	0.3050	0.2495	0.1814	0.0998	0.0044
2.5	0.2670	0.2162	0.1554	0.0842	0.0037
3.0	0.2365	0.1901	0.1353	0.0726	0.0031
3.5	0.2119	0.1692	0.1196	0.0636	0.0027
4.0	0.1917	0.1523	0.1070	0.0565	0.0024
4.5	0.1750	0.1384	0.0967	0.0508	0.0021
5.0	0.1608	0.1268	0.0882	0.0462	0.0019
(m=3, n=3)					
0.25	0.1866	0.1481	0.1039	0.0548	0.0023
0.5	0.2806	0.2281	0.1646	0.0897	0.0039
1.0	0.3278	0.2698	0.1976	0.1096	0.0049
1.5	0.3104	0.2543	0.1852	0.1021	0.0045
2.0	0.2806	0.2281	0.1646	0.0897	0.0039
2.5	0.2517	0.2030	0.1452	0.0783	0.0034
3.0	0.2263	0.1814	0.1287	0.0688	0.0029
3.5	0.2048	0.1633	0.1151	0.0611	0.0026
4.0	0.1866	0.1481	0.1039	0.0548	0.0023
4.5	0.1712	0.1353	0.0945	0.0496	0.0021
5.0	0.1579	0.1244	0.0865	0.0452	0.0019

TABLE 12c. NATURAL FREQUENCY RATIO.



		$\hat{\Omega}^2$ ( $h^2/ab=0.04, \hat{\beta}=0$ )		
		1	2	3
$b/a$ / $n$	(m=1)			
	0.25	0.6760	0.6394	0.5865
	0.5	0.7800	0.6891	0.5770
	1.0	0.8159	0.6394	0.4699
	1.5	0.8036	0.5708	0.3849
	2.0	0.7800	0.5105	0.3240
	2.5	0.7535	0.4602	0.2791
	3.0	0.7268	0.4182	0.2449
	3.5	0.7008	0.3829	0.2181
	4.0	0.6760	0.3530	0.1965
	4.5	0.6525	0.3273	0.1788
5.0	0.6303	0.3050	0.1640	
	(m=2)			
0.25	0.3530	0.3428	0.3270	
0.5	0.5105	0.4699	0.4149	
1.0	0.6394	0.5257	0.4055	
1.5	0.6803	0.5057	0.3542	
2.0	0.6891	0.4699	0.3071	
2.5	0.6838	0.4332	0.2689	
3.0	0.6717	0.3994	0.2383	
3.5	0.6563	0.3693	0.2136	
4.0	0.6394	0.3428	0.1933	
4.5	0.6220	0.3194	0.1764	
5.0	0.6045	0.2989	0.1622	
	(m=3)			
0.25	0.1965	0.1933	0.1882	
0.5	0.3240	0.3071	0.2827	
1.0	0.4699	0.4055	0.3300	
1.5	0.5417	0.4249	0.3125	
2.0	0.5770	0.4149	0.2827	
2.5	0.5924	0.3946	0.2536	
3.0	0.5964	0.3715	0.2281	
3.5	0.5935	0.3485	0.2065	
4.0	0.5865	0.3270	0.1882	
4.5	0.5770	0.3071	0.1726	
5.0	0.5650	0.2891	0.1593	

TABLE 12d. NATURAL FREQUENCY RATIO.

$\hat{\Omega}^2$  MAXIMUM

m	n	$\frac{a}{b}$	$\frac{B}{A}$	$(h^2/ab=0.04)$					
				0.0	0.01	0.25	0.50	0.75	0.99
1	1	1.0	1.0	0.8159	0.8144	0.7688	0.6891	0.5257	0.0424
2	1	2.0	1.0	0.6891	0.6869	0.6244	0.5257	0.3565	0.0217
3	1	3.0	1.0	0.5964	0.5940	0.5257	0.4249	0.2698	0.0146
1	2	0.5	1.0	0.6891	0.6869	0.6244	0.5257	0.3565	0.0217
2	2	1.0	1.0	0.5257	0.5232	0.4539	0.3565	0.2169	0.0110
3	2	1.5	1.0	0.4249	0.4224	0.3565	0.2698	0.1559	0.0073
1	3	0.33	1.0	0.5964	0.5940	0.5257	0.4249	0.2698	0.0146
2	3	0.67	1.0	0.4249	0.4224	0.3565	0.2698	0.1559	0.0073
3	3	1.0	1.0	0.3300	0.3278	0.2698	0.1976	0.1096	0.0049

TABLE 12e. SUMMARY SHEET FOR NATURAL FREQUENCY SQUARE RATIO.

$\frac{\hat{\alpha}}{\hat{\beta}}$	0.01	0.25	$\hat{\Omega}^2$ 0.50	0.75	$(h^2/ab=0.05)$ 0.99
(m=1, n=1)					
0.25	0.6229	0.5559	0.4549	0.2944	0.0164
0.5	0.7374	0.6803	0.5865	0.4149	0.0276
1.0	0.7783	0.7268	0.6394	0.4699	0.0342
1.5	0.7642	0.7106	0.6207	0.4501	0.0317
2.0	0.7374	0.6803	0.5865	0.4149	0.0276
2.5	0.7077	0.6472	0.5501	0.3794	0.0239
3.0	0.6781	0.6148	0.5155	0.3472	0.0208
3.5	0.6497	0.5842	0.4837	0.3190	0.0193
4.0	0.6229	0.5559	0.4549	0.2944	0.0164
4.5	0.5979	0.5297	0.4289	0.2730	0.0148
5.0	0.6745	0.5057	0.4055	0.2543	0.0135
(m=2, n=1)					
0.25	0.3017	0.2466	0.1791	0.0984	0.0043
0.5	0.4524	0.3849	0.2944	0.1726	0.0083
1.0	0.5841	0.5155	0.4149	0.2618	0.0140
1.5	0.6276	0.5608	0.4598	0.2985	0.0167
2.0	0.6371	0.5708	0.4699	0.3071	0.0174
2.5	0.6313	0.5657	0.4675	0.3019	0.0170
3.0	0.6184	0.5511	0.4501	0.2904	0.0157
3.5	0.6020	0.5340	0.4330	0.2764	0.0150
4.0	0.5841	0.5155	0.4149	0.2618	0.0140
4.5	0.5658	0.4968	0.3969	0.2476	0.0130
5.0	0.5476	0.4784	0.3794	0.2341	0.0121
(m=3, n=1)					
0.25	0.1623	0.1280	0.0891	0.0466	0.0020
0.5	0.2751	0.2233	0.1609	0.0875	0.0038
1.0	0.4125	0.3472	0.2618	0.1506	0.0070
1.5	0.4835	0.4149	0.3210	0.1912	0.0094
2.0	0.5193	0.4501	0.3530	0.2143	0.0108
2.5	0.5378	0.4658	0.3676	0.2252	0.0115
3.0	0.5392	0.4699	0.3715	0.2281	0.0117
3.5	0.5363	0.4670	0.3687	0.2260	0.0115
4.0	0.5291	0.4561	0.3620	0.2210	0.0112
4.5	0.5193	0.4501	0.3537	0.2143	0.0108
5.0	0.5080	0.4389	0.3428	0.2068	0.0103

TABLE 13a. NATURAL FREQUENCY RATIO.

$\frac{\hat{\beta}}{a/b}$	$\hat{\Omega}^2$					$(h^2/ab=0.05)$ 0.99
	0.01	0.25	0.50	0.75	0.75	
	(m=1, n=2)					
0.25	0.5841	0.5155	0.4149	0.2618	0.0140	
0.5	0.6371	0.5708	0.4699	0.3017	0.0174	
1.0	0.5841	0.5155	0.4149	0.2618	0.0140	
1.5	0.5130	0.4438	0.3472	0.2101	0.0105	
2.0	0.4524	0.3849	0.2944	0.1726	0.0083	
2.5	0.4030	0.3383	0.2543	0.1457	0.0068	
3.0	0.3628	0.3013	0.2233	0.1257	0.0057	
3.5	0.3295	0.2713	0.1989	0.1104	0.0049	
4.0	0.3017	0.2466	0.1791	0.0984	0.0043	
4.5	0.2782	0.2260	0.1629	0.0887	0.0039	
5.0	0.2579	0.2084	0.1493	0.0807	0.0035	
	(m=2, n=2)					
0.25	0.2923	0.2383	0.1726	0.0944	0.0042	
0.5	0.4125	0.3472	0.2618	0.1506	0.0070	
1.0	0.4674	0.2994	0.3071	0.1814	0.0088	
1.5	0.4476	0.3802	0.2904	0.1698	0.0081	
2.0	0.4125	0.3472	0.2618	0.1506	0.0070	
2.5	0.3771	0.3144	0.2341	0.1326	0.0061	
3.0	0.3450	0.2852	0.2101	0.1174	0.0053	
3.5	0.3168	0.2600	0.1897	0.1048	0.0047	
4.0	0.2923	0.2383	0.1726	0.0945	0.0042	
4.5	0.2710	0.2130	0.1581	0.0891	0.0037	
5.0	0.2524	0.2037	0.1457	0.0786	0.0034	
	(m=3, n=2)					
0.25	0.1595	0.0756	0.0875	0.0457	0.0019	
0.5	0.2598	0.1353	0.1506	0.0814	0.0035	
1.0	0.3507	0.2101	0.2143	0.1200	0.0054	
1.5	0.3691	0.2419	0.2281	0.1287	0.0059	
2.0	0.3597	0.2487	0.2210	0.1242	0.0056	
2.5	0.3405	0.2449	0.2068	0.1153	0.0052	
3.0	0.3188	0.2348	0.1912	0.1020	0.0047	
3.5	0.2976	0.2227	0.1763	0.0967	0.0043	
4.0	0.2778	0.2101	0.1627	0.0886	0.0039	
4.5	0.2599	0.1979	0.1506	0.0814	0.0035	
5.0	0.2436	0.1865	0.1399	0.0752	0.0032	

TABLE 13b. NATURAL FREQUENCY RATIO.

$\frac{a}{b} \backslash \hat{\beta}$	$\hat{\Omega}^2$					$(h^2/ab=0.05)$ $\hat{\beta} = 0.99$
	0.01 (m=1, n=3)	0.25	0.50	0.75		
0.25	0.5291	0.4598	0.3620	0.2210	0.0112	
0.5	0.5193	0.4501	0.3520	0.2143	0.0108	
1.0	0.4125	0.3472	0.2618	0.1506	0.0070	
1.5	0.3314	0.2730	0.2002	0.1112	0.0050	
2.0	0.2751	0.2233	0.1609	0.0875	0.0038	
2.5	0.2347	0.1885	0.1341	0.0719	0.0031	
3.0	0.2044	0.1629	0.1148	0.0649	0.0026	
3.5	0.1809	0.1434	0.1004	0.0528	0.0022	
4.0	0.1623	0.1280	0.0891	0.0466	0.0020	
4.5	0.1471	0.1155	0.0801	0.0417	0.0017	
5.0	0.1345	0.1053	0.0727	0.0377	0.0016	
(m=2, n=3)						
0.25	0.2778	0.2257	0.1627	0.0886	0.0039	
0.5	0.3597	0.2985	0.2210	0.1242	0.0056	
1.0	0.3507	0.2904	0.2143	0.1200	0.0054	
1.5	0.3028	0.2476	0.1799	0.0988	0.0044	
2.0	0.2599	0.2101	0.1506	0.0814	0.0035	
2.5	0.2256	0.1808	0.1283	0.0685	0.0029	
3.0	0.1986	0.1581	0.112	0.0589	0.0025	
3.5	0.1770	0.1401	0.0980	0.0515	0.0022	
4.0	0.1595	0.1257	0.0875	0.0457	0.0019	
4.5	0.1450	0.1139	0.0759	0.0411	0.0017	
5.0	0.1329	0.1041	0.0719	0.0373	0.0015	
(m=3, n=3)						
0.25	0.1551	0.1221	0.0848	0.0443	0.0019	
0.5	0.2378	0.1912	0.1362	0.0730	0.0031	
1.0	0.2806	0.2281	0.1646	0.0897	0.0039	
1.5	0.2647	0.2143	0.1539	0.0834	0.0036	
2.0	0.2378	0.1912	0.1362	0.0730	0.0031	
2.5	0.2120	0.1693	0.1196	0.0636	0.0027	
3.0	0.1897	0.1577	0.1057	0.0558	0.0024	
3.5	0.1709	0.1350	0.0943	0.0495	0.0021	
4.0	0.1551	0.1221	0.0848	0.0443	0.0019	
4.5	0.1418	0.1112	0.0770	0.0400	0.0017	
5.0	0.1305	0.1021	0.0704	0.0365	0.0015	

TABLE 13c. NATURAL FREQUENCY RATIO.

TABLE 13d. NATURAL FREQUENCY RATIO.

$\frac{a}{b} \backslash n$ (m=1)	$\hat{\Omega}^2$ ( $h^2/ab=0.05, \hat{\beta}=0$ )		
	1	2	3
0.25	0.6253	0.5865	0.5316
0.5	0.7394	0.6394	0.5218
1.0	0.7800	0.5865	0.4149
1.5	0.7660	0.5155	0.3336
2.0	0.7394	0.4549	0.2771
2.5	0.7098	0.4055	0.2365
3.0	0.6803	0.3651	0.2060
3.5	0.6520	0.3318	0.1824
4.0	0.6523	0.3038	0.1636
4.5	0.6003	0.2802	0.1483
5.0	0.5770	0.2599	0.1356
(m=2)			
0.25	0.3038	0.2944	0.2799
0.5	0.4549	0.4149	0.3620
1.0	0.5865	0.4699	0.3520
1.5	0.6299	0.4501	0.3049
2.0	0.6394	0.4149	0.2618
2.5	0.6337	0.3794	0.2274
3.0	0.6207	0.3472	0.2002
3.5	0.6044	0.3190	0.1785
4.0	0.5865	0.2944	0.1609
4.5	0.5682	0.2730	0.1463
5.0	0.5501	0.2543	0.1341
(m=3)			
0.25	0.1636	0.1609	0.1564
0.5	0.2771	0.2618	0.2397
1.0	0.4149	0.3530	0.2827
1.5	0.4860	0.3715	0.2667
2.0	0.5218	0.3620	0.2397
2.5	0.5376	0.3428	0.2137
3.0	0.5417	0.3210	0.1912
3.5	0.5388	0.2997	0.1723
4.0	0.5316	0.2799	0.1564
4.5	0.5218	0.2618	0.1430
5.0	0.5105	0.2455	0.1316

TABLE 13d. NATURAL FREQUENCY RATIO.

m	n	$\frac{a}{b}$	$\Omega^2$ MAXIMUM					
			( $h^2/ab=0.05$ ) 0.0	0.01	0.25	0.50	0.75	0.99
1	1	1.0	0.7800	0.7783	0.7268	0.6394	0.4699	0.0342
2	1	2.0	0.6394	0.6371	0.5708	0.4699	0.3071	0.0174
3	1	3.0	0.5417	0.5392	0.4699	0.3715	0.2281	0.0117
1	2	0.5	0.6394	0.6371	0.5708	0.4699	0.3071	0.0174
2	2	1.0	0.4699	0.4674	0.3994	0.3071	0.1814	0.0088
3	2	1.5	0.3715	0.3691	0.3071	0.2281	0.1287	0.0059
1	3	0.33	0.5417	0.5392	0.4699	0.3715	0.2281	0.0117
2	3	0.67	0.3715	0.3691	0.3071	0.2281	0.1287	0.0059
3	3	1.0	0.2827	0.2806	0.2281	0.1646	0.0897	0.0039

TABLE 13e. SUMMARY SHEET FOR NATURAL FREQUENCY SQUARE RATIO.

$\frac{c}{b} \hat{\beta}$	$\hat{\Omega}^2$ (B.S.A. TI. RI)					$(h^2/ab=0.01)$
	0.01	0.25	0.50	0.75	0.99	
(m=1, n=1)						
0.25	15.535	11.769	7.846	3.923	0.157	
0.5	5.678	4.302	2.868	1.434	0.057	
1.0	3.694	2.699	1.866	0.933	0.037	
1.5	4.312	3.266	2.178	1.089	0.044	
2.0	5.678	4.302	2.868	1.434	0.057	
2.5	7.543	5.714	3.810	1.905	0.076	
3.0	9.829	7.447	4.965	2.482	0.099	
3.5	12.501	9.471	6.314	3.157	0.126	
4.0	15.534	11.769	7.846	3.923	0.157	
4.5	18.908	14.325	9.550	4.775	0.191	
5.0	22.608	17.128	11.419	5.710	0.228	
(m=2, n=1)						
0.25	168.404	127.630	85.122	42.578	1.704	
0.5	55.165	41.796	27.867	13.935	0.558	
1.0	21.026	15.929	10.620	5.310	0.212	
1.5	14.969	11.340	7.560	3.780	0.151	
2.0	13.866	10.504	7.003	3.502	0.140	
2.5	14.523	11.003	7.335	3.668	0.147	
3.0	16.109	12.204	8.136	4.068	0.163	
3.5	18.322	13.880	9.254	4.627	0.185	
4.0	21.025	15.929	10.620	5.310	0.212	
4.5	24.147	18.294	12.197	6.099	0.244	
5.0	27.644	20.943	13.963	6.982	0.279	
(m=3, n=1)						
0.25	596.646	452.578	302.104	151.239	6.057	
0.5	208.523	158.051	105.422	52.738	2.111	
1.0	73.499	55.690	37.133	18.569	0.743	
1.5	44.099	33.441	22.276	11.139	0.446	
2.0	34.042	25.791	17.195	8.598	0.344	
2.5	30.319	22.970	15.314	7.657	0.306	
3.0	29.416	22.286	14.858	7.429	0.297	
3.5	30.059	22.774	15.183	7.592	0.304	
4.0	31.694	24.012	16.009	8.005	0.320	
4.5	34.041	25.790	17.195	8.598	0.344	
5.0	36.948	27.993	18.663	9.332	0.373	

TABLE 14a. LOWER SET OF NATURAL FREQUENCY INCLUDING SHEAR AND ROTARY INERTIA.



$\frac{a}{b} \hat{\beta}$	$\hat{\Omega}^2$ (B.S.A. TI. RI)					$(h^2/ab=0.01)$ 0.99
	0.01	0.25	0.50	0.75		
(m=1, n=2)						
0.25	21.026	15.929	10.620	5.310	0.212	
0.5	13.855	10.504	7.003	3.502	0.140	
1.0	21.025	15.929	10.620	5.310	0.212	
1.5	35.644	27.005	18.004	9.003	0.360	
2.0	55.164	41.796	27.867	13.935	0.558	
2.5	78.716	59.643	39.769	19.888	0.796	
3.0	105.721	80.111	53.421	26.717	1.069	
3.5	135.736	102.863	68.598	34.310	1.373	
4.0	168.401	127.628	85.121	42.578	1.704	
4.5	203.421	154.182	102.840	51.445	2.059	
5.0	240.548	182.339	121.631	60.851	2.435	
(m=2, n=2)						
0.25	181.473	137.539	91.734	45.887	1.836	
0.5	73.499	55.690	37.133	18.569	0.743	
1.0	49.510	37.511	25.010	12.506	0.500	
1.5	57.102	43.264	28.846	14.425	0.577	
2.0	73.449	55.690	37.132	18.569	0.743	
2.5	95.141	72.092	48.072	24.041	0.962	
3.0	120.772	91.519	61.030	30.524	1.221	
3.5	149.703	113.451	75.662	37.845	1.514	
4.0	181.470	137.537	91.732	45.886	1.836	
4.5	215.722	163.510	109.065	54.561	2.184	
5.0	252.178	191.160	127.519	63.798	2.553	
(m=3, n=2)						
0.25	615.106	466.593	311.468	155.931	6.245	
0.5	236.712	179.430	119.690	59.879	2.396	
1.0	115.679	87.659	58.455	29.236	1.170	
1.5	100.864	76.430	50.965	25.488	1.020	
2.0	108.192	81.976	54.665	27.339	1.094	
2.5	124.902	94.650	63.119	31.569	1.263	
3.0	147.344	111.663	74.469	37.248	1.490	
3.5	173.965	131.846	87.935	43.986	1.760	
4.0	203.931	154.569	103.098	51.574	2.064	
4.5	236.709	179.427	119.688	59.878	2.396	
5.0	271.919	206.134	137.514	68.802	2.754	

TABLE 14b. LOWER SET OF NATURAL FREQUENCY INCLUDING SHEAR AND ROTARY INERTIA.

$\frac{\alpha}{b} \hat{\beta}$	$\hat{\Omega}^2$ (B.S.A. TI. RI)					$(h^2/ab=0.01)$
	0.01	0.25	0.50	0.75	0.99	
(m=1, n=3)						
0.25	31.694	24.012	16.009	8.005	0.320	
0.5	34.042	25.790	17.195	8.598	0.344	
1.0	73.499	55.690	37.133	18.569	0.743	
1.5	133.834	101.421	67.636	33.829	1.354	
2.0	208.521	158.049	105.420	52.737	2.111	
2.5	294.127	222.980	148.760	73.432	2.979	
3.0	388.368	294.482	196.501	98.338	3.937	
3.5	489.607	371.319	247.819	124.043	4.967	
4.0	596.637	452.570	302.098	151.237	6.056	
4.5	708.526	537.530	358.868	179.683	7.197	
5.0	824.553	625.647	417.757	209.196	8.380	
(m=2, n=3)						
0.25	203.933	154.571	103.099	51.575	2.064	
0.5	108.182	81.977	54.665	27.339	1.094	
1.0	115.679	87.659	58.455	29.236	1.170	
1.5	166.971	126.543	84.397	42.216	1.689	
2.0	236.710	179.428	119.688	59.878	2.396	
2.5	318.914	241.784	161.314	80.718	3.231	
3.0	410.575	311.335	207.755	103.975	4.163	
3.5	509.759	386.615	258.036	129.161	5.172	
4.0	615.095	466.585	311.462	155.928	6.245	
4.5	725.562	550.466	367.513	184.015	7.370	
5.0	840.373	637.663	425.788	213.221	8.541	
(m=3, n=3)						
0.25	646.152	490.166	327.219	163.823	6.561	
0.5	285.998	216.813	144.643	72.371	2.897	
1.0	199.375	151.114	100.792	50.421	2.018	
1.5	227.195	172.211	114.872	57.467	2.300	
2.0	285.996	216.812	144.642	72.370	2.897	
2.5	361.439	274.049	182.856	91.505	3.663	
3.0	448.289	339.957	226.872	113.550	4.546	
3.5	543.776	412.439	275.288	137.804	5.518	
4.0	646.141	490.157	327.213	163.820	6.561	
4.5	754.147	572.174	382.021	191.286	7.662	
5.0	866.874	657.792	439.241	219.964	8.811	

TABLE 14c. LOWER SET OF NATURAL FREQUENCY INCLUDING SHEAR AND ROTARY INERTIA.

$\frac{b^2 a}{m}$	$\hat{\Omega}^2$ (B.S.A. TI.)	$(h^2/ab=0.01), (\hat{\beta}=0)$		
		1	2	3
	(m=1)			
0.25	15.691	21.238	32.014	
0.5	5.736	14.006	34.385	
1.0	3.732	21.238	74.241	
1.5	4.355	36.004	135.184	
2.0	5.736	55.721	210.623	
2.5	7.619	79.510	297.090	
3.0	9.929	106.788	392.275	
3.5	12.628	137.105	494.530	
4.0	15.691	170.099	602.631	
4.5	19.099	205.472	715.639	
5.0	22.836	242.972	832.825	
	(m=2)			
0.25	170.102	183.303	205.989	
0.5	55.722	74.241	109.274	
1.0	21.238	50.010	116.856	
1.5	15.219	57.678	168.655	
2.0	14.006	74.241	239.096	
2.5	14.670	96.102	322.125	
3.0	16.272	121.990	414.706	
3.5	18.507	151.213	514.884	
4.0	21.238	183.300	621.274	
4.5	24.391	217.896	732.845	
5.0	27.923	254.719	848.804	
	(m=3)			
0.25	602.641	621.285	652.642	
0.5	210.625	239.098	288.879	
1.0	74.241	116.846	201.385	
1.5	44.544	101.882	229.484	
2.0	34.386	109.273	288.877	
2.5	30.625	126.162	365.077	
3.0	29.723	148.830	452.798	
3.5	30.363	175.718	549.242	
4.0	32.014	205.986	652.631	
4.5	34.385	239.094	761.716	
5.0	37.321	274.658	875.569	

TABLE 14d. LOWER SET OF NATURAL FREQUENCY INCLUDING SHEAR AND ROTARY INERTIA.

$\hat{\Omega}^2$  (B.S.A. TI. RI) MINIMUM

m	n	$\frac{a}{b}$	$(h^2/ab=0.01)$					
			0.0	0.01	0.25	0.50	0.75	0.99
1	1	1.0	3.732	3.694	2.799	1.866	0.933	0.037
2	1	2.0	14.006	13.866	10.504	7.003	3.502	0.140
3	1	3.0	29.723	29.416	22.286	14.858	7.429	0.297
1	2	0.50	14.006	13.866	10.504	7.003	3.502	0.140
2	2	1.0	50.010	49.510	37.511	25.010	12.506	0.500
3	2	1.5	101.882	100.864	76.430	50.965	25.488	1.020
1	3	0.33	29.723	29.416	22.286	14.858	7.429	0.297
2	3	0.67	101.882	100.864	76.430	50.965	25.488	1.020
3	3	1.0	201.385	199.375	151.114	100.792	50.421	2.018

TABLE 14e. LOWER SET OF NATURAL FREQUENCY INCLUDING SHEAR AND ROTARY INERTIA.

$\frac{\hat{\alpha}}{a/b}$	$\hat{\Omega}^2$ (B.S.A. TI. RI)				
	0.01	0.25	0.50	0.75	( $h^2/ab=0.02$ ) 0.99
(m=1, n=1)					
0.25	13.791	10.449	6.967	3.484	0.139
0.5	5.256	3.982	2.655	1.328	0.053
1.0	3.466	2.626	1.751	0.875	0.035
1.5	4.027	3.051	2.034	1.017	0.041
2.0	5.256	3.982	2.655	1.328	0.053
2.5	6.911	5.236	3.491	1.745	0.070
3.0	8.911	6.751	4.501	2.251	0.090
3.5	11.214	8.496	5.665	2.833	0.113
4.0	13.791	10.449	6.967	3.484	0.139
4.5	16.619	12.592	8.396	4.199	0.168
5.0	19.679	14.911	9.942	4.972	0.199
(m=2, n=1)					
0.25	126.599	96.015	64.083	32.077	1.284
0.5	45.368	34.385	22.934	11.472	0.459
1.0	18.375	13.923	9.283	4.642	0.186
1.5	13.313	10.087	6.725	3.363	0.135
2.0	12.378	9.378	6.253	3.127	0.125
2.5	12.936	9.801	6.535	3.268	0.131
3.0	14.275	10.816	7.212	3.606	0.144
3.5	16.130	12.221	8.149	4.075	0.163
4.0	18.375	13.992	9.283	4.642	0.186
4.5	20.941	15.867	10.580	5.291	0.212
5.0	23.785	18.023	12.018	6.010	0.240
(m=3, n=1)					
0.25	398.176	302.312	201.977	101.194	4.055
0.5	153.776	116.648	77.867	38.983	1.561
1.0	59.178	44.858	29.923	14.970	0.599
1.5	36.837	27.916	18.618	9.312	0.373
2.0	28.920	21.915	14.614	7.309	0.292
2.5	25.943	19.658	13.109	6.556	0.262
3.0	25.216	19.108	12.741	6.372	0.255
3.5	25.734	19.500	13.003	6.503	0.260
4.0	27.045	20.494	13.666	6.835	0.273
4.5	28.920	21.915	14.614	7.309	0.292
5.0	31.225	23.663	15.780	7.892	0.316

TABLE 15a. LOWER SET OF NATURAL FREQUENCY INCLUDING SHEAR AND ROTARY INERTIA.

$\frac{\alpha/\beta}{\hat{\beta}}$	$\hat{\Omega}^2$ (B.S.A. TI. RI)					$(h^2/ab=0.02)$ 0.99
	0.01 (m=1, n=2)	0.25	0.50	0.75		
0.25	18.375	13.923	9.283	4.642	0.186	
0.5	12.378	9.378	6.253	3.127	0.125	
1.0	18.375	13.923	9.283	4.642	0.186	
1.5	30.193	22.880	15.258	7.631	0.305	
2.0	45.368	34.384	22.933	11.472	0.459	
2.5	63.045	47.790	31.880	15.590	0.638	
3.0	82.703	62.703	41.835	20.934	0.838	
3.5	103.978	78.846	52.615	26.332	1.054	
4.0	126.596	96.014	64.082	32.076	1.284	
4.5	150.352	114.048	76.130	38.112	1.526	
5.0	175.081	132.825	88.676	44.399	1.778	
 (m=2, n=2)						
0.25	135.518	102.786	68.606	34.343	1.373	
0.5	59.178	44.858	29.923	14.970	0.599	
1.0	41.030	31.095	20.739	10.373	0.415	
1.5	46.845	35.505	23.681	11.846	0.474	
2.0	59.178	44.857	29.922	14.970	0.599	
2.5	75.068	56.910	37.968	18.998	0.760	
3.0	93.437	70.847	47.273	23.657	0.947	
3.5	113.710	86.232	57.548	28.803	1.153	
4.0	135.516	102.784	68.605	34.342	1.375	
4.5	158.593	120.305	80.310	40.207	1.610	
5.0	182.744	138.644	92.565	46.348	1.856	
 (m=3, n=2)						
0.25	409.283	310.752	207.620	104.023	4.169	
0.5	172.546	130.900	87.390	43.755	1.752	
1.0	89.821	68.103	45.441	22.740	0.910	
1.5	79.208	60.051	40.065	20.047	0.803	
2.0	84.468	64.042	42.729	21.382	0.856	
2.5	96.359	73.064	48.754	24.399	0.977	
3.0	112.073	84.990	56.718	28.388	1.137	
3.5	130.402	98.902	66.011	33.043	1.323	
4.0	150.694	114.308	76.303	38.199	1.530	
4.5	172.543	130.898	87.389	43.754	1.752	
5.0	195.668	148.450	99.125	49.645	1.988	

TABLE 15b. LOWER SET OF NATURAL FREQUENCY INCLUDING SHEAR AND ROTARY INERTIA.

$\frac{a}{b} \hat{\beta}$	$\hat{\Omega}^2$ (B.S.A. RI. TI)				
	0.01 (m=1, n=3)	0.25	0.50	0.75	(h <sup>2</sup> /ab=0.02) 0.99
0.25	27.046	20.494	13.666	6.835	0.273
0.5	28.920	21.915	14.614	7.309	0.292
1.0	59.178	44.857	29.922	14.970	0.599
1.5	102.645	77.834	51.939	25.994	1.041
2.0	153.774	116.647	77.866	38.982	1.561
2.5	210.094	159.416	106.447	53.305	2.135
3.0	270.165	205.048	136.949	68.595	2.748
3.5	333.066	252.838	168.899	84.611	3.390
4.0	398.170	302.307	201.974	101.193	4.055
4.5	465.029	353.113	235.942	118.222	4.738
5.0	533.316	405.002	270.635	135.614	5.435
(m=2, n=3)					
0.25	150.696	114.309	76.304	38.180	1.530
0.5	84.468	64.042	42.729	21.382	0.856
1.0	89.821	68.103	45.441	22.740	0.910
1.5	125.616	95.269	63.585	31.827	1.274
2.0	172.544	130.899	87.389	43.754	1.752
2.5	226.062	171.545	114.554	57.369	2.298
3.0	284.091	215.628	144.022	72.140	2.890
3.5	345.422	262.227	175.176	87.758	3.517
4.0	409.277	310.748	207.617	104.022	4.169
4.5	475.117	360.778	241.067	120.791	4.841
5.0	542.556	412.023	275.329	137.967	5.530
(m=3, n=3)					
0.25	427.894	324.894	217.075	108.763	4.359
0.5	204.827	155.416	103.774	51.965	2.082
1.0	147.630	111.982	74.749	37.421	1.499
1.5	166.235	126.108	84.188	42.150	1.688
2.0	204.825	155.415	103.773	51.965	2.082
2.5	253.170	192.137	128.318	64.268	2.575
3.0	307.569	233.466	155.948	78.118	3.130
3.5	366.174	277.995	185.718	93.043	3.728
4.0	427.887	324.889	217.072	108.762	4.359
4.5	491.994	373.603	249.642	125.090	5.013
5.0	557.998	423.757	283.174	141.899	5.687

TABLE 15c. LOWER SET OF NATURAL FREQUENCY INCLUDING SHEAR AND ROTARY INERTIA.

$\frac{a}{b} \backslash n$	$\hat{\Omega}^2$ (B.S.A. TI. RI) $(h^2/ab=0.02), (\beta=0)$		
	(m=1)	1	2
0.25	13.930	18.560	27.319
0.5	5.309	12.503	29.212
1.0	3.501	18.560	59.774
1.5	4.068	30.498	103.677
2.0	5.309	45.825	155.319
2.5	6.981	63.680	212.202
3.0	9.001	83.536	272.872
3.5	11.327	105.024	336.401
4.0	13.930	127.869	402.154
4.5	16.878	151.863	469.680
5.0	19.877	176.839	538.647
	(m=2)		
0.25	127.871	136.880	152.209
0.5	45.826	59.775	85.319
1.0	18.560	41.444	90.725
1.5	13.448	47.317	126.879
2.0	12.503	59.774	174.276
2.5	13.066	75.824	228.330
3.0	14.420	94.377	286.937
3.5	16.293	114.853	348.880
4.0	18.560	136.878	413.372
4.5	21.152	160.186	479.868
5.0	24.025	184.578	547.979
	(m=3)		
0.25	402.160	413.378	432.174
0.5	155.321	174.278	206.882
1.0	59.775	90.725	149.114
1.5	37.208	80.006	167.905
2.0	29.212	85.318	206.880
2.5	26.204	97.328	255.707
3.0	25.471	113.200	310.650
3.5	25.994	131.712	369.839
4.0	27.318	152.208	432.168
4.5	29.211	174.276	496.914
5.0	31.540	197.632	563.575

TABLE 15d. LOWER SET OF NATURAL FREQUENCY INCLUDING SHEAR AND ROTARY INERTIA.



$\hat{\Omega}^2$  (B.S.A. TI. RI) MINIMUM

m	n	$\frac{a}{b}$	$\frac{A}{B}$ ( $h^2/ab=0.02$ )	$\hat{\Omega}^2$ (B.S.A. TI. RI) MINIMUM				
				0.0	0.01	0.25	0.50	0.75
1	1	1.0	3.501	3.466	2.626	1.751	0.875	0.035
2	1	2.0	12.503	12.378	9.378	6.253	3.127	0.125
3	1	3.0	25.471	25.216	19.108	12.741	6.372	0.255
1	2	0.5	12.503	12.378	9.378	6.253	3.127	0.125
2	2	1.0	41.444	41.030	31.095	20.739	10.373	0.415
3	2	1.5	80.006	79.208	60.051	40.065	20.047	0.803
1	3	0.33	25.471	25.216	19.108	12.741	6.372	0.255
2	3	0.67	80.006	79.208	60.051	40.065	20.047	0.803
3	3	1.0	149.114	147.630	111.982	74.749	37.421	1.499

TABLE 15e. SUMMARY SHEET FOR LOWER SET OF NATURAL FREQUENCY INCLUDING SHEAR AND ROTARY INERTIA.

$\frac{b}{a} / \frac{\hat{\alpha}}{\beta}$	$\hat{\Omega}^2$ (B.S.A. TI. RI)				$(h^2/ab=0.03)$ 0.99
	0.01	0.25	0.50	0.75	
	(m=1, n=1)				
0.25	12.434	9.422	6.283	3.142	0.126
0.5	4.900	3.712	2.475	1.238	0.050
1.0	3.268	2.476	1.651	0.826	0.033
1.5	3.782	2.866	1.911	0.955	0.038
2.0	4.900	3.712	2.475	1.238	0.050
2.5	6.388	4.840	3.227	1.614	0.065
3.0	8.167	6.188	4.126	2.063	0.083
3.5	10.192	7.723	5.150	2.575	0.103
4.0	12.434	9.422	6.283	3.142	0.126
4.5	14.870	11.269	7.515	3.759	0.150
5.0	17.482	13.249	8.836	4.420	0.177
	(m=2, n=1)				
0.25	102.114	77.490	51.747	25.915	1.038
0.5	38.726	29.362	19.591	9.803	0.392
1.0	16.372	12.407	8.274	4.139	0.166
1.5	12.020	9.109	6.074	3.038	0.122
2.0	11.207	8.492	5.663	2.832	0.113
2.5	11.693	8.860	5.908	2.955	0.118
3.0	12.853	9.740	6.495	3.248	0.130
3.5	14.451	10.951	7.303	3.653	0.146
4.0	16.372	12.407	8.274	4.139	0.166
4.5	18.552	14.060	9.377	4.691	0.188
5.0	20.952	15.880	10.592	5.298	1.212
	(m=3, n=1)				
0.25	300.684	228.368	152.617	76.481	3.065
0.5	122.647	93.088	62.174	31.142	1.248
1.0	49.811	37.774	25.208	12.617	0.505
1.5	31.778	24.091	16.072	8.041	0.322
2.0	25.224	19.135	12.764	6.385	0.256
2.5	22.761	17.252	11.507	5.756	0.230
3.0	22.153	16.791	11.199	5.602	0.224
3.5	22.587	17.119	11.419	5.712	0.229
4.0	23.683	17.951	11.973	5.990	0.240
4.5	25.244	19.135	12.764	6.385	0.255
5.0	27.157	20.585	13.732	6.870	0.275

TABLE 16a. LOWER SET OF NATURAL FREQUENCY INCLUDING SHEAR AND ROTARY INERTIA.

$\frac{a}{b} \cdot \beta$	$\Omega^2$ (B.S.A. TI. RI)					$(h^2/ab=0.03)$ 0.99
	0.01	0.25	0.50	0.75		
(m=1, n=2)						
0.25	16.372	12.407	8.274	4.139		0.166
0.5	11.207	8.492	5.663	2.832		0.113
1.0	16.372	12.407	8.274	4.139		0.166
1.5	26.301	19.937	13.299	6.653		0.266
2.0	38.726	29.362	19.591	9.803		0.392
2.5	52.884	40.106	26.766	13.397		0.536
3.0	68.344	51.843	34.607	17.325		0.694
3.5	84.821	64.355	42.968	21.515		0.862
4.0	102.113	77.488	51.746	25.915		1.038
4.5	120.073	91.132	60.866	30.486		1.221
5.0	138.590	105.201	70.271	35.201		1.411
(m=2, n=2)						
0.25	108.879	82.629	55.182	27.637		1.107
0.5	49.811	37.774	25.208	12.617		0.505
1.0	35.204	26.689	17.807	8.910		0.357
1.5	39.920	30.268	20.196	10.106		0.405
2.0	49.810	37.773	25.208	12.617		0.505
2.5	62.370	47.307	31.577	15.807		0.633
3.0	76.686	58.177	38.840	19.446		0.779
3.5	92.286	70.025	46.757	23.414		0.938
4.0	108.878	82.628	55.181	27.637		1.107
4.5	126.262	95.834	64.010	32.062		1.285
5.0	144.296	109.536	73.170	36.654		1.469
(m=3, n=2)						
0.25	308.601	234.383	156.638	78.496		3.146
0.5	136.699	103.764	69.311	34.720		1.391
1.0	73.883	56.048	37.417	18.733		0.750
1.5	65.614	49.770	33.222	16.631		0.666
2.0	69.720	52.888	35.305	17.675		0.708
2.5	78.946	59.894	39.987	20.021		0.812
3.0	91.033	69.073	46.121	23.096		0.925
3.5	105.002	79.683	53.213	26.650		1.068
4.0	120.330	91.328	60.997	30.552		1.224
4.5	136.697	103.762	69.310	34.719		1.391
5.0	153.888	116.824	78.043	39.097		1.567

TABLE 16b. LOWER SET OF NATURAL FREQUENCY INCLUDING SHEAR AND ROTARY INERTIA.

$\frac{a}{b} \backslash \hat{\beta}$	$\hat{\Omega}^2$ (B.S.A. TI. RI)					$(h^2/ab=0.03)$
	0.01 (m=1, n=3)	0.25	0.50	0.75	0.99	
0.25	23.683	17.951	11.973	5.990	0.240	
0.5	25.244	19.135	12.764	6.385	0.256	
1.0	49.811	37.774	25.208	12.617	0.505	
1.5	83.795	63.576	42.447	21.254	0.851	
2.0	122.646	93.087	62.173	31.141	1.248	
2.5	164.552	124.927	83.460	41.813	1.676	
3.0	208.547	158.358	105.812	53.019	2.125	
3.5	254.048	192.933	128.928	64.607	2.589	
4.0	300.679	228.365	152.615	76.480	3.065	
4.5	348.184	264.456	176.740	88.571	3.550	
5.0	396.379	301.068	201.209	100.833	4.041	
(m=2, n=3)						
0.25	120.332	91.329	60.998	30.552	1.224	
0.5	69.720	52.888	35.305	17.675	0.708	
1.0	73.883	56.048	37.417	18.773	0.750	
1.5	101.367	76.922	51.367	25.725	1.031	
2.0	136.698	103.763	69.310	34.719	1.391	
2.5	176.308	133.860	89.433	44.807	1.796	
3.0	218.665	166.046	110.952	55.596	2.228	
3.5	262.931	199.683	133.441	66.869	2.680	
4.0	308.596	234.380	156.635	78.495	3.146	
4.5	355.323	269.879	180.465	90.388	3.623	
5.0	402.878	306.005	204.509	102.486	4.108	
(m=3, n=3)						
0.25	321.842	244.443	163.363	81.867	3.281	
0.5	160.664	121.973	81.485	40.823	1.636	
1.0	118.025	89.576	59.826	29.965	1.201	
1.5	131.985	100.182	66.916	33.519	1.343	
2.0	160.663	121.972	81.484	40.822	1.636	
2.5	196.160	148.945	99.519	49.864	1.998	
3.0	235.664	178.964	119.589	59.925	2.402	
3.5	277.814	210.991	141.000	70.658	2.832	
4.0	321.837	244.440	163.360	81.866	3.281	
4.5	367.251	278.941	186.421	93.423	3.744	
5.0	413.730	314.248	210.017	105.247	4.218	

TABLE 16c. LOWER SET OF NATURAL FREQUENCY INCLUDING SHEAR AND ROTARY INERTIA.

$\frac{a}{b} \eta$ (m=1)	$\hat{\Omega}^2$ (B.S.A. TI. RI) $(h^2/ab=0.03), (\beta=0)$		
	1	2	3
0.25	12.560	16.537	23.922
0.5	4.949	11.320	25.498
1.0	3.301	16.537	50.312
1.5	3.821	26.566	84.636
2.0	4.949	39.115	123.875
2.5	6.452	53.416	166.199
3.0	8.249	69.031	210.632
3.5	10.295	85.672	256.587
4.0	12.560	103.137	303.683
4.5	15.020	121.276	351.661
5.0	17.658	139.978	400.337
(m=2)			
0.25	103.138	109.971	121.538
0.5	39.116	50.312	70.421
1.0	16.537	35.558	74.625
1.5	12.142	40.322	102.384
2.0	11.320	50.311	138.067
2.5	11.811	62.997	178.072
3.0	12.983	77.456	220.851
3.5	14.596	93.212	265.558
4.0	16.537	109.969	311.679
4.5	18.739	127.527	358.871
5.0	21.163	145.741	406.901
(m=3)			
0.25	303.688	311.683	325.056
0.5	123.876	138.068	162.272
1.0	50.312	74.625	119.207
1.5	32.098	66.273	133.307
2.0	25.499	70.420	162.271
2.5	22.991	79.739	198.122
3.0	22.376	91.947	238.020
3.5	22.814	106.055	280.589
4.0	23.921	121.536	325.052
4.5	25.498	138.066	370.918
5.0	27.430	155.429	417.862

TABLE 16d. LOWER SET OF NATURAL FREQUENCY INCLUDING SHEAR AND ROTARY INERTIA.

$\hat{\Omega}^2$  (B.S.A. TI. RI) MINIMUM

m	n	$\frac{a/b}{\beta}$	$(h^2/ab=0.03)$					
			0.0	0.01	0.25	0.50	0.75	0.99
1	1	1.0	3.301	3.268	2.476	1.651	0.826	0.033
2	1	2.0	11.320	11.207	8.492	5.663	2.832	0.113
3	1	3.0	22.376	22.153	16.791	11.199	5.602	0.224
1	2	0.5	11.320	11.207	8.492	5.663	2.832	0.113
2	2	1.0	35.558	35.204	26.689	17.807	8.910	0.357
3	2	1.5	66.273	65.614	49.770	33.222	16.631	0.666
1	3	0.33	22.376	22.153	16.791	11.199	5.602	0.224
2	3	0.67	66.273	65.614	49.770	33.222	16.631	0.666
3	3	1.0	119.207	118.025	89.576	59.826	29.965	1.201

TABLE 16e. SUMMARY SHEET FOR LOWER SET OF NATURAL FREQUENCY INCLUDING SHEAR AND ROTARY INERTIA.

$\frac{b^2}{a^2} / \hat{\beta}$	$\hat{\Omega}^2$ (B.S.A. TI. RI)				$(h^2/ab=0.04)$ 0.99
	0.01	0.25	0.50	0.75	
(m=1, n=1)					
0.25	11.342	8.596	5.733	2.868	0.115
0.5	4.594	3.481	2.321	1.161	0.046
1.0	3.094	2.345	1.563	0.782	0.031
1.5	3.569	2.704	1.803	0.902	0.036
2.0	4.594	3.481	2.321	1.161	0.046
2.5	5.946	4.506	3.005	1.503	0.060
3.0	7.5482	5.720	3.814	1.908	0.076
3.5	9.356	7.091	4.729	2.365	0.095
4.0	11.342	8.596	5.733	2.868	0.115
4.5	13.484	10.219	6.817	3.410	0.137
5.0	15.761	11.948	7.970	3.987	0.160
(m=2, n=1)					
0.25	85.841	65.166	43.532	21.808	0.874
0.5	33.880	25.697	17.152	8.568	0.344
1.0	14.795	11.214	7.481	3.742	0.150
1.5	10.977	8.319	5.549	2.775	0.111
2.0	10.258	7.774	5.185	2.593	0.104
2.5	10.687	8.400	5.402	2.702	0.108
3.0	11.711	8.876	5.920	2.961	0.119
3.5	13.115	9.941	6.631	3.317	0.133
4.0	14.794	11.214	7.481	3.742	0.150
4.5	16.691	12.653	8.441	4.223	0.169
5.0	18.767	14.228	9.492	4.749	0.190
(m=3, n=1)					
0.25	242.112	183.896	122.900	61.589	2.469
0.5	102.321	77.688	51.905	26.006	1.042
1.0	43.136	32.725	21.848	10.939	0.438
1.5	28.019	21.248	14.180	7.097	0.284
2.0	22.455	17.026	11.360	5.685	0.228
2.5	20.325	15.410	10.281	5.145	0.206
3.0	19.802	15.013	10.016	5.012	0.201
3.5	20.175	15.296	10.205	5.107	0.204
4.0	21.117	16.010	10.682	5.345	0.214
4.5	22.455	17.026	11.360	5.685	0.228
5.0	24.090	18.266	12.188	6.100	0.224

TABLE 17a. LOWER SET OF NATURAL FREQUENCY INCLUDING SHEAR AND ROTARY INERTIA.

$\frac{a/b}{\beta}$	$\hat{\Omega}^2$ (B.S.A. TI. RI)					$(h^2/ab=0.04)$ 0.99
	0.01	0.25	0.50	0.75		
(m=1, n=2)						
0.25	14.795	11.214	7.481	3.742	0.150	
0.5	10.258	7.774	5.185	2.593	0.104	
1.0	14.795	11.214	7.481	3.742	0.150	
1.5	23.359	17.712	11.818	5.914	0.237	
2.0	33.879	25.696	17.151	8.586	0.344	
2.5	45.686	34.661	23.141	11.587	0.464	
3.0	58.421	44.333	29.606	14.827	0.594	
3.5	71.856	54.540	36.429	18.247	0.731	
4.0	85.839	65.165	43.532	21.808	0.874	
4.5	100.261	76.123	50.859	25.481	1.021	
5.0	115.041	87.354	58.368	29.246	1.172	
(m=2, n=2)						
0.25	91.284	69.302	46.298	23.195	0.930	
0.5	43.137	32.725	21.848	10.939	0.438	
1.0	30.915	23.446	15.648	7.833	0.314	
1.5	34.882	26.458	17.660	8.840	0.354	
2.0	43.136	32.725	21.847	10.939	0.438	
2.5	53.516	40.608	27.116	13.579	0.544	
3.0	65.238	49.513	33.068	16.562	0.664	
3.5	77.906	59.137	39.502	19.788	0.793	
4.0	91.283	69.301	46.297	23.194	0.930	
4.5	105.211	79.884	53.373	26.742	1.072	
5.0	119.579	90.803	60.673	30.402	1.218	
(m=3, n=2)						
0.25	248.249	188.557	126.015	63.150	2.531	
0.5	113.535	86.210	57.603	28.862	1.157	
1.0	62.951	47.775	31.906	15.980	0.640	
1.5	56.182	42.633	28.469	14.257	0.571	
2.0	59.548	45.189	30.178	15.114	0.606	
2.5	67.080	50.912	34.003	17.031	0.682	
3.0	76.893	58.367	38.987	19.530	0.783	
3.5	88.166	66.933	44.714	22.401	0.898	
4.0	100.467	76.279	50.963	25.534	1.023	
4.5	113.534	86.209	57.602	28.862	1.157	
5.0	127.195	96.589	64.542	32.341	1.296	

TABLE 17b. LOWER SET OF NATURAL FREQUENCY INCLUDING SHEAR AND ROTARY INERTIA.



$\frac{\sigma_a}{\beta}$	$\hat{\Omega}^2$ (B.S.A. TI. RI)				
	0.01 (m=1, n=3)	0.25	0.50	0.75	(h <sup>2</sup> /ab=0.04) 0.99
0.25	21.117	16.011	10.682	5.345	0.214
0.5	22.455	17.026	11.360	5.685	0.228
1.0	43.136	32.725	21.848	10.939	0.438
1.5	71.023	53.907	36.006	18.035	0.723
2.0	102.320	77.687	51.904	26.006	1.042
2.5	135.639	103.006	68.832	34.492	1.382
3.0	170.286	129.332	86.432	43.314	1.736
3.5	205.859	156.357	104.496	52.367	2.099
4.0	242.109	183.893	122.899	61.588	2.468
4.5	278.869	211.812	141.554	70.934	2.843
5.0	316.028	240.028	160.404	80.377	3.221
(m=2, n=3)					
0.25	100.468	76.280	50.964	25.534	1.023
0.5	59.548	45.190	30.178	15.114	0.606
1.0	62.951	47.775	31.906	15.980	0.640
1.5	85.239	64.708	43.227	21.655	0.868
2.0	113.534	86.209	57.602	28.862	1.157
2.5	144.926	110.063	73.550	36.857	1.477
3.0	178.216	135.357	90.459	45.332	1.817
3.5	212.779	161.614	108.010	54.128	2.170
4.0	248.245	188.554	126.013	63.149	2.531
4.5	284.382	215.998	144.350	72.335	2.899
5.0	321.030	243.826	162.941	81.647	3.272
(m=3, n=3)					
0.25	258.503	196.345	131.219	65.757	2.636
0.5	132.562	100.668	67.269	33.708	1.351
1.0	98.621	74.877	50.025	25.063	1.004
1.5	109.779	83.355	55.694	27.905	1.118
2.0	132.562	100.667	67.269	33.708	1.351
2.5	160.560	121.942	81.492	40.837	1.637
3.0	191.513	145.459	97.212	48.717	1.953
3.5	224.356	170.409	113.887	57.073	2.288
4.0	258.500	196.342	131.217	65.756	2.635
4.5	293.585	222.987	149.020	74.674	2.993
5.0	329.377	250.164	167.174	83.768	3.357

TABLE 17c. LOWER SET OF NATURAL FREQUENCY INCLUDING SHEAR AND ROTARY INERTIA.

TABLE 17c. LOWER SET OF NATURAL FREQUENCY INCLUDING SHEAR AND ROTARY INERTIA.

$\frac{a}{b} \eta$ (m=1)	$\hat{\Omega}^2$ (B.S.A. TI. RI) $(h^2/ab=0.04), (\hat{\beta}=0)$		
	1	2	3
0.25	11.456	14.944	21.330
0.5	4.640	10.361	22.681
1.0	3.126	14.944	43.570
1.5	3.605	23.594	71.735
2.0	4.640	34.220	103.343
2.5	6.006	46.145	136.995
3.0	7.624	59.006	171.987
3.5	9.451	72.576	207.915
4.0	11.456	86.699	244.526
4.5	13.619	101.264	281.654
5.0	15.920	116.191	319.184
(m=2)			
0.25	86.700	92.198	101.474
0.5	34.220	43.570	60.145
1.0	14.944	31.226	63.582
1.5	11.087	35.233	86.092
2.0	10.361	43.569	114.670
2.5	10.795	54.053	146.375
3.0	11.829	65.892	179.996
3.5	13.247	78.687	214.904
4.0	14.943	92.196	250.724
4.5	16.859	106.263	287.221
5.0	18.956	120.775	324.236
(m=3)			
0.25	244.530	250.728	261.085
0.5	103.344	114.671	133.887
1.0	43.570	63.582	99.608
1.5	28.300	56.745	110.877
2.0	22.681	60.145	133.887
2.5	20.530	67.752	162.164
3.0	20.001	77.663	193.426
3.5	20.378	89.049	226.596
4.0	21.330	101.472	261.081
4.5	22.681	114.669	296.517
5.0	24.332	128.466	332.667

TABLE 17d. LOWER SET OF NATURAL FREQUENCY INCLUDING SHEAR AND ROTARY INERTIA.

		$\overset{A}{\Omega}^2$ (B.S.A. TI. RI) MINIMUM						
m	n	$\frac{\alpha}{\beta}$	$(h^2/ab=0.04)$					
			0.0	0.01	0.25	0.50	0.75	0.99
1	1	1.0	3.126	3.094	2.345	1.563	0.782	0.031
2	1	2.0	10.361	10.258	7.774	5.185	2.593	0.104
3	1	3.0	20.001	19.802	15.013	10.016	5.012	0.201
1	2	0.5	10.361	10.258	7.774	5.185	2.593	0.104
2	2	1.0	31.226	30.915	23.446	15.648	7.833	0.314
3	2	1.5	56.745	56.182	42.633	28.469	14.257	0.571
1	3	0.33	20.001	19.802	15.013	10.016	5.012	0.201
2	3	0.67	56.745	56.182	42.633	28.469	14.257	0.571
3	3	1.0	99.608	98.621	74.877	50.025	25.063	1.004

TABLE 17e. SUMMARY SHEET FOR LOWER SET OF NATURAL FREQUENCY INCLUDING SHEAR AND ROTARY INERTIA.

$\frac{a}{b}$	$\hat{\Omega}^2$ (B.S.A. TI. RI)					$(h^2/ab=0.05)$
	0.01	0.25	0.50	0.75	0.99	
	(m=1, n=1)					
0.25	10.441	7.914	5.280	2.642	0.106	
0.5	4.327	3.279	2.187	1.094	0.044	
1.0	2.940	2.228	1.485	0.743	0.030	
1.5	3.381	2.562	1.708	0.854	0.034	
2.0	4.327	3.279	2.187	1.094	0.044	
2.5	5.567	4.219	2.814	1.407	0.056	
3.0	7.025	5.324	3.551	1.776	0.071	
3.5	8.659	6.563	4.378	2.190	0.088	
4.0	10.440	7.914	5.280	2.641	0.106	
4.5	12.350	9.363	6.247	3.126	0.125	
5.0	14.371	10.896	7.270	3.638	0.146	
	(m=2, n=1)					
0.25	74.171	56.320	37.632	18.856	0.756	
0.5	30.166	22.887	15.281	7.652	0.307	
1.0	13.515	10.247	6.837	3.421	0.137	
1.5	10.113	7.666	5.114	2.559	0.102	
2.0	9.469	7.177	4.488	2.395	0.096	
2.5	9.854	7.470	4.983	2.493	0.100	
3.0	10.771	8.165	5.447	2.725	0.109	
3.5	12.023	9.115	6.081	3.043	0.122	
4.0	13.515	10.247	6.837	3.421	0.137	
4.5	15.193	11.520	7.687	3.847	0.154	
5.0	17.022	12.908	8.614	4.311	0.173	
	(m=3, n=1)					
0.25	202.861	154.076	102.965	51.594	2.068	
0.5	87.921	66.769	44.618	22.358	0.896	
1.0	38.111	28.921	19.314	9.673	0.388	
1.5	25.099	19.040	12.710	6.363	0.255	
2.0	20.256	15.363	10.253	5.132	0.206	
2.5	18.391	13.947	9.308	4.659	0.187	
3.0	17.932	13.599	9.075	4.542	0.182	
3.5	18.259	13.847	9.241	4.625	0.185	
4.0	19.085	14.474	9.660	4.835	0.194	
4.5	20.256	15.362	10.253	5.132	0.206	
5.0	21.683	16.446	10.977	5.495	0.220	

TABLE 18a. LOWER SET OF NATURAL FREQUENCY INCLUDING SHEAR AND ROTARY INERTIA.

$\frac{a}{b} \backslash \hat{\beta}$	$\hat{\Omega}^2$ (B.S.A. TI. RI)					$(h^2/ab=0.05)$ 0.99
	0.01	0.25	0.50	0.75		
(m=1, n=2)						
0.25	13.515	10.247	6.837	3.421	0.137	
0.5	9.469	7.177	4.788	2.395	0.096	
1.0	13.515	10.247	6.837	3.421	0.137	
1.5	21.046	15.962	10.654	5.333	0.214	
2.0	30.166	22.887	15.281	7.652	0.307	
2.5	40.288	30.575	20.420	10.227	0.410	
3.0	51.108	38.796	25.915	12.982	0.520	
3.5	62.442	47.408	31.673	15.869	0.636	
4.0	74.170	56.319	37.631	18.856	0.756	
4.5	86.206	65.466	43.746	21.921	0.879	
5.0	98.491	74.800	49.987	25.050	1.004	
(m=2, n=2)						
0.25	78.720	59.777	39.943	20.015	0.802	
0.5	38.111	28.921	19.314	9.673	0.388	
1.0	27.607	20.944	13.982	7.001	0.280	
1.5	31.030	23.543	15.719	7.871	0.315	
2.0	38.110	28.921	19.314	9.673	0.388	
2.5	46.951	35.638	23.804	11.924	0.478	
3.0	56.869	43.173	28.842	14.449	0.579	
3.5	67.524	51.270	34.255	17.163	0.688	
4.0	78.719	59.777	39.943	20.014	0.802	
4.5	90.325	68.596	45.839	22.970	0.921	
5.0	102.255	77.660	51.899	26.008	1.042	
(m=3, n=2)						
0.25	207.865	157.875	105.503	52.865	2.119	
0.5	97.242	73.851	49.353	24.731	0.991	
1.0	54.938	41.706	27.861	13.957	0.559	
1.5	49.212	37.355	24.952	12.499	0.501	
2.0	52.062	39.520	26.400	13.225	0.530	
2.5	58.422	44.353	29.630	14.845	0.595	
3.0	66.674	50.623	33.823	16.946	0.679	
3.5	76.115	57.798	38.620	19.351	0.776	
4.0	86.378	65.596	43.823	21.965	0.880	
4.5	97.241	73.850	49.352	24.731	0.991	
5.0	108.561	82.452	55.102	27.613	1.107	

TABLE 18b. LOWER SET OF NATURAL FREQUENCY INCLUDING SHEAR AND ROTARY INERTIA.

$\frac{a}{b} \hat{\beta}$	$\hat{\Omega}^2$ (B.S.A. TI. RI)				
	0.01	0.25	0.50	0.75	( $h^2/ab=0.05$ ) 0.99
(m=1, n=3)					
0.25	19.085	14.474	9.660	4.835	0.194
0.5	20.256	15.363	10.253	5.132	0.206
1.0	38.111	28.921	19.314	9.673	0.388
1.5	61.742	46.876	31.317	15.690	0.629
2.0	87.920	66.768	44.617	22.358	0.896
2.5	115.543	87.756	58.648	29.391	1.178
3.0	144.083	109.438	73.139	36.653	1.469
3.5	173.247	131.588	87.941	44.069	1.766
4.0	202.858	154.074	102.963	51.594	2.068
4.5	232.801	176.806	118.147	59.198	2.372
5.0	262.997	199.728	133.455	66.863	2.679
(m=2, n=3)					
0.25	86.379	65.597	43.834	21.965	0.880
0.5	52.062	39.521	26.400	13.225	0.530
1.0	54.938	41.706	27.861	13.957	0.559
1.5	73.667	55.938	37.376	18.728	0.751
2.0	97.241	73.851	49.352	24.731	0.991
2.5	123.209	93.580	62.541	31.342	1.256
3.0	150.595	114.384	76.445	38.309	1.535
3.5	178.907	135.887	90.813	45.507	1.824
4.0	207.862	157.873	105.501	52.865	2.119
4.5	237.284	180.210	120.420	60.337	2.418
5.0	267.058	202.809	135.513	67.894	2.721
(m=3, n=3)					
0.25	216.221	164.220	109.740	54.988	2.204
0.5	113.000	85.825	57.357	28.743	1.152
1.0	84.840	64.427	43.052	21.573	0.865
1.5	94.122	71.481	47.768	23.937	0.959
2.0	113.000	85.824	57.356	28.743	1.152
2.5	136.086	103.363	69.080	34.618	1.388
3.0	161.500	122.667	81.980	41.082	1.647
3.5	188.369	143.072	95.613	47.912	1.920
4.0	216.219	164.218	109.739	54.987	2.204
4.5	244.767	185.890	124.214	62.236	2.494
5.0	273.831	207.950	138.945	69.612	2.789

TABLE 18c. LOWER SET OF NATURAL FREQUENCY INCLUDING SHEAR AND ROTARY INERTIA.

$\frac{a}{b}$	$\hat{\Omega}^2$ (B.S.A. TI. RI) $(h^2/ab=0.05), (\hat{\beta}=0)$		
	1	2	3
(m=1)			
0.25	10.546	13.651	19.277
0.5	4.371	9.564	20.459
1.0	2.970	13.651	38.493
1.5	3.415	21.257	62.360
2.0	4.371	30.469	88.799
2.5	5.624	40.692	116.697
3.0	7.096	51.620	145.522
3.5	8.746	63.067	174.977
4.0	10.546	74.911	204.884
4.5	12.474	87.068	235.126
5.0	14.516	99.475	265.625
(m=2)			
0.25	74.913	79.507	87.242
0.5	30.469	38.493	52.583
1.0	13.651	27.885	55.488
1.5	10.215	31.341	74.404
2.0	9.564	38.492	98.213
2.5	9.953	47.421	124.440
3.0	10.879	57.438	152.099
3.5	12.144	68.200	180.694
4.0	13.651	79.506	209.938
4.5	15.346	91.228	239.655
5.0	17.194	103.276	269.726
(m=3)			
0.25	204.887	209.941	218.381
0.5	88.800	98.213	114.129
1.0	38.493	55.488	85.688
1.5	25.352	49.705	95.063
2.0	20.460	52.583	114.128
2.5	18.576	59.006	137.445
3.0	18.112	67.341	163.113
3.5	18.443	76.877	190.840
4.0	19.277	87.241	218.378
4.5	20.459	98.212	247.212
5.0	21.901	109.646	276.567

TABLE 18d. LOWER SET OF NATURAL FREQUENCY INCLUDING SHEAR AND ROTARY INERTIA.

$\hat{\Omega}^2$  (B.S.A. TI. RI) MINIMUM

m	n	$\frac{a/b}{\hat{P}}$	$(h^2/ab=0.05)$					
			0.0	0.01	0.25	0.50	0.75	0.99
1	1	1.0	2.970	2.940	2.228	1.485	0.743	0.030
2	1	2.0	9.954	9.469	7.177	4.788	2.395	0.096
3	1	3.0	18.112	17.932	13.599	9.075	4.542	0.182
1	2	0.50	9.954	9.469	7.177	4.788	2.395	0.096
2	2	1.0	27.885	27.607	20.944	13.982	7.001	0.280
3	2	1.50	49.705	49.212	37.355	24.952	12.499	0.501
1	3	0.33	18.112	17.932	13.599	9.075	4.542	0.182
2	3	0.67	49.705	49.212	37.355	24.952	12.499	0.501
3	3	1.0	85.688	84.840	64.427	43.052	21.573	0.865

TABLE 18e. SUMMARY SHEET FOR LOWER SET OF NATURAL FREQUENCY INCLUDING SHEAR AND ROTARY INERTIA.



$\frac{a}{b} \sqrt{\frac{b^2}{a^2}}$	$\Omega^2$ (B.S.A. TI. RI)				
	0.01	0.02	0.03	0.04	0.05
(m=1, n=1)					
0.25	4963.48	1397.73	689.01	424.89	295.41
0.5	4698.59	1268.93	605.00	363.00	246.62
1.0	4622.08	1231.48	580.48	344.88	232.32
1.5	4647.63	1244.00	588.68	350.95	237.11
2.0	4698.59	1268.93	605.00	363.00	246.62
2.5	4759.53	1298.67	624.44	377.34	257.93
3.0	4825.27	1330.67	645.33	392.73	270.07
3.5	4893.62	1363.87	666.96	408.67	282.63
4.0	4963.47	1397.72	689.01	424.89	295.41
4.5	5034.22	1431.95	711.26	441.27	308.32
5.0	5105.52	1466.37	733.64	457.72	321.28
(m=2, n=1)					
0.25	6693.66	2226.08	1226.63	820.80	607.97
0.5	5590.91	1699.56	884.94	568.99	408.98
1.0	5075.71	1451.99	724.29	450.85	315.87
1.5	4950.96	1391.67	685.06	421.99	293.13
2.0	4925.90	1379.52	677.16	416.17	288.54
2.5	4940.94	1386.81	681.90	419.66	291.30
3.0	4975.98	1403.78	692.94	427.79	297.70
3.5	5022.36	1426.21	707.53	438.52	306.16
4.0	5075.71	1451.99	724.29	450.85	315.87
4.5	5133.63	1479.93	742.44	464.20	326.39
5.0	5194.66	1509.34	761.54	478.25	337.45
(m=3, n=1)					
0.25	9402.12	3522.29	2073.08	1448.22	1106.19
0.5	7006.50	2375.31	1323.68	892.49	664.75
1.0	5807.97	1803.40	952.27	618.54	448.08
1.5	5445.04	1629.66	839.61	535.65	382.69
2.0	5298.15	1559.13	793.86	502.01	356.17
2.5	5239.09	1530.72	775.42	488.45	345.49
3.0	5224.29	1523.61	770.80	485.05	342.82
3.5	5234.86	1528.69	774.10	487.48	344.73
4.0	5261.25	1541.39	782.34	493.54	349.50
4.5	5298.14	1559.13	793.85	502.00	356.17
5.0	5342.32	1580.35	807.63	512.13	364.15

TABLE 19a. HIGHER SET OF NATURAL FREQUENCY INCLUDING SHEAR AND ROTARY INERTIA.

$\frac{a}{b}$	$\hat{\Omega}^2$ (B.S.A. TI. RI)				
	0.01	0.02	0.03	0.04	0.05
(m=1, n=2)					
0.25	5075.71	1451.99	724.29	450.85	315.87
0.5	4925.90	1379.52	677.16	416.16	288.54
1.0	5075.71	1451.99	724.29	450.85	315.87
1.5	5322.70	1570.93	801.51	507.64	360.61
2.0	5590.90	1699.56	884.93	568.99	408.98
2.5	5865.50	1830.90	970.10	631.66	458.44
3.0	6141.85	1962.86	1055.69	694.71	508.23
3.5	6418.14	2094.68	1141.25	757.79	558.11
4.0	6693.63	2226.07	1226.62	820.79	607.96
4.5	6967.99	2356.94	1311.72	883.66	657.75
5.0	7241.10	2487.27	1396.56	946.38	707.47
(m=2, n=2)					
0.25	6798.24	2275.97	1259.06	844.74	626.93
0.5	5807.96	1803.40	952.27	618.54	448.08
1.0	5518.10	1664.68	862.32	552.36	395.86
1.5	5615.13	1711.16	892.46	574.52	413.35
2.0	5807.96	1803.40	952.26	618.53	448.07
2.5	6037.37	1912.98	1023.34	670.87	489.40
3.0	6283.70	2030.54	1099.62	727.08	533.83
3.5	6538.65	2152.15	1178.59	785.33	579.90
4.0	6798.22	2275.96	1259.05	844.74	626.93
4.5	7060.27	2400.97	1340.37	904.83	674.53
5.0	7323.59	2526.65	1422.20	965.35	722.51
(m=3, n=2)					
0.25	9501.26	3569.97	2104.34	1471.46	1124.69
0.5	7213.61	2474.15	1388.01	940.06	702.45
1.0	6236.51	2008.02	1085.00	716.31	525.31
1.5	6094.42	1940.22	1041.00	683.89	499.68
2.0	6165.55	1974.17	1063.03	700.12	512.51
2.5	6321.41	2048.53	1111.30	735.70	540.64
3.0	6518.61	2142.59	1172.38	780.75	576.27
3.5	6738.50	2247.47	1240.53	831.06	616.10
4.0	6971.86	2358.78	1312.92	884.54	658.45
4.5	7213.57	2474.13	1388.00	940.05	702.48
5.0	7460.70	2592.12	1464.87	996.93	747.55

TABLE 19b. HIGHER SET OF NATURAL FREQUENCY INCLUDING SHEAR AND ROTARY INERTIA.

$\frac{b/a}{h^2/ab}$	$\Omega^2$ (B.S.A. TI. RI)				
	0.01	0.02	0.03	0.04	0.05
(m=1, n=3)					
0.25	5261.25	1541.39	782.34	493.54	349.50
0.5	5298.14	1559.13	793.85	502.01	356.17
1.0	5807.96	1803.41	952.27	618.54	488.08
1.5	6401.38	2086.68	1136.06	753.96	555.08
2.0	7006.48	2375.31	1323.67	892.49	664.75
2.5	7611.02	2663.93	1511.68	1031.59	775.05
3.0	8212.07	2951.38	1699.33	1170.56	885.48
3.5	8809.05	3237.47	1886.45	1309.54	995.87
4.0	9402.06	3522.26	2073.06	1448.20	1106.18
4.5	9991.40	3805.91	2259.19	1586.66	1216.41
5.0	10577.44	4088.56	2444.93	1724.94	1326.55
(m=2, n=3)					
0.25	6971.87	2358.79	1312.93	884.55	658.46
0.5	6165.55	1974.17	1063.03	700.12	512.51
1.0	6236.50	2008.02	1085.00	716.31	525.31
1.5	6682.01	2220.53	1223.01	818.13	605.86
2.0	7213.59	2474.14	1388.00	940.06	702.45
2.5	7774.45	2742.03	1562.63	1069.32	805.00
3.0	8346.70	3015.84	1741.46	1201.91	910.31
3.5	8923.31	3292.29	1922.35	1336.20	1017.07
4.0	9501.21	2569.94	2104.32	1471.46	1124.68
4.5	10078.89	3848.08	2286.89	1607.27	1232.82
5.0	10655.69	4126.34	2469.77	1743.44	1341.30
(m=3, n=3)					
0.25	9666.21	3649.33	2156.41	1510.18	1155.52
0.5	7556.45	2637.85	1494.68	1019.00	765.06
1.0	6937.21	2342.26	1302.17	876.60	652.16
1.5	7144.68	2441.25	1366.59	924.22	689.90
2.0	7556.43	2637.85	1494.67	1018.99	765.06
2.5	8045.59	2871.70	1647.28	1132.06	854.82
3.0	8570.36	3123.00	1811.54	1253.92	951.65
3.5	9113.32	3383.52	1982.11	1380.60	1052.39
4.0	9666.16	3649.30	2156.39	1510.17	1155.51
4.5	10224.52	3918.28	2333.01	1641.60	1260.16
5.0	10785.98	4189.27	2511.16	1774.28	1365.87

TABLE 19c. HIGHER SET OF NATURAL FREQUENCY INCLUDING SHEAR AND ROTARY INERTIA.

		$\hat{\Omega}$ (B.S.A. TI. RI) MINIMUM ( $\hat{\beta}=0$ )					
m	n	$\frac{a}{b}$ / $\frac{h^2}{ab}$	0.01	0.02	0.03	0.04	0.05
1	1	1.0	4622.08	1231.48	580.58	344.88	232.32
2	1	2.0	4925.90	1379.52	677.16	416.17	288.54
3	1	3.0	5224.29	1523.61	770.80	485.05	342.82
1	2	0.5	4925.90	1379.52	677.16	416.17	288.54
2	2	1.0	5518.10	1664.68	862.32	552.36	395.86
3	2	1.5	6094.42	1940.22	1041.00	683.89	499.68
1	3	0.33	5224.29	1523.61	770.80	485.05	342.82
2	3	0.67	6094.42	1940.22	1041.00	683.89	499.68
3	3	1.0	6937.21	2342.26	1302.17	876.60	652.16

TABLE 19d. SUMMARY SHEET FOR HIGHER SET OF NATURAL FREQUENCY INCLUDING SHEAR AND ROTARY INERTIA.

$\hat{\Omega}^2$  (B.S.A. TI. RI.)

(m=1, n=1,  $h^2/ab=0.02$ )

$\frac{a}{b} \backslash \hat{\beta}$	0.01	0.25	0.50	0.75	0.99
0.25	1397.72	1397.56	1397.40	1397.25	1397.09
0.5	1268.93	1268.89	1268.84	1268.80	1268.76
1.0	1231.48	1231.45	1231.43	1231.41	1231.38
1.5	1243.99	1243.97	1243.94	1243.91	1243.88
2.0	1268.93	1268.89	1268.84	1268.80	1268.76
2.5	1298.66	1298.60	1298.54	1298.48	1298.42
3.0	1330.67	1330.58	1330.50	1330.41	1330.32
3.5	1363.86	1363.75	1363.63	1363.51	1363.39
4.0	1397.72	1397.56	1397.40	1397.24	1397.09
4.5	1431.94	1431.74	1431.53	1431.33	1431.13
5.0	1466.36	1466.11	1465.84	1465.60	1465.35

TABLE 20a. HIGHER SET OF NATURAL FREQUENCY INCLUDING SHEAR AND ROTARY INERTIA

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